# INROADS: GROUP

# Proposed Residential Subdivision, 94 Wilson Drive, Marulan

Traffic Report

Revision 2 27 June 2023

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# 1.0 Introduction

InRoads Group was engaged to undertake a Traffic Impact Assessment of a proposal for Stage 3 of the Equinox Residential Subdivision, located at 94 Wilson Drive, Marulan.

The subject site on which the proposed development is to be delivered was rezoned by Goulburn Mulwaree Council ('Council') to permit residential development following the gazettal of the *Goulburn Mulwaree Local Environmental Plan 2009*.

Stage 1 of the subdivision (involving 22 lots) was approved by Council in 2018, with construction of this stage now complete, and all lots sold. Stage 2 of the subdivision (involving an additional 126 residential lots) was approved in 2021 under DA /0334/1819, and is currently being delivered and sold in sub-stages.

The Stage 3 development (to which this report relates) comprises an additional 125 residential lots, which will bring the total yield of the subdivision to 273 residential lots. The intention is for the ultimate residential subdivision to provide in the order of 525 residential lots.

This report focuses upon the development proposed under this application (i.e. Stage 3), however consideration has also been given to the potential ultimate development in order to ensure that the Stage 3 development does not preclude or inhibit the provision of any transport infrastructure which may be required to support the ultimate development, and that the internal road network proposed to be delivered under Stage 3 will be adequate to cater for the volumes generated by the ultimate development.

This report provides relevant background information regarding the proposal, and documents the results and findings of our investigations addressing the following key traffic design elements and issues:

- The existing surrounding road network;
- Vehicular access points to the development;
- The internal road network / road hierarchy;
- Development traffic generation and distribution; and
- The anticipated traffic impacts of the proposal.

The results of our investigations addressing the above key traffic-related issues are discussed in the following sections.

# 2.0 Context

# 2.1 Subject Site and Current Use

The subject site is located at 94 Wilson Drive, approximately 1km to the southwest of the Marulan Town Centre on the western side of the Hume Highway as shown in **Figure 2.1a** below.

The site is legally described as Lot 1 in DP221236, Lot 1 in DP1136538, and Lot 2 in DP1136538, and is approximately 96.4 hectares in area. It is bounded by the Main Southern Railway Line to the north, Wilson Drive (also known as Shepherd Lane) to the south, rural and industrial land to the west, and Wilson Drive, public recreation, residential and industrial land to the east.

The majority of the site was previously used for a variety of agricultural activities including cultivation of cereal crops and pasture as well as grazing for pigs, cattle, sheep, and horses.



Figure 2.1a: Site Location



Figure 2.1b: Subject Site

# 2.2 Planning Context

A rezoning application was submitted to the Council in 2005, and a draft master plan was subsequently prepared which proposed in the order of 560–570 residential lots on approximately 49 hectares of the site, with 23 hectares of industrial land and 19 hectares reserved for open space.

This rezoning application responded to the regional need for additional residential and industrial land. It is understood that prior to merging with the City of Goulburn, Mulwaree Shire Council prepared a 'settlement strategy' to identify opportunities for future housing and employment growth in the area, and it understood that the subject site was considered for rezoning in the context of that settlement strategy.

The rezoning subsequently occurred with the gazettal of the *Goulburn Mulwaree Local Environmental Plan 2009*, with the subject site (which was previously zoned Rural (urban investigation)) now zoned R1 General Residential, RU6 Transition, IN2 Light Industrial, and RU2 Rural Landscape, as shown in **Figure 2.2** below.

Importantly, the subject proposal (as discussed in the following sections) is therefore consistent with Council's planning for Marulan.

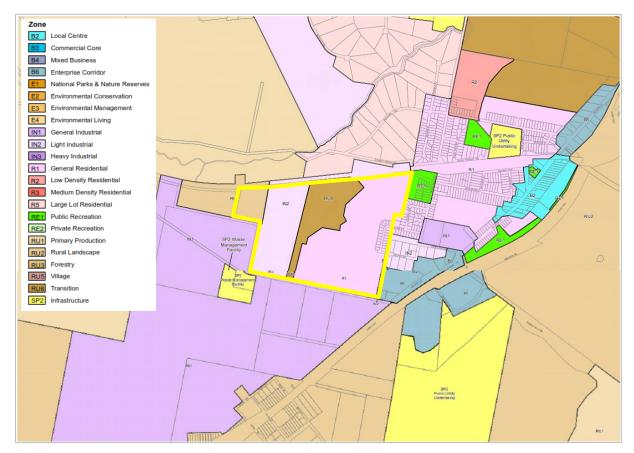


Figure 2.2: Land Zoning, Goulburn Mulwaree Local Environmental Plan 2009

Source: Land Zoning Map - Sheet LZN\_003C and Sheet LZN\_003D

# 2.3 Surrounding Road Network

The Marulan road hierarchy as defined in the Goulburn Mulwaree Council DCP is as shown in **Figure 2.3** below. As shown in this diagram, the following roads are designated by Council as Collector Roads:

- 1. Wilson Drive (part)
- 2. Portland Avenue (part)
- 3. Medway Road
- 4. George Street (part)
- 5. Brayton Road (part)

Each of the above roads, as well as key lower order roads in proximity to the site, are described following.

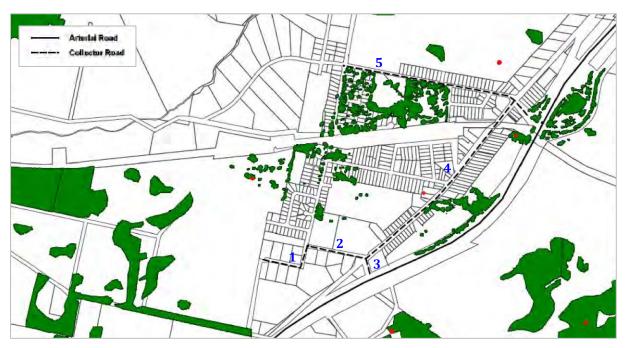


Figure 2.3: Road Hierarchy and Remnant Vegetation Map, Goulburn Mulwaree Council DCP

### 2.3.1 Collector Roads

### 2.3.1.1 Wilson Drive

The collector road section of Wilson Drive connects from the eastern boundary of the subject site to Portland Avenue, bending at 90 degrees to the north. It provides access to a small number of industrial developments along its length.

This section of Wilson Drive has a two-lane, two-way undivided cross-section, with a varying pavement width of approximately 6m which is adequate to accommodate two-way flow of cars. No kerbs are provided along the length of this road, which is of a rural standard and has unsealed shoulders. Kerbside parking demand is very limited, therefore the full pavement width is typically available for through traffic.

Both the existing width and the construction standard of this section of Wilson Drive appear to fall short of the 'Collector' standard of this road as designated by Council.

In accordance with the Voluntary Planning Agreement (VPA) for Stage 2 of the subdivision, the applicant is responsible for the upgrade and widening of Wilson Drive to increase the pavement width from approximately 6m (with no kerb and channel) to 9m (with kerb and channel), for the length of Wilson Drive from the eastern boundary of the subject site to Portland Avenue, including pavement works at the intersection of Portland Avenue and Wilson Drive, as shown in the plans included as **Appendix A**. These upgrade works are currently underway, and due for completion in around September 2023.

## 2.3.1.2 Portland Avenue (east-west section)

The collector road section of Portland Avenue (the east – west section) connects from Wilson Drive at its western end to George Street at its eastern end. It provides access to a small number of industrial developments along its length, as well as the Caltex Truck Stop on the corner of George Street.

This section of Portland Avenue has a two-lane, two-way undivided cross-section, with a varying pavement width of approximately 7.5m, which is adequate to accommodate two-way flow of heavy vehicles.

No kerbs are provided along the length of this road, which is of a rural standard and has unsealed shoulders. Kerbside parking demand is very limited, therefore the full pavement width is typically available for through traffic.

Both the existing width and the construction standard of this section of Portland Avenue appear to fall short of the 'Collector' standard of this road as designated by Council.

### 2.3.1.3 Medway Road

Medway Road is a short section of road (approximately 50m) connecting from the Hume Highway to George Street. It has a two-way, two-lane divided cross-section, with no property access along its length. Kerb and sealed shoulders are provided on Medway Road.

### 2.3.1.4 George Street

George Street is the main road running through Marulan. It extends in a north-east to south-west direction, parallel and to the west of the Hume Highway. It commences as a northbound off-ramp from the Hume Highway to the south of the town, and terminates approximately 2km to the north of the town centre.

George Street provides direct access to the two (2) Hume Highway interchanges to the north and south of the town (i.e. Brayton Road and Medway Road respectively).

George Street generally has a two-lane, two-way undivided cross-section, and as the main road running through the town centre, provides access to residential, commercial, and industrial properties along its length, as well as community uses such as Marulan Public School.

The southern section of George Street has sealed shoulders of adequate width to accommodate kerbside parking, and the section through the town centre provides on-street angle parking.

# 2.3.1.5 Brayton Road

The collector road section of Brayton Road extends in a generally east-west direction, connecting from the Hume Highway Northern Interchange to Stoney Creek Road / Maclura Drive where it then continues in a northerly direction. It provides access to primarily residential properties along its length.

The collector road section of Brayton Road has a two-lane, two-way undivided cross-section, with a pavement width of approximately 9.5m, which is more than adequate to accommodate two-way flow of traffic.

Kerbs are provided along the majority of the collector road section of Brayton Road, and kerbside parking is permitted clear of intersections and property access driveways in accordance with NSW road rules.

### 2.3.2 Local Roads

### 2.3.2.1 Collins Street

Collins Street connects from Portland Avenue at its eastern end travelling in a generally east-west direction, and has recently been extended to the south as part of Stage 1 of the subject subdivision.

The east-west section of Collins Street has a two-lane, two-way undivided cross-section with a pavement width of approximately 10.5m, which is adequate to accommodate two-way flow of traffic and kerbside parking. Kerb and channel is provided along both sides of this section of Collins Street, which provides access to small residential catchment.

The north-south (new) section of Collins Street has a two-lane, two-way undivided cross-section with a pavement width of 6m and verge widths of 4.5m on both sides, with an overall road reserve width of 15m (as approved under Stage 1 of the subject subdivision).

A pedestrian footpath is provided along the western side of the road, and a turning head has been constructed to accommodate vehicle turnaround at the termination of this road, prior to its extension as part of Stage 2 of the subdivision (as proposed under this application).

Collins Street will form a secondary access to the subject subdivision, as described in the following sections.

### 2.3.2.2 Portland Avenue (north-south section)

Portland Avenue (north-south section) connects from Wilson Drive at its southern end and terminates at the Main Southern Railway Line. It provides access to primarily residential properties along its length.

This section of Portland Avenue has a two-lane, two-way undivided cross-section, with a pavement width of approximately 11m, which is adequate to accommodate two-way flow of traffic and kerbside parking. Kerbs are provided along some sections of this road.

### 2.3.2.3 Goulburn Street

Goulburn Street connects from Portland Avenue at its western end to George Street at its eastern end. It provides access to primarily residential properties along its length.

Goulburn Street has a two-lane, two-way undivided cross-section, with a pavement width of approximately 11m, which is adequate to accommodate two-way flow of traffic and kerbside parking.

Kerbs are provided along the majority of Goulburn Street, and kerbside parking is permitted clear of intersections and property access driveways in accordance with NSW road rules.

### 2.3.3 Key Intersections

### 2.3.3.1 Hume Highway Northern Interchange

The Hume Highway Northern Interchange is a high-standard, high-capacity interchange connecting to Brayton Road to the north of Marulan Town Centre, as shown in **Figure 2.3.3.1** over page.

It provides for all movements between the highway and Marulan, with the exception of the northbound exit movement (which is catered for to the south).



Figure 2.3.3.1: Hume Highway Northern Interchange

# 2.3.3.2 Hume Highway Southern Intersection

The Hume Highway Southern Intersection caters for the left-in, left-out only movements between the Hume Highway and Medway Road, to the south of Marulan Town Centre. It also caters for the through movement from the service centre on the southern side of the highway, to Medway Road.



Figure 2.3.3.2: Hume Highway Southern Intersection

# 2.4 Public Transport Services and Accessibility

The Main Southern Railway Line passes through Marulan, and adjoins the northern boundary of the subject site. There is a rail station at Marulan located to the north of the town centre, approximately 1.5km from the subject site as shown in **Figure 2.4a** below.

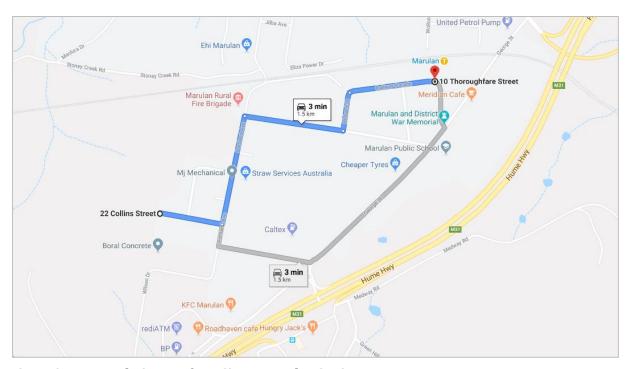


Figure 2.4a: Travel Distance from Site to Marulan Station

As part of the Intercity Trains Network, rail services operate on the Southern Highlands Line through Marulan connecting from Goulburn via Marulan to Moss Vale, Campbelltown and/or Sydney City (Central Station).

**Table 2.4** below summarises the daily train services which currently stop at Marulan Station, which are relatively limited.

Table 2.4: Daily Train Services stopping at Marulan Station

	Eastbound Services (Daily)	Westbound Services (Daily)
Monday - Friday	3 terminating at Campbelltown 1 terminating at Central 1 terminating at Moss Vale (Bus)	4 terminating at Goulburn
Weekends and Public Holidays	1 terminating at Central 1 terminating at Moss Vale (Bus) 1 terminating at Campbelltown (Sat only) 1 terminating at Moss Vale (Sun only)	3 terminating at Goulburn

A public bus service also operates between Goulburn and Moss Vale (SH100), stopping on George Street just north of the Main Southern Railway Line approximately 1.7km from the subject site as shown in **Figure 2.4b** below.

This bus stop is serviced by only one (1) service in each direction per day on weekdays, and one (1) service in each direction per day on weekends and public holidays.

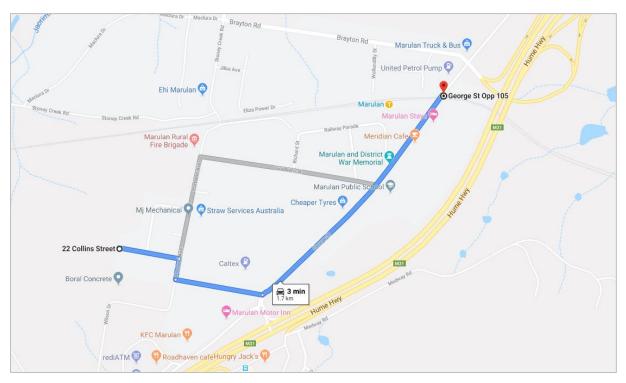


Figure 2.4b: Travel Distance from Site to George Street Bus Stop

Overall, the subject site, and Marulan in general, are not currently well serviced by public transport. This is reflected in the ABS Census data for the Goulburn Region (Statistical Area Level 2) as shown in **Figure 2.4c** over page, which indicates that less than 1% of persons within the region (which includes Marulan) commute using public transport.

Given the limited residential and employment density in Marulan it is understood that the limited public transport services do not currently present a significant issue for local residents and workers, however the relevant authorities may wish to investigate opportunities to extend existing bus services or increase frequency of services, prior to residential and employment density in the town (which Council's planning seeks to increase) reaching a point at which this may be warranted, or is able to be supported.

This is reflected in the Goulburn Mulwaree Council Delivery Program 2017 – 2021, which identifies the objective to 'improve public transport links to connect towns within the region and increase access to major centres' (CSP Strategy IN2).

It is also reflected in the Draft Goulburn Mulwaree Council Delivery Program 2022 – 2026, which identifies the objective to 'liaise with service providers to increase public bus coverage within the LGA' (Strategy D.1).

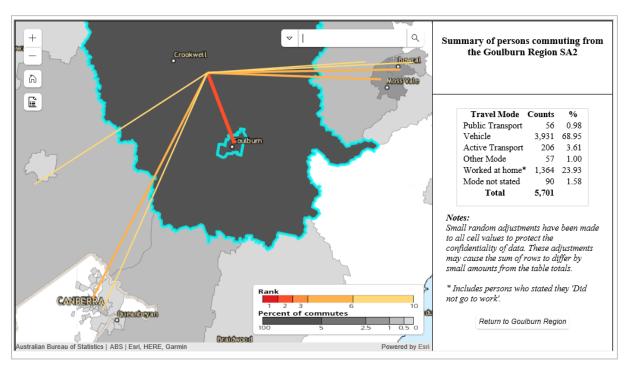


Figure 2.4c: Summary of Persons Commuting from the Goulburn Region SA2

Source: 2071.0.55.001 - Census of Population and Housing: Commuting to Work - More Stories from the Census, 2016

 $\frac{https://www.abs.gov.au/ausstats/abs@.nsf/Lookup/by\%20Subject/2071.0.55.001\sim2016\sim Main\%20Features\sim Journey\%20to\%20Work\%20from\%20Place\%20of\%20Usual\%20Residence\sim55$ 

# 3.0 Proposal

As previously noted, Stages 1 and 2 of the subdivision (involving a total of 148 lots) have been approved by Council. These lots will be accessed via one of two (2) accesses onto the external road network, via the extension of Wilson Road to the west (the primary access) and the extension of Collins Street.

The development for which approval is now sought is Stage 3 of the development, to the north of approved Stage 1 and Stage 2. It will comprise 125 residential lots and 2 drainage reserve lots. Stage 3 subdivision plans are included for reference as **Appendix B**, with an extract from the general arrangement plan provided for reference as **Figure 3** below.

It is intended that the remaining residentially zoned land be subdivided in future to achieve an overall subdivision yield in the order of 525 lots, however these future stages of the subdivision would be subject to future development applications. This application relates to Stage 3 of the subdivision (125 residential lots) only.

The key traffic elements of the proposal are discussed further in the following sections.

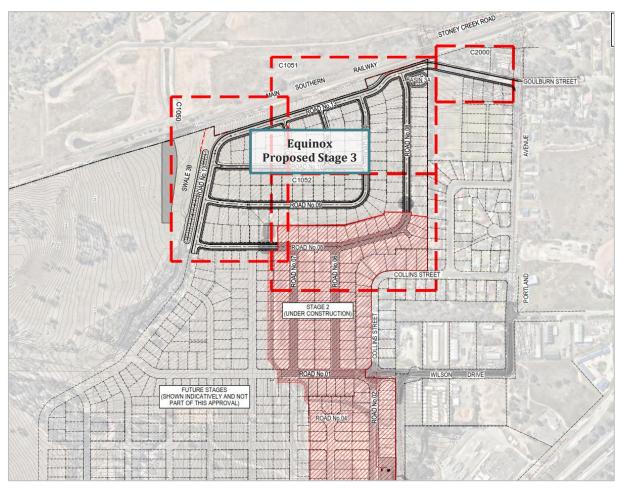


Figure 3: Extract from Stage 3 Subdivision Plan

## 3.1 External Access Points

As previously noted, approved Stages 1 and 2 will be accessed via one of two (2) accesses onto the external road network, via the extension of Wilson Road to the west (the primary access) and the extension of Collins Street.

Under Stage 3, it is proposed that Goulburn Street be extended to the west via the existing road corridor to the site, to provide a third access point to the subdivision. This additional (northern) access point is identified in **Figure 3.1** below. Whilst not required from a traffic capacity perspective, this additional access point will distribute traffic volumes to/from the ultimate subdivision, minimising any impact of traffic generated by the proposed development upon the local road network.

The Goulburn Street road corridor connecting from Portland Avenue to the subject site has a width of 20m. As shown in the plans of the proposal included as **Appendix B**, the proposed extension of Goulburn Street to the north-eastern corner of the subject site within this existing 20m wide road corridor will comprise an 11m wide carriageway with kerb and channel, a 2.5m wide footpath along the northern side of the road, and a 1.2m wide footpath along the southern side of the road.

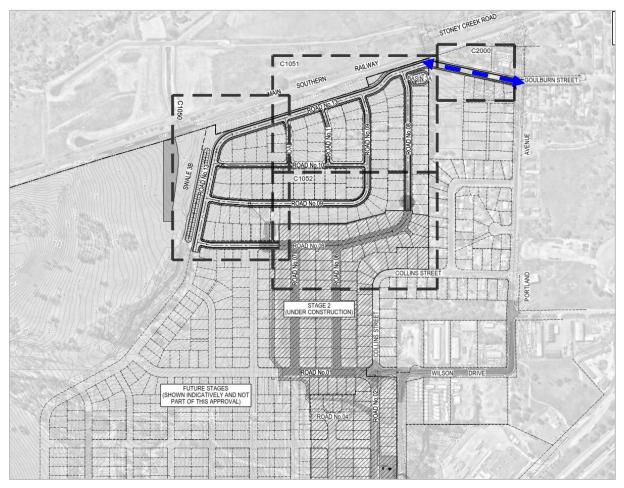


Figure 3.1: Northern External Access Point to Subdivision (Stage 3) - Goulburn Street

# 3.2 Internal Road Network

The Stage 3 internal road network comprises roads which are 18m wide (i.e. 9m pavement width and 4.5m wide verges) and roads which are 15m wide (i.e. 6m pavement width and 4.5m wide verges), as shown in the plans included as **Appendix B**.

These proposed road widths are consistent with those for 'Local Streets' and 'Access Streets' as stipulated in Table D.1.5A of Council's Design Specifications – Standards for Engineering Works.

No temporary turnarounds will be required for Stage 3, as there will be no terminating roads under this stage of the development.

Given Council's Design Specifications – Standards for Engineering Works does not provide indicative traffic volumes for the various classifications of roads, reference has been made to the Australian Model Code for Residential Development (AMCORD). A summary of the relevant information from this document is provided in **Table 3.2** following.

Table 3.2: Summary of Key Characteristics of Street Types (AMCORD)

	1	Proposed					
Street Type	Indicative Max Traffic Volume Range (vpd)	Carriageway Width (m)	Verge Width (m)	Minimum Road Reserve Width (m)	Carriageway Width (m)	Verge Width (m)	Road Reserve Width (m)
Access Street	0 - 300	5.0	3.5	12.0			
Access Street	300 - 1000	5.0 – 5.5	4.0	13.0	6.0	4.5	15.0
Access Street	1000 - 2000	5.5 or 7.0	4.0	13.5			
Minor Collector	1000 - 3000	7.0 – 7.5	4.5	16.5	9.0	4.5	18.0

Source: Australian Model Code for Residential Development (AMCORD)

An assessment of the forecast traffic generation of the subdivision against the volume thresholds outlined in the table above has been undertaken, with the results discussed in the following section.

## 3.3 Traffic Generation and Distribution

The proposed development is a low-density residential subdivision, which is a land use typically recognised as a low traffic generator.

The RMS' Guide to Traffic Generating Developments Updated Traffic Surveys (Technical Direction TDT 2013/04a) recommends the following trip generation rates for low-density residential dwellings in regional areas:

• Daily: 7.4 trips per dwelling

AM Peak Hour: 0.71 trips per dwelling
PM Peak Hour: 0.78 trips per dwelling

The application of the above trip generation rates leads to the forecast traffic volumes as outlined in **Table 3.3** below.

Table 3.3: Forecast Traffic Generation

	RMS Trip Generation Rate	APPROVED Stages 1 and 2 (148 lots)	PROPOSED Stage 3 (125 lots)	TOTAL Stages 1 -3 (273 lots)
Daily	7.4 trips per dwelling	1095 vpd	925 vpd	2020 vpd
AM Peak Hour	0.71 trips per dwelling	105 vph	89 vph	194 vph
PM Peak Hour	0.78 trips per dwelling	115 vph	98 vph	213 vph

Based upon the information in the table above, following the completion of Stage 3 of the subdivision (total 273 residential lots), it is forecast that the subdivision will generate in the order of **2,020 vehicle trips per day**. Once distributed onto the internal road network, these volumes will be well within the AMCORD thresholds for Access Streets and Minor Collectors as outlined in **Table 3.2** above.

The <u>ultimate subdivision</u> (which will be the subject of future development applications) is expected to comprise in the order 525 residential lots, and is therefore expected to generate in the order of **3,885 vehicle trips per day**. Once distributed onto the internal road network, it is anticipated that these volumes will be within the AMCORD thresholds for Access Streets and Minor Collectors as outlined in **Table 3.2** above.

In light of the above, the road hierarchy and road widths proposed to be delivered under Stage 3 of the development are expected to be more than adequate to cater for the traffic volumes generated by the ultimate subdivision (which will be the subject of future development applications).

Consideration has also been given to peak hour traffic impacts, consistent with standard practice. The peak hour traffic volumes in the table above are relatively low, and once distributed onto the surrounding road network, these additional traffic volumes are not expected to have any notable impact upon the performance of these roads. Notwithstanding this, in order to determine the actual increase in traffic volumes on the surrounding roads, the following distribution assumptions have been applied:

- 65% of trips generated by the subdivision will be regional trips (i.e. accessing the Hume Highway), and 35% will be local trips (i.e. not accessing the Hume Highway). This is consistent with the assumptions outlined in the Traffic Assessments prepared in support of the rezoning application;
- Of the local trips, 80% will have an origin / destination within the Marulan Town Centre area (e.g. Marulan Public School, the local supermarket), and 20% will have an origin / destination to the northwest on Brayton Road (e.g. local places of industrial employment).
- Of the regional trips, 60% will access/egress the Hume Highway using the southern intersection at Medway Road, and 40% will access/egress the Hume Highway using the northern interchange at Brayton Road;
- 60% of traffic accessing the subdivision will do so using Wilson Drive, 30% will use the new Goulburn Street extension, and 10% will use Collins Street;
- For the arrival / departure splits, consistent with standard practice:
  - At a daily level, a distribution of 50% arrivals and 50% departures has been assumed;
  - During the AM peak, a distribution of 30% arrivals and 70% departures has been assumed; and
  - During the PM peak, a distribution of 70% arrivals and 30% departures has been assumed.

A spreadsheet model has been developed using the above assumptions to forecast the additional traffic volumes expected to be generated by Stage 3 of the subdivision on the local road network, with the figures included as **Appendix C** showing these forecast traffic volumes (both Stage 3 in isolation, and Stages 1 – 3 in total). The expected impacts of these additional traffic volumes are discussed in the following section.



# 3.4 Traffic Impact of Proposal

As shown in the figures included in **Appendix C**, once distributed onto the surrounding road network, the additional traffic volumes expected to be generated by Stages 1 - 3 of the subdivision are low, and therefore not expected to have any notable impact upon the performance of the road network. Specifically:

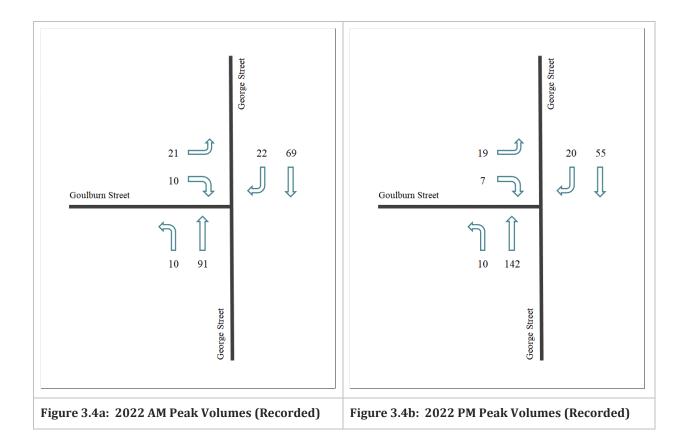
- At the Hume Highway Northern Interchange (with Brayton Road), the forecast additional traffic volumes are less than 60vph in the peak hours, or approximately one (1) additional movement per minute, on average, during the peak hours. Given this is a high-standard and high-capacity interchange, this additional traffic will have a negligible impact upon the performance of this interchange.
- At the Hume Highway Southern Intersection (with Medway Road), the forecast additional traffic volumes
  are 90vph or less in the peak hours. Given the movements at this intersection to/from the Hume
  Highway are limited to left-in and left-out only, this additional traffic will have a negligible impact upon
  the performance of this intersection.
- As shown in the figures included in **Appendix C**, the forecast additional traffic volumes on the local road network (including Portland Avenue, Medway Road, George Street, Goulburn Street and Brayton Road) are generally low, and would have a limited impact upon these roads or the intersections of these roads from a capacity perspective.

Importantly, it should be noted that the proposed development is consistent with the rezoning of the subject site which occurred with the gazettal of the *Goulburn Mulwaree Local Environmental Plan 2009*, and it is therefore assumed that the impact of the proposed development upon the local and State-controlled road network has been considered as part of the planning for the area.

Notwithstanding the above, and given the proposal includes an additional access point onto Goulburn Street, intersection capacity analyses have been undertaken of the George Street / Goulburn Street intersection with the additional traffic forecast to be generated by the proposed development.

For the purpose of this assessment, traffic counts were undertaken at this intersection on Wednesday 25 May 2022. The detailed results of these counts are included as **Appendix D**, with the figures below showing the recorded turning movement volumes during the morning and afternoon peak periods, which occurred from 8 – 9am and from 3 – 4pm respectively.

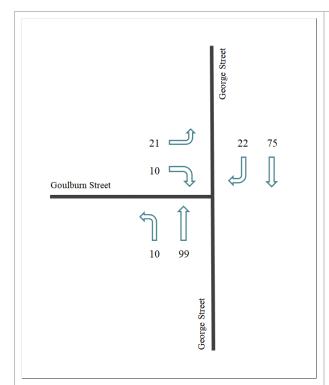
As shown in these figures, the recorded turning movement volumes at this intersection were very low.



For the purpose of this assessment, it has been assumed that Stage 3 of the subdivision (in addition to approved Stages 1 and 2) will be completed and dwellings constructed on all lots by 2025. This has therefore been assessed as the year of opening, along with the 10-year design horizon (i.e. 2035).

In order to forecast future traffic volumes at the George Street / Goulburn Street intersection, it has been assumed that through traffic on George Street will grow at a rate of 3% per annum compound.

The resulting forecast future year traffic volumes are as shown in the figures below.



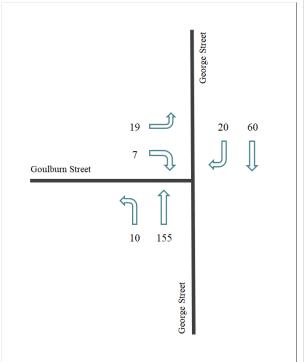
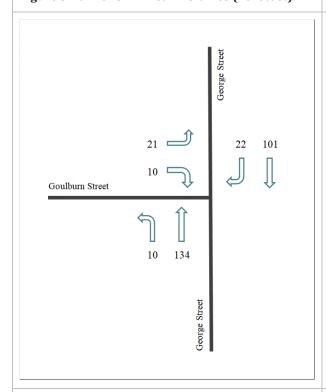


Figure 3.4c: 2025 AM Peak Volumes (Forecast)

Figure 3.4d: 2025 PM Peak Volumes (Forecast)



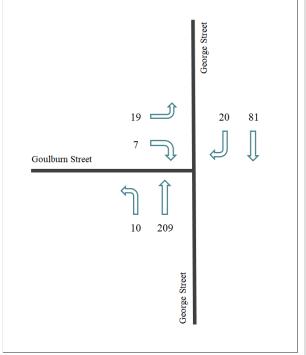
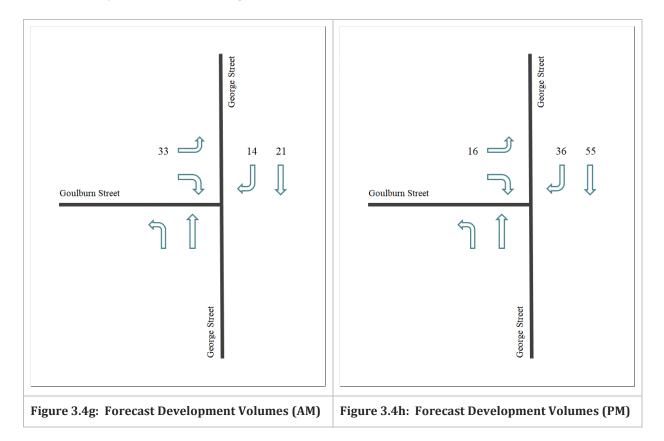


Figure 3.4e: 2035 AM Peak Volumes (Forecast)

Figure 3.4f: 2035 PM Peak Volumes (Forecast)

Based upon the aforementioned assumptions, the forecast additional turning movement volumes at the George Street / Goulburn Street intersection as a result of the proposed development (Stages 1 – 3 inclusive i.e. 273 residential lots) are as shown in the figures below.



Intersection modelling has been undertaken using SIDRA, to assess the impact of the forecast additional turning movement volumes at the George Street / Goulburn Street intersection upon the operation of the intersection, from a capacity perspective.

The model produces a range of outputs, including Degree of Saturation (DOS) and Average Vehicle Delay per vehicle (AVD). The AVD is in turn related to a level of service (LOS) criteria. These performance measures can be interpreted using the following explanations:

- **Degree of Saturation (DOS):** a measure of the operational performance of intersections. For intersections controlled by give way/stop control, satisfactory intersection operation is generally indicated by a DOS of 0.8 or less.
- Average Vehicle Delay (AVD): a measure of the operational performance of an intersection or
  movement. In general, levels of acceptability of AVD depend on the time of day (motorists generally
  accept higher delays during peak commuter periods) and the road system being modelled (motorists are
  more likely to accept longer delays on side streets than on the main road system).
- **Level of Service (LOS):** a comparative measure which provides an indication of the operating performance of an intersection or movement based upon the thresholds below:

Table 3.4.2a: Level of Service Criteria (Give Way and Stop Controlled Intersections)

Level of Service	Average Delay per Vehicle (seconds)	Description
A	Less than 14	Good operation
В	15 - 28	Acceptable delays and spare capacity
С	29 - 42	Satisfactory, but accident study required
D	43 - 56	Near capacity and accident study required
Е	57 – 70	At capacity, requires other control mode
F	More than 70	Unsatisfactory, requires other control mode

The results of the intersection analyses of the George Street / Goulburn Street intersection with the traffic expected to be generated by Stages 1 – 3 of the development are summarised following:

Table 3.2.4b: Summary of Results of Intersection Analyses - George Street / Goulburn Street Intersection

Design Year	Peak Hour	Design Scenario	Intersection Degree of Saturation	Critical Movement Average Delay (secs)	Critical Movement Level of Service	95 <sup>th</sup> Percentile Queue (m)
	AM Peak Hour	Base	0.060	6.2	LOS A	1.0
2025		With Development	0.078	6.4	LOS A	1.7
2023	PM Peak Hour	Base	0.091	6.4	LOS A	1.0
	РМ Реак ноиг	With Development	0.103	6.9	LOS A	2.7
	AM Peak Hour	Base	0.080	6.5	LOS A	1.1
2005		With Development	0.093	6.7	LOS A	1.8
2035	D14 D 1 11	Base	0.121	6.8	LOS A	1.1
	PM Peak Hour	With Development	0.121	7.3	LOS A	2.9

The results of the analyses reveal that the George Street / Goulburn Street intersection is predicted to operate well within acceptable limits from a capacity perspective with the traffic expected to be generated by Stages 1-3 of the subdivision (273 residential lots), with significant spare capacity. Furthermore, the traffic generated by the proposed development will have a marginal impact upon the performance of the intersection, in terms of queuing and delays.

In summary, it is concluded that no upgrades are required to the external road network to support the proposed Stage 3 development, from a capacity perspective (with the exception of the proposed extension of Goulburn Street to the site, providing a third access point to the subdivision).

It should be noted that future stages of the development (beyond Stage 3) would be subject to future development applications, at which time Council would have the opportunity to assess the impacts of any additional development yield. It is anticipated however that the subject intersections on the local road network will have more than adequate capacity to accommodate the traffic generated by the ultimate development.

Importantly, the Stage 3 development would not preclude or inhibit the provision of any external road upgrades which may be warranted to support the ultimate development.



# 4.0 Recommendations

In light of the information contained within this report, it is considered that the proposal is satisfactory from a traffic operations perspective, and it is recommended that the development application be approved from a traffic engineering perspective.

# 4.1 Qualifications

This report has been prepared and/or approved by:

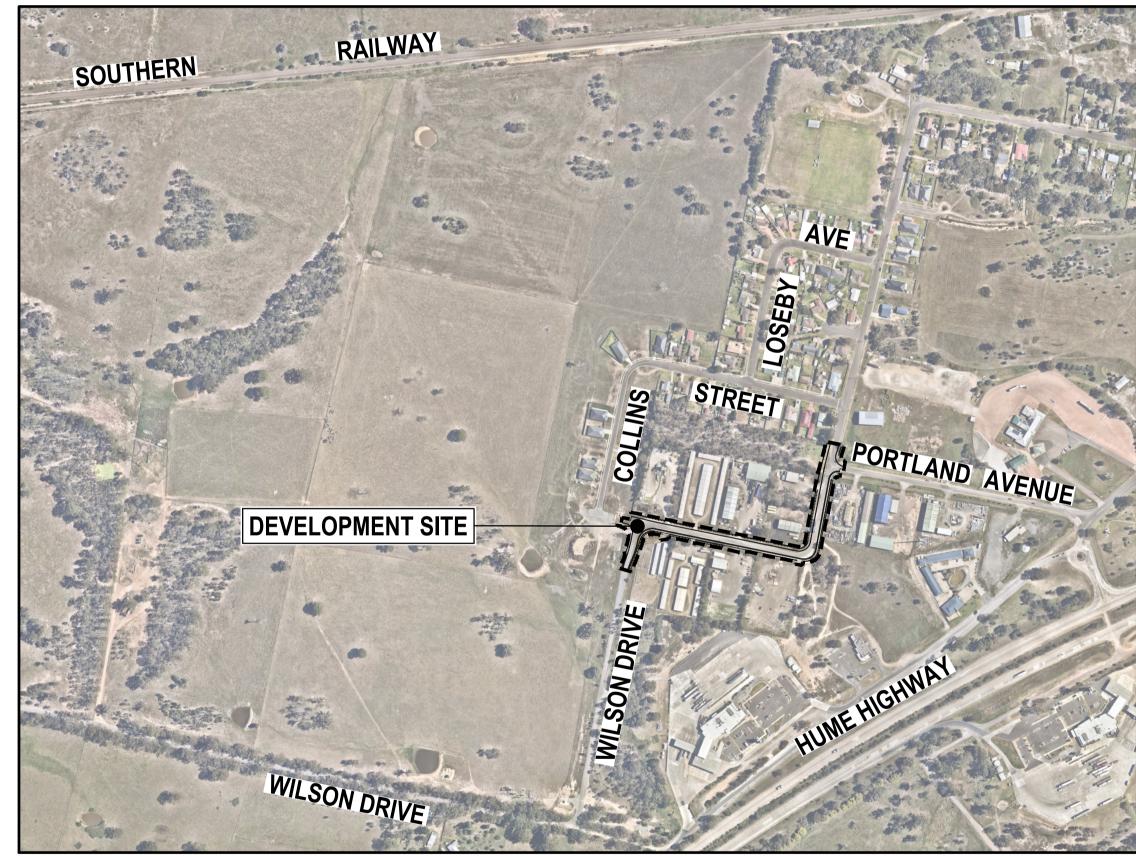
### **Anne Coutts**

Director, InRoads Group
BECivil | CPEng | RPEQ | NER | FIEAust | MAITPM

# **APPENDIX A**

Stage 2 Upgrade Works (Wilson Drive) – Currently Underway

# WILSON DRIVE, MARULAN CIVIL WORKS

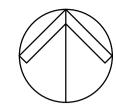


**LOCALITY PLAN** 

**NOT TO SCALE** 

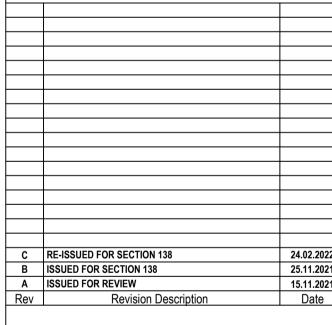
SOURCE: NEARMAP

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DRAWING SCHEDULE			
Sheet Number	Sheet Title	REV.	
C-00-0001	COVER SHEET LOCALITY PLAN AND INDEX SHEET	С	
C-00-0101	GENERAL NOTES AND SPECIFICATIONS	С	
C-00-0111	LEGEND	С	
C-00-0201	KEY PLAN	С	
C-01-0001	BULK EARTHWORKS PLAN	С	
C-01-1001	CUT FILL PLAN	С	
C-02-0001	SITEWORKS DRAINAGE PLAN SHEET 1	С	
C-02-0002	SITEWORKS DRAINAGE PLAN SHEET 2	С	
C-02-0003	SITEWORKS DRAINAGE PLAN SHEET 3	С	
C-02-2001	PAVEMENT PLAN	С	
C-02-6001	SITEWORKS DETAILS	С	
C-03-5001	STORMWATER LONGITUDINAL SECTIONS SHEET 1	В	
C-03-5002	STORMWATER LONGITUDINAL SECTIONS SHEET 2	В	
C-03-6001	STORMWATER DRAINAGE DETAILS SHEET 1	С	
C-03-6002	STORMWATER DRAINAGE DETAILS SHEET 2	С	
C-03-6003	STORMWATER DRAINAGE DETAILS SHEET 3	В	
C-04-0001	ROAD SETOUT PLAN	С	
C-04-3001	WILSON DRIVE 01 & 02 LONGITUDINAL SECTIONS	С	
C-04-4001	WILSON DRIVE (01) CROSS SECTIONS SHEET 1	С	
C-04-4002	WILSON DRIVE (01) CROSS SECTIONS SHEET 2	С	
C-04-4004	WILSON DRIVE (01) CROSS SECTIONS SHEET 3	С	
C-04-4005	WILSON DRIVE (01) CROSS SECTIONS SHEET 4	С	
C-04-4006	WILSON DRIVE (01) CROSS SECTIONS SHEET 5	С	
C-04-4007	WILSON DRIVE (02) CROSS SECTIONS SHEET 6	С	
C-04-1001	ROAD KERB RETURNS - SHEET 1	В	
C-04-1002	ROAD KERB RETURNS - SHEET 2	В	
C-05-5001	SIGNAGE AND LINEMARKING PLAN	В	
C-06-0001	EROSION AND SEDIMENT CONTROL PLAN SHEET 1	С	
C-06-0002	EROSION AND SEDIMENT CONTROL PLAN SHEET 2	С	
C-06-0003	EROSION AND SEDIMENT CONTROL PLAN SHEET 3	С	
C-06-9001	EROSION AND SEDIMENT CONTROL DETAILS	С	





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FDC CONSTRUCTION

Project
WILSON DRIVE, MARULAN

Title
COVER SHEET LOCALITY PLAN AND
INDEX SHEET

Scale: AS SHOWN

Drawn Designed Checked Approved B.K./H.D.

Project Number Drawing Number Revision C

- ALL WORK IS TO BE UNDERTAKEN IN ACCORDANCE WITH GOULBURN MULWAREE COUNCIL CONSTRUCTION SPECIFICATIONS, THE DETAILS SHOWN ON THE DRAWINGS AND THE SPECIFICATIONS AND THE DIRECTIONS OF THE PRINCIPAL'S REPRESENTATIVE.
- WHERE NEW WORKS ABUT EXISTING THE CONTRACTOR SHALL ENSURE THAT A SMOOTH EVEN PROFILE, FREE FROM ABRUPT CHANGES IS OBTAINED.
- THE CONTRACTOR SHALL ARRANGE ALL SURVEY SETOUT TO BE CARRIED OUT BY A REGISTERED SURVEYOR.
- CARE IS TO BE TAKEN WHEN EXCAVATING NEAR EXISTING SERVICES. NO MECHANICAL EXCAVATIONS ARE TO BE UNDERTAKEN OVER COMMUNICATIONS OR ELECTRICAL SERVICES. HAND EXCAVATE IN THESE AREAS.
- ALL SERVICE TRENCHES UNDER VEHICULAR PAVEMENTS SHALL BE BACKFILLED WITH AN APPROVED GRANULAR MATERIAL AND COMPACTED TO 98% STANDARD MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS.1289.5.1.1.
- ALL TRENCH BACKFILL MATERIAL NOT IN PAVEMENTS SHALL BE COMPACTED TO THE SAME DENSITY AS THE ADJACENT MATERIAL.
- ON COMPLETION OF PIPE INSTALLATION ALL DISTURBED AREAS MUST BE RESTORED TO ORIGINAL. INCLUDING KERBS, FOOTPATHS, CONCRETE AREAS, GRAVEL AND GRASSED AREAS AND ROAD PAVEMENTS.
- PROVIDE 10mm WIDE EXPANDING CORK JOINTS BETWEEN CONCRETE PAVEMENTS AND ALL BUILDINGS, WALLS, FOOTINGS, COLUMNS, KERBS, DISHDRAINS, GRATED DRAINS, BOLLARD FOOTINGS ETC
- 11. CONTRACTOR TO OBTAIN ALL AUTHORITY APPROVALS.
- 12. ALL BATTERS TO BE GRASSED LINED IN ACCORDANCE WITH GOULBURN MULWAREE COUNCIL CONSTRUCTION SPECIFICATIONS AND LANDSCAPE ARCHITECTS SPECIFICATION.
- 13. MAKE SMOOTH TRANSITION TO EXISTING SERVICES AND MAKE GOOD.
- 14. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY DIVERSION DRAINS AND MOUNDS TO ENSURE THAT AT ALL TIMES EXPOSED SURFACES ARE FREE DRAINING AND WHERE NECESSARY EXCAVATE SUMPS AND PROVIDE PUMPING EQUIPMENT TO DRAIN EXPOSED AREAS.
- 15. THESE PLANS SHALL BE READ IN CONJUNCTION WITH GOULBURN MULWAREE COUNCIL CONSTRUCTION SPECIFICATIONS AND APPROVED LANDSCAPE, ELECTRICAL AND TELECOMMUNICATIONH DRAWINGS AND SPECIFICATIONS.
- 16. TRENCHES THROUGH EXISTING ROAD AND CONCRETE PAVEMENTS SHALL BE SAWCUT TO FULL DEPTH OF CONCRETE AND A MIN 50mm IN BITUMINOUS PAVING.
- 17. ON COMPLETION OF WORKS ALL DISTURBED AREAS MUST BE RESTORED TO ORIGINAL INCLUDING, BUT NOT LIMITED TO, KERBS, FOOTPATHS, CONCRETE AREAS, GRASS AND LANDSCAPED AREAS.

# **EXISTING SERVICES AND FEATURES**

- EXISTING SERVICES HAVE BEEN PLOTTED FROM SUPPLIED DATA AND AS SUCH THEIR ACCURACY CANNOT BE GUARANTEED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK. ANY DISCREPANCIES SHALL BE REPORTED TO THE SUPERINTENDENT.
- THE CONTRACTOR SHALL ALLOW FOR THE CAPPING OFF, EXCAVATION, REMOVAL AND DISPOSAL IF REQUIRED OF ALL EXISTING SERVICES IN AREAS AFFECTED BY WORKS WITHIN THE CONTRACT AREA, AS SHOWN ON THE DRAWINGS UNLESS DIRECTED OTHERWISE BY THE
- THE CONTRACTOR SHALL ENSURE THAT AT ALL TIMES SERVICES TO ALL BUILDINGS NOT AFFECTED BY THE WORKS ARE NOT DISRUPTED.
- PRIOR TO COMMENCEMENT OF ANY WORKS THE CONTRACTOR SHALL GAIN WRITTEN APPROVAL OF HIS PROGRAMME FOR THE RELOCATION/CONSTRUCTION OF TEMPORARY
- EXISTING BUILDINGS, EXTERNAL STRUCTURES, AND TREES SHOWN ON THESE DRAWINGS ARE FEATURES EXISTING PRIOR TO ANY DEMOLITION WORKS.
- CONTRACTOR SHALL CONSTRUCT TEMPORARY SERVICES TO MAINTAIN EXISTING SUPPLY TO BUILDINGS REMAINING IN OPERATION DURING WORKS TO THE SATISFACTION AND APPROVAL OF THE SUPERINTENDENT. ONCE DIVERSION IS COMPLETE AND COMMISSIONED THE CONTRACTOR SHALL REMOVE ALL SUCH TEMPORARY SERVICES AND MAKE GOOD TO THE SATISFACTION OF THE SUPERINTENDENT AND THE RELEVANT SERVICE AUTHORITY.
- INTERRUPTION TO SUPPLY OF EXISTING SERVICES SHALL BE DONE SO AS NOT TO CAUSE ANY INCONVENIENCE TO THE PRINCIPAL. CONTRACTOR TO GAIN APPROVAL OF SUPERINTENDENT FOR TIME OF INTERRUPTION. THE CONTRACTOR IS RESPONSIBLE FOR ALL
- CLEARANCE AND COVER REQUIREMENTS SHALL BE OBTAINED FROM THE GOULBURN MULWAREE COUNCIL AND RELEVANT SERVICE AUTHORITY BEFORE COMMENCEMENT OF WORKS AND SHALL BE ADHERED TO AT ALL TIMES.
- CARE IS TO BE TAKEN WHEN EXCAVATING NEAR EXISTING SERVICES. NO MECHANICAL EXCAVATIONS ARE TO BE UNDERTAKEN OVER TELECOM OR ELECTRICAL SERVICES. HAND EXCAVATE IN THESE AREAS ONLY.

# **ROADWORKS NOTES**

- ALL BASECOURSE AND SUB-BASECOURSE MATERIALS SHALL CONFORM WITH GOULBURN MULWAREE COUNCIL CONSTRUCTION SPECIFICATIONS AND AUSTRALIAN STANDARDS.
- ALL BASECOURSE AND SUB-BASE MATERIALS SHALL BE COMPACTED TO ACHIEVE A MINIMUM OF 100% STANDARD MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT OF +OR- 2% IN ACCORDANCE WITH AS1289 E1.1.
- ALL WEARING SURFACES SHALL BE ASPHALTIC CONCRETE LAID TO THE THICKNESS SPECIFIED AND IN ACCORDANCE WITH THE SPECIFICATION.
- CONCRETE FOR KERB SHALL HAVE A CONCRETE STRENGTH OF 20MPA AT 28 DAYS, MINIMUM SLUMP OF 60MM AND MAXIMUM AGGREGATE SIZE OF 40MM.

# STORMWATER NOTES

- 1. ALL DRAINAGE PIPES GREATER THAN Ø300mm SHALL BE CLASS 2 APPROVED SPIGOT AND SOCKET REINFORCED CONCRETE PIPES WITH RUBBER RING JOINTS (UNO).
- WHERE DRAINAGE LINE PASS UNDER VEHICULAR PAVEMENTS PIPES SHALL BE CLASS 4 APPROVED SPIGOT AND SOCKET REINFORCED CONCRETE PIPES WITH RUBBER RING JOINTS
- 3. ALL DRAINAGE PIPES LESS THAN OR EQUAL TO Ø300mm SHALL BE uPVC DWV GRADE CLASS SN8 IN ACCORDANCE WITH AS/NZS1260:2009-PVC-U PIPES AND FITTINGS FOR DRAIN, WASTE AND VENT APPLICATION WITH SOLVENT WELDED JOINTS.
- EQUIVALENT STRENGTH REINFORCED CONCRETE OR FIBROUS REINFORCED CONCRETE MAY BE USED SUBJECT TO APPROVAL BY THE SUPERINTENDENT.
- PIPES FOR SUB-SOIL DRAINS SHALL BE SLOTTED 100MM DIAMETER CLASS 1000 WRAPPED IN GEOFABRIC, UNO, COMPLYING WITH THE REQUIREMENTS OF AS 2439
- ALL PIPE JUNCTIONS UP TO AND INCLUDING 300 DIA. AND TAPERS SHALL BE VIA PURPOSE MADE FITTINGS.
- 7. ALL MILD STEEL FIXTURES INCLUDING GRATES, FRAMES, STEP IRONS, LADDERS, ETC., SHALL BE HOT DIP GALVANISED. GALVANISING SHALL COMPLY WITH THE REQUIREMENTS OF AS 1214
- 8. MINIMUM GRADE TO STORMWATER LINES TO BE 1%. (U.N.O.)

OR AS 1650, AS APPROPRIATE.

- CONTRACTOR TO SUPPLY AND INSTALL ALL FITTINGS AND SPECIALS INCLUDING VARIOUS PIPE ADAPTORS TO ENSURE PROPER CONNECTION BETWEEN DISSIMILAR PIPEWORK.
- 10. ALL CONNECTIONS TO EXISTING DRAINAGE PITS SHALL BE MADE IN A TRADESMAN-LIKE MANNER AND THE INTERNAL WALL OF THE PIT AT THE POINT OF ENTRY SHALL BE CEMENT RENDERED TO ENSURE A SMOOTH FINISH.
- 11. PRECAST PITS SHALL NOT BE USED UNLESS WRITTEN APPROVAL IS OBTAINED FROM THE SUPERINTENDENT.
- 12. WHERE TRENCHES ARE IN ROCK, THE PIPE SHALL BE BEDDED ON A MIN. 50MM CONCRETE BED (OR 75MM THICK BED OF 12MM BLUE METAL) UNDER THE BARREL OF THE PIPE. THE PIPE COLLAR AT NO POINT SHALL BEAR ON THE ROCK. IN OTHER THAN ROCK, PIPES SHALL BE LAID ON A 75MM THICK SAND BED. IN ALL CASES BACKFILL THE TRENCH WITH SAND TO 200MM ABOVE THE PIPE. WHERE THE PIPE IS UNDER PAVEMENTS BACKFILL REMAINDER OF TRENCH WITH SAND OR APPROVED GRANULAR BACKFILL COMPACTED IN 150MM LAYERS TO 98% STANDARD MAX. DRY DENSITY.
- 13. BEDDING SHALL BE (U.N.O.) TYPE H1, IN ACCORDANCE WITH GOULBURN MULWAREE COUNCIL CONSTRUCTION SPECIFICATIONS AND CURRENT RELEVANT AUSTRALIAN STANDARDS.
- 14. BACKFILL TRENCH WITH SAND OR APPROVED GRANULAR BACKFILL TO 300mm(MIN) ABOVE THE PIPE. WHERE THE PIPE IS UNDER PAVEMENTS BACKFILL REMAINDER OF TRENCH TO PAVEMENT SUBGRADE WITH SAND OR APPROVED GRAVEL SUB-BASE COMPACTED IN 150mm LAYERS TO 98% STANDARD MAXIMUM DRY DENSITY. THE CONTRACTOR IS TO ENSURE COMPACTION EQUIPMENT IS APPROPRIATE FOR THE PIPE CLASS USED
- 15. WHERE STORMWATER LINES PASS UNDER FLOOR SLABS DWV GRADE uPVC RUBBER RING JOINTS ARE TO BE USED (UNO).
- 16. WHERE SUBSOIL DRAINAGE LINES PASS UNDER VEHICULAR PAVEMENTS, UNSLOTTED uPVC DWV GRADE CLASS SN8 PIPE SHALL BE USED.
- 17. 100mm DIA. SUBSOIL DRAINAGE PIPE 3m LONG WRAPPED IN FILTER SOCK TO BE PROVIDED IN PIPE TRENCHES UPSTREAM OF ALL PITS.
- 18. CONTRACTOR TO SUPPLY AND INSTALL ALL FITTINGS AND SPECIALS INCLUDING VARIOUS PIPE ADAPTORS TO ENSURE PROPER CONNECTION BETWEEN DISSIMILAR PIPEWORK.
- 19. PITS DEEPER THAN 1000mm SHALL HAVE ACCESS LADDERS OR STEP IRONS INSTALLED AND SHALL BE IN ACCORDANCE WITH THE LOCAL OR STATUTORY AUTHORITY REQUIREMENTS
- ALL FRAMES, COVERS AND GRATINGS FOR PITS, SUMPS, DRAINS, GRATED DRAINS ETC MUST BE PROVIDED TO SUIT CLASS D DUTIES AND ALL GRATES SHALL BE SLIP RESISTANT AND
- 21. WHERE A PIT IS IDENTIFIED AS A CONFINED SPACE, PIT COVERS SHALL BE PROVIDED WITH STANDARD CONFINED SPACE SIGNAGE
- 22. SUBSOIL DRAINAGE LINES SHALL BE INSTALLED AT THE BASE OF ALL RETAINING WALLS AND FOR ALL STORMWATER PITS. ALL SUBSOIL LINES SHALL BE CONNECTED TO DRAIN TO THE STORMWATER DRAINAGE SYSTEM
- 23. CAPPED FLUSHING POINTS MUST BE PROVIDED FOR ALL SUBSOIL AND SEEPAGE DRAINAGE SYSTEMS AT THE END OF EACH PIPE, AT 30M SPACING AND AT CHANGES IN DIRECTIONS
- 24. INSPECTION OPENINGS AND CLEAROUTS MUST BE PROVIDED AT EVERY JUNCTION, BEND, CHANGE OF DIRECTION AND AT THE BASE OF ALL DOWNPIPES IMMEDIATELY ABOVE WHERE THE DOWNPIPE PENETRATES THE GROUND OR SLAB ON GROUND
- 25. ALL SUBSOIL PIPES SHALL BE FACTORY SLOTTED HDPE, MINIMUM 100MM DIAMETER SN8 CLASS, SIMILAR OR EQUAL TO VINIDEX DRAINCOIL, CERTIFIED UPVC, IN ACCORDANCE WITH AS1260, AS2032 (PIPE) & AS3789 (JOINTING) INSTALLED ON GEOTEXTILE FABRIC WITH 150MM SURROUND OF 25MM BLUE METAL AGGREGATE. UNO

# SUBSOIL DRAINAGE NOTES

- DN100 SLOTTED uPVC SUBSOIL DRAINAGE WRAPPED IN GEOFABRIC SHALL BE PROVIDED IN THE FOLLOWING LOCATIONS AND CONNECTED TO THE SITE STORMWATER DRAINAGE SYSTEM (U.N.O):
  - UNDER KERBS ADJACENT TO ALL PAVEMENTS
  - AT THE BASE OF THE HIGH SIDE OF ALL RETAINING WALLS
- AT THE BASE OF RAINGARDENS

# **CONCRETE JOINTING NOTES**

# PEDESTRIAN FOOTPATH JOINTING:

- 1. DOWELED JOINTS ARE TO BE LOCATED WHERE POSSIBLE AT TANGENT POINTS OF CURVES AND ELSEWHERE AT MAX 6.0m CENTRES.
- TOOLED JOINTS ARE TO BE LOCATED AT A MAX 1.5 x WIDTH OF THE PAVEMENT.
- 3. WHERE POSSIBLE JOINTS SHOULD BE LOCATED TO MATCH KERBING AND/OR ADJACENT
- 4. ALL PEDESTRIAN FOOTPATH JOINTING LAYOUTS AS FOLLOWS (UNO)

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1.5 x	W		. 6	6.0m MAX	(			
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(1.5m MAX) OR EVERY THIRD JOINT ALL RAMPED CROSSINGS SHALL BE DOWELED INTO ADJOINING PATH PAVEMENT

# **EARTHWORKS NOTES**

- ALL WORK SHALL COMPLY WITH GOULBURN MULWAREE COUNCIL CONSTRUCTION SPECIFICATIONS AND AS3798 (2007) - GUIDELINES ON EARTHWORKS FOR COMMERCIAL AND RESIDENTIAL DEVELOPMENTS.
- ALL WORK SHALL COMPLY WITH THE PROJECT GEOTECHNICAL REPORT DOUGLAS PARTNERS - REPORT ON GEOTECHNICAL INVESTIGATION REF - 88505.03.R.001.Rev0. PSI

# **MARCH 2019**

- AFTER DEMOLITION STRIP REMAINING TOPSOIL THROUGHOUT TO EXPOSE NATURALLY OCCURRING AND ENGINEERING MATERIAL AND STOCKPILE ON SITE FOR REUSE AS DIRECTED BY THE SUPERINTENDENT.
- EXCAVATE TO SUBGRADE LEVELS, SEGREGATING AND STOCKPILING MATERIALS FOR LATER REUSE.
- PROOF ROLL ALL SOFT OR WET AREAS SHALL BE DRIED TO OPTIMUM MOISTURE AND RE-COMPACTED TO 95% MMDD, WHERE MATERIAL IS DEEMED UNSUITABLE BY THE SUPERINTENDENT AND CANNOT BE USED ON SITE SHALL REMOVED FROM SITE.
- ALL FILL MATERIAL SHALL BE FROM A SOURCE APPROVED BY THE SUPERINTENDENT AND SHALL COMPLY WITH THE FOLLOWING -
- a. FREE FROM ORGANIC AND PERISHABLE MATTER,
- b. MAXIMUM PARTICLE SIZE 75mm,
- c. PLASTICITY INDEX BETWEEN 2% AND 15%.
- ALL FILL MATERIAL SHALL BE PLACED IN MAXIMUM 200mm THICK LAYERS AND COMPACTED AT OPTIMUM MOISTURE CONTENT (+ OR - 2%) TO ACHIEVE A DRY DENSITY DETERMINED IN ACCORDANCE WITH AS1289.5.1.1-2003-METHODS OF TESTING SOILS FOR ENGINEERING PURPOSES OF NOT LESS THAN THE FOLLOWING STANDARD MINIMUM DRY DENSITY -

### LOCATION STANDARD DRY DENSITY (MMDD)

UNDER BUILDING SLABS	98%
VEHICULAR PAVED AREAS	100%
NON-VEHICULAR PAVED AREAS	98%
LANDSCAPED AREAS	95%

- THE CONTRACTOR SHALL PROGRAM THE EARTHWORKS OPERATION SO THAT THE WORKING AREAS ARE ADEQUATELY DRAINED DURING THE PERIOD OF CONSTRUCTION. THE SURFACE SHALL BE GRADED AND SEALED OFF TO REMOVE DEPRESSIONS, ROLLER MARKS AND SIMILAR WHICH WOULD ALLOW WATER TO POND AND PENETRATE THE UNDERLYING MATERIAL. ANY DAMAGE RESULTING FROM THE CONTRACTOR NOT OBSERVING THESE REQUIREMENTS SHALL BE RECTIFIED BY THE CONTRACTOR AT THEIR OWN EXPENSE.
- TESTING OF THE FILL MATERIAL SHALL BE CARRIED OUT BY AN APPROVED NATA REGISTERED LABORATORY AT THE CONTRACTORS EXPENSE.
- ROCK LEVELS SHOWN ON BULK EARTHWORKS PLANS AND SECTIONS ARE INFERRED. CONTRACTOR TO CONFIRM DEPTH ON SITE. TYP. INFERRED ROCK LEVELS BASED ON GEOTECHNICAL INVESTIGATION.
- PROPOSED BULK EARTHWORKS SURFACE LEVEL SHOWN DOES NOT INCLUDE THE LANDSCAPE TOPSOIL SETDOWNS.
- 12. EXISTING SURFACE LEVEL SHOWN DOES NOT INCLUDE STRIPPING.

# CONCRETE NOTES

# **GENERAL**

- ALL WORKMANSHIP AND MATERIALS SHALL COMPLY WITH GOULBURN MULWAREE COUNCIL CONSTRUCTION SPECIFICATIONS AND AS 3600 CURRENT EDITIONS WITH AMENDMENTS, AND THE ACSE CONCRETE SPECIFICATION EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- 2. VERIFY ALL SETTING OUT DIMENSIONS WITH THE ARCHITECT AND/OR THE SURVEYOR.
- 3. DO NOT OBTAIN DIMENSIONS BY SCALING THE DRAWINGS.
- IN CASE OF DOUBT ASK.

# CONCRETE

1. PLACE CONCRETE OF THE FOLLOWING CHARACTERISTIC COMPRESSIVE STRENGTH F"C AS DEFINED IN AS.3600 OR M.R. FORM 609. ADD WATER REDUCING ADMIXTURE EQUAL TO WRDA.

LOCATION	AS.3600 F'c MPa AT 28 DAYS	SPECIFIED SLUMP		NOMINAL AGG. SIZE
ALL KERB PITS ETC.	25	80	20	
VEHICULAR PAVEMENTS	32	80	20	

- USE "A.C.S.E. SPECIFICATION TYPE A" CEMENT.
- 3. ALL CONCRETE SHALL BE SUBJECT TO PROJECT CONTROL SAMPLE AND TESTING TO
- CONSOLIDATE BY VIBRATION

# REINFORCEMENT

- 1. FIX REINFORCEMENT AS SHOWN ON DRAWINGS. THE TYPE AND GRADE IS INDICATED BY A SYMBOL AS SHOWN BELOW. ON THE DRAWING N IS FOLLOWED BY A NUMERAL WHICH INDICATES THE SIZE IN MILLIMETRES. A MARK NUMERAL (IF USED) FOLLOWS THIS NUMERAL.
  - N. HOT ROLLED DEFORMED BAR, GRADE 410Y
  - S. HOT ROLLED DEFORMED BAR, GRADE 230S
  - R. PLAIN ROUND BAR, GRADE 230R SL. HARD DRAWN WIRE FABRIC.
- PROVIDE BAR SUPPORTS OR SPACERS TO GIVE THE FOLLOWING CONCRETE COVER TO ALL REINFORCEMENT UNLESS NOTED OTHERWISE.

# FOOTINGS - 75 BOTTOM, 65 TOP AND SIDES SLABS

- 20 TOP AND BOTTOM, 30 WHEN EXPOSED TO WEATHER. **BEAMS** 

- 50 BOTTOM AND SIDES (TO STIRRIPS) TOP COVER AS DETAILED COLUMNS - 40 TO TIES AND SPIRALS 50 WHEN EXPOSED TO WEATHER

- 25 GENERALLY 30 WHEN CAST IN FORMS BUT LATER EXPOSED TO WEATHER OR GROUND. 65 WHEN CAST DIRECTLY IN CONTACT WITH GROUND.

1. CURE ALL CONCRETE IN ACCORDANCE BY IMPERMEABLE MEMBRANE OR WATER CURING METHODS.

# LINEMARKING NOTES

- 1. THE WORK SHALL INCLUDE ALL LINE MARKING TO ROADS, HARDSTANDS, PATHS, CARPARKS AND THE TRAFFICABLE AREAS.
- 2. THE PAVEMENT MARKING AND PAINT SHALL BE IN ACCORDANCE WITH GOULBURN MULWAREE COUNCIL CONSTRUCTION SPECIFICATIONS AND AS 1742.2 AND THE RELEVANT LOCAL AND STATE AUTHORITY GUIDELINES.
- 3. PAINT SHALL BE TYPE 3, CLASS A AND THE COLOUR SHALL BE WHITE AND NOT SUBJECT TO DISCOLOURATION BY BITUMEN FROM THE ROAD SURFACE. EACH LINE SHALL BE 80mm WIDE. UNO IN LEGEND. ALL PAINT SHALL BE APPLIED BY MECHANICAL SPRAYER.
- 4. LINE MARKING SHALL BE SPOTTED OUT AND APPROVED PRIOR TO SPRAYING.
- PAINT SHALL BE APPLIED AT A WET THICKNESS OF BETWEEN 0.35mm TO 0.40mm.
- 6. ALL EXISTING PAVEMENT MARKING WHICH IS LOCATED ON EXISTING PAVEMENT TO BE RETAINED SHALL BE REMOVED BY GRINDING WHERE THE EXISTING MARKINGS ARE MADE REDUNDANT BY THE PROPOSED WORKS.
- 7. ALL PAVEMENT MARKING TO BE IN ACCORDANCE WITH THE CURRENT VERSION OF THE REGULATORY SIGNS MANUAL, AS1742.2, AS2890.1:2004 AND THE RELEVANT LOCAL AND STATE AUTHORITY GUIDELINES AND REQUIREMENTS.
- 8. TRANSITION LINEMARKING TO SUIT EXISTING WHERE REQUIRED
- 9. RELOCATE/ REMOVE EXISTING SIGNS AS REQUIRED.
- 10. REMOVE ALL REDUNDANT PAVEMENT MARKING AS REQUIRED.
- 11. PROVIDE ADEQUATE APPROACH WARNING SIGNS DURING AND AFTER CONSTRUCTION
- 13. ALL LINEMARKING TO BE WHITE IN COLOUR WITH THE EXCEPTION OF C2 AND C3 LINES AND LINEMARKING ON CONCRETE PAVEMENTS WHICH ARE TO BE YELLOW

12. PROVIDE RETRO-REFLECTIVE PAVEMENT MARKERS TO COUNCIL AND RMS REQUIREMENTS.

# **EROSION AND SEDIMENT CONTROL NOTES**

# SEDIMENT CONTROL INSTRUCTIONS

- 1. SEDIMENT FENCES WILL BE INSTALLED AS SHOWN ON THE PLAN AND ELSEWHERE AT THE DISCRETION OF THE SITE SUPERINTENDENT TO CONTAIN SOIL AS NEAR AS POSSIBLE TO THEIR SOURCE.
- 2. SEDIMENT FENCES WILL NOT HAVE CATCHMENT AREAS EXCEEDING 900 SQUARE METRES AND HAVE A STORAGE DEPTH OF AT LEAST 0.6 METRES.
- 3. SEDIMENT REMOVED FROM ANY TRAPPING DEVICES WILL BE RELOCATED WHERE FURTHER POLLUTION TO DOWNSLOPE LANDS AND WATERWAYS CANNOT OCCUR.
- 4. STOCKPILES ARE NOT TO BE LOCATED WITHIN 5 METRES OF HAZARD AREAS INCLUDING AREAS OF HIGH VELOCITY FLOWS SUCH AS WATERWAYS. PAVED AREAS AND DRIVEWAYS.
- WATER WILL BE PREVENTED FROM DIRECTLY ENTERING THE PERMANENT DRAINAGE SYSTEM UNLESS THE CATCHMENT AREA HAS BEEN PERMANENTLY LANDSCAPED AND/OR WATER HAS BEEN TREATED BY AN APPROVED DEVICE. 6. TEMPORARY SEDIMENT TRAPS WILL REMAIN IN PLACE UNTIL AFTER THE LANDS THEY ARE PROTECTING ARE COMPLETELY REHABILITATED.
- ACCESS TO SITES SHOULD BE STABILISED TO REDUCE THE LIKELIHOOD OF VEHICLES TRACKING SOIL MATERIALS ONTO PUBLIC ROADS AND ENSURE ALL-WEATHER ENTRY/EXIT.

# SOIL EROSION CONTROL INSTRUCTIONS

- EARTH BATTERS WILL BE CONSTRUCTED WITH AS LOW A GRADIENT AS PRACTICABLE BUT NO
- STEEPER, UNLESS OTHERWISE NOTED, THAN:
- a. 2(H):1(V) WHERE SLOPE LENGTH LESS THAN 12 METRES
- b. 2.5(H):1(V) WHERE SLOPE LENGTH BETWEEN 12 AND 16 METRES. c. 3(H):1(V) WHERE SLOPE LENGTH BETWEEN 16 AND 20 METRES.

d. 4(H):1(V) WHERE SLOPE LENGTH GREATER THAN 20 METRES.

- 2. ALL WATERWAYS, DRAINS, SPILLWAYS AND THEIR OUTLETS WILL BE CONSTRUCTED TO BE STABLE IN AT LEAST THE 1:20 YEAR ARI, TIME OF CONCENTRATION STORM EVENT.
- 3. WATERWAYS AND OTHER AREAS SUBJECT TO CONCENTRATED FLOWS AFTER CONSTRUCTION ARE TO HAVE A MAXIMUM GROUNDCOVER C-FACTOR OF 0.05 (70% GROUND COVER) WITHIN 10 WORKING DAYS FROM COMPLETION OF FORMATION. FLOW VELOCITIES ARE TO BE LIMITED TO THOSE SHOWN IN TABLE 5-1 OF "MANAGING URBAN STORMWATER -SOILS AND CONSTRUCTION", DEPT OF HOUSING 1998 (BLUE BOOK). FOOT AND VEHICULAR TRAFFIC WILL BE PROHIBITED IN THESE AREAS.
- 4. STOCKPILES AFTER CONSTRUCTION ARE TO HAVE A MAXIMUM GROUND-COVER C-FACTOR OF 0.1 (60% GROUND-COVER) WITHIN 10 WORKING DAYS FROM COMPLETION OF FORMATION.
- 5. ALL LANDS, INCLUDING WATERWAYS AND STOCKPILES, DURING CONSTRUCTION ARE TO HAVE A MAXIMUM GROUND-COVER C-FACTOR OF 0.15 (50% GROUND COVER) WITHIN 20 WORKING DAYS FROM INACTIVITY EVEN THOUGH WORKS MAY CONTINUE LATER.
- 6. FOR AREAS OF SHEET FLOW USE THE FOLLOWING GROUND COVER PLANT SPECIES FOR

TEMPORARY COVER: JAPANESE MILLET 20 KG/HA AND OATS 20 KG/HA.

- 7. PERMANENT REHABILITATION OF LANDS AFTER CONSTRUCTION WILL ACHIEVE A GROUND-COVER C-FACTOR OF LESS THAN 0.1 AND LESS THAN 0.05 WITHIN 60 DAYS, NEWLY PLANTED LANDS WILL BE WATERED REGULARLY UNTIL AN EFFECTIVE COVER IS ESTABLISHED AND PLANTS ARE GROWING VIGOROUSLY. FOLLOW-UP SEED AND FERTILISER
- 8. RE-VEGETATION SHOULD BE AIMED AT RE-ESTABLISHING NATURAL SPECIES. NATURAL SURFACE SOILS SHOULD BE REPLACED AND NON-PERSISTANT ANNUAL COVER CROPS SHOULD BE USED.

# WASTE CONTROL INSTRUCTIONS

HAVE CONTAINMENT BUNDS.

WILL BE APPLIED AS NECESSARY.

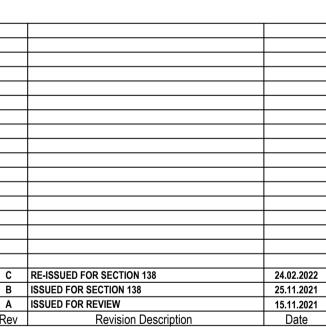
- ACCEPTABLE BINS WILL BE PROVIDED FOR ANY CONCRETE AND MORTAR SLURRIES, PAINTS ACID WASHING, LIGHTWEIGHT WASTE MATERIALS AND LITTER. CLEARANCE SERVICES WILL BE PROVIDED AT LEAST WEEKLY. DISPOSAL OF WASTE WILL BE IN A MANNER APPROVED BT THE SITE SUPERINTENDENT.
- 2. ALL POSSIBLE POLLUTANT MATERIALS ARE TO BE STORED WELL CLEAR OF ANY POORLY DRAINED AREAS, FLOOD PRONE AREAS, STREAMBANKS, CHANNELS AND STORMWATER DRAINAGE AREAS. STORE SUCH MATERIALS IN A DESIGNATED AREA UNDER COVER WHERE POSSIBLE AND WITHIN CONTAINMENT BUNDS.
- 3. ALL SITE STAFF AND SUB-CONTACTORS ARE TO BE INFORMED OF THEIR OBLIGATION TO USE WASTE CONTROL FACILITIES PROVIDED.
- IS NOT POLLUTED BY SEDIMENT, TOXIC MATERIALS OR PETROLEUM PRODUCTS. 5. PROVIDE DESIGNATED VEHICULAR WASHDOWN AND MAINTENANCE AREAS WHICH ARE TO

4. 4. ANY DE-WATERING ACTIVITIES ARE TO BE CLOSELY MONITORED TO ENSURE THAT WATER

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| WILSON DRIVE, MARULAN

GENERAL NOTES AND SPECIFICATIONS

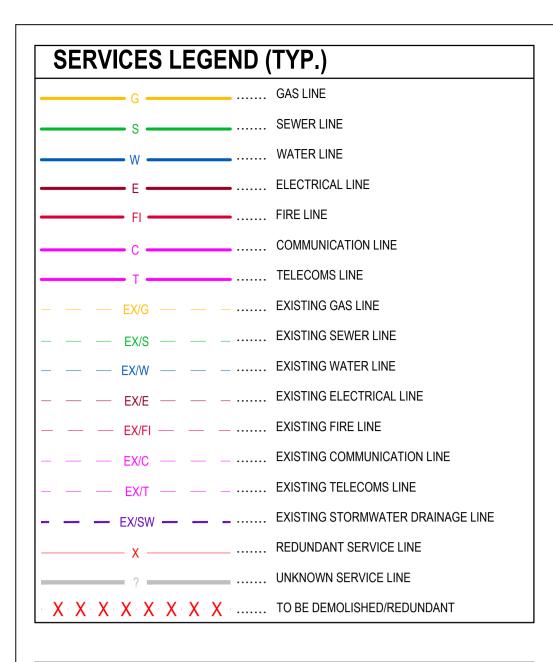
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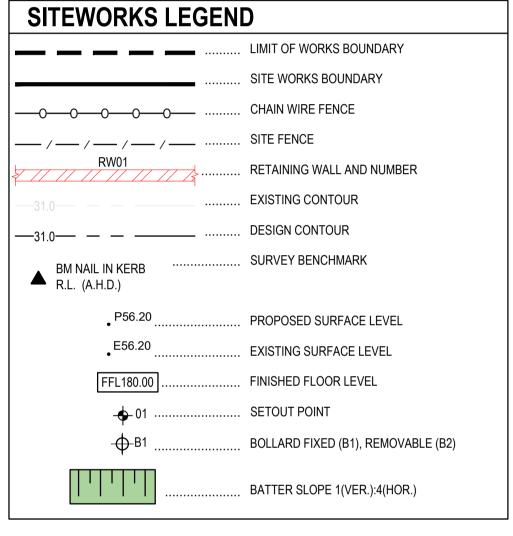
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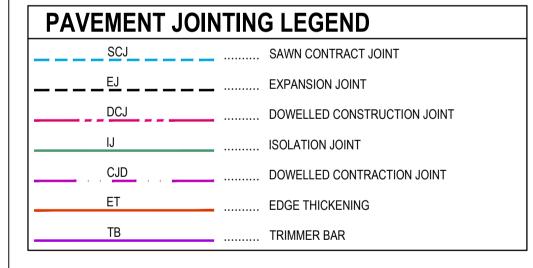
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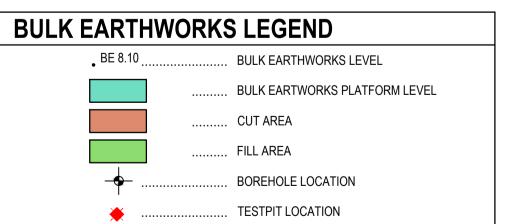
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SIGNAGE LEGEND	
	SIGN POST
	STREET SIGN POST

STORMWATER DRAI	NAGE I EGEND
sw	STORMWATER DRAINAGE LINE
SW	STORMWATER DRAINAGE (TEMPORARY)
SSD — CO	SUBSOIL DRAINAGE LINE WITH CLEAR OUT
	STORMWATER OVERLAND FLOWPATH
>>>	
	GRATED PITS (VARIES IN SIZES)
	JUNCTION PITS (VARIES IN SIZES)
	KERB INLET PITS (VARIES IN TYPE & SIZES)
	GRATED DRAIN (VARIES IN TYPE & SIZES)
	HEADWALL (VARIES IN TYPE & SIZES)
OSD	ON-SITE DETENTION TANK (OSD)
	STORMWATER DRAINAGE BOX CULVERT
BASIN	STORMWATER DRAINAGE BASIN
	STORMWATER QUALITY IMPROVEMENT DEVICE (SQID)  STORMWATER DRAINAGE LINE WITH:
Ø375mm RCP2(PIPE SIZE 15.0m @ 1.0%(PIPE GRADI 10m(PIPE LENGTH) US/IL:(UP STREAM INVER DS/IL:(DOWN STREAM INV	E) T LEVEL)
A00	STORMWATER DRAINAGE STRUCTURE NUMBER
PIT NUMBER  STORMWATER LINE NO	JMBER
SWDP_O	DOWNPIPE CONNECTION (Ø150uPVC U.N.O)
SWRP_O	RODDING POINT
LLY 0000	CONTINUATION ON BUILDING HYDRAULICS ENGINEERING DRAWINGS

DEMOLITION LEGEND				
	EXISTING BUILDING AND ASSOCIATED			
	EXISTING INFRASTRUCTURE TO BE DEMOLISHED AND REMOVED FROM SITE			
	EXISTING VEGETATION, INCLUDING ALL TREE AND SHRUBS TO BE CLEARED, GRUBBED, AN DISPOSED OF TO STOCKPILE			

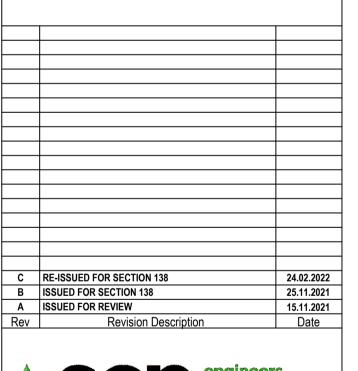
ABBREVIATIONS (ROAD)		
K&G	KERB AND GUTTER	
K&T	KERB AND TOE	
КО	KERB ONLY	
IK	INTEGRAL KERB	
MK	MOUNTABLE KERB	
IMK	INTEGRAL MOUNTABLE KERB	
RK&G	ROLL KERB AND GUTTER	
RK&T	ROLL KERB AND TOE	
LK	LAYBACK KERB	
FK	FLUSH KERB	
PK	PRECAST KERB	
BDD	BRICK DISH DRAIN	
BK	BRICK KERB	
TK	TIMBER KERB	
CES	CONCRETE EDGE STRIP	
TES	TIMBER EDGE STRIP	
BES	BRICK EDGE STRIP	
PR	PRAM RAMP	
VC	VEHICULAR CROSSING	
FSL	FINISHED SURFACE LEVEL	
ESL	EXISTING SURFACE LEVEL	
FFL	FINISHED FLOOR LEVEL	

FFL	FINISHED FLOOR LEVEL		
4 D D D E \ //	ATIONIC (OTODIANA TED)		
ABBREVIATIONS (STORMWATER)			
S.G.G.P	SINGLE GRATED GULLY PIT		
E.K.I.	EXTENDED KERB INLET		
G.S.I.P.	GRATED SURFACE INLET PIT		
G.D.	GRATED DRAIN		
J.P.	JUNCTION PIT		
MH	MANHOLE		
H.W.	HEADWALL		
RCP	REINFORCED CONCRETE PIPE		
RRJ	RUBBER RING JOINT		
C2, C3, C4	PIPE CLASSIFICATIONS		
RCBC	REINFORCED CONCRETE BOX CULVERT		
A.D.D.	APRON DISH DRAIN		
G.R.P.	GLASS REINFORCED POLYMER		
DP	DOWNPIPE		
HER	HIGH END RISER		
IR	INTERMEDIATE RISER		
CO	CLEAROUT		
DP	DOWNPIPE		
FRP	FIBRE REINFORCED POLYMER		
SQID	STORMWATER QUALITY IMPROVEMENT DEVICE		
SP	SURCHARGE PIT		

ABBREVIATIONS (STRUCTURAL)			
SCJ	SAWN CONTRACTION JOINT		
EJ	EXPANSION JOINT		
DCJ	DOWELLED CONTRACTION JOINT		
IJ	ISOLATION JOINT		
DDJ	DIAMOND DOWELLED JOINT		
DDJI	DIAMOND DOWELLED JOINT INTERFACE WITH STRUCTURE		
DEJ	DOWELLED EXPANSION JOINT		
TJ	FOOTPATH TOOLED JOINT		
ET	EDGE THICKENING		

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WILSON DRIVE, MARULAN

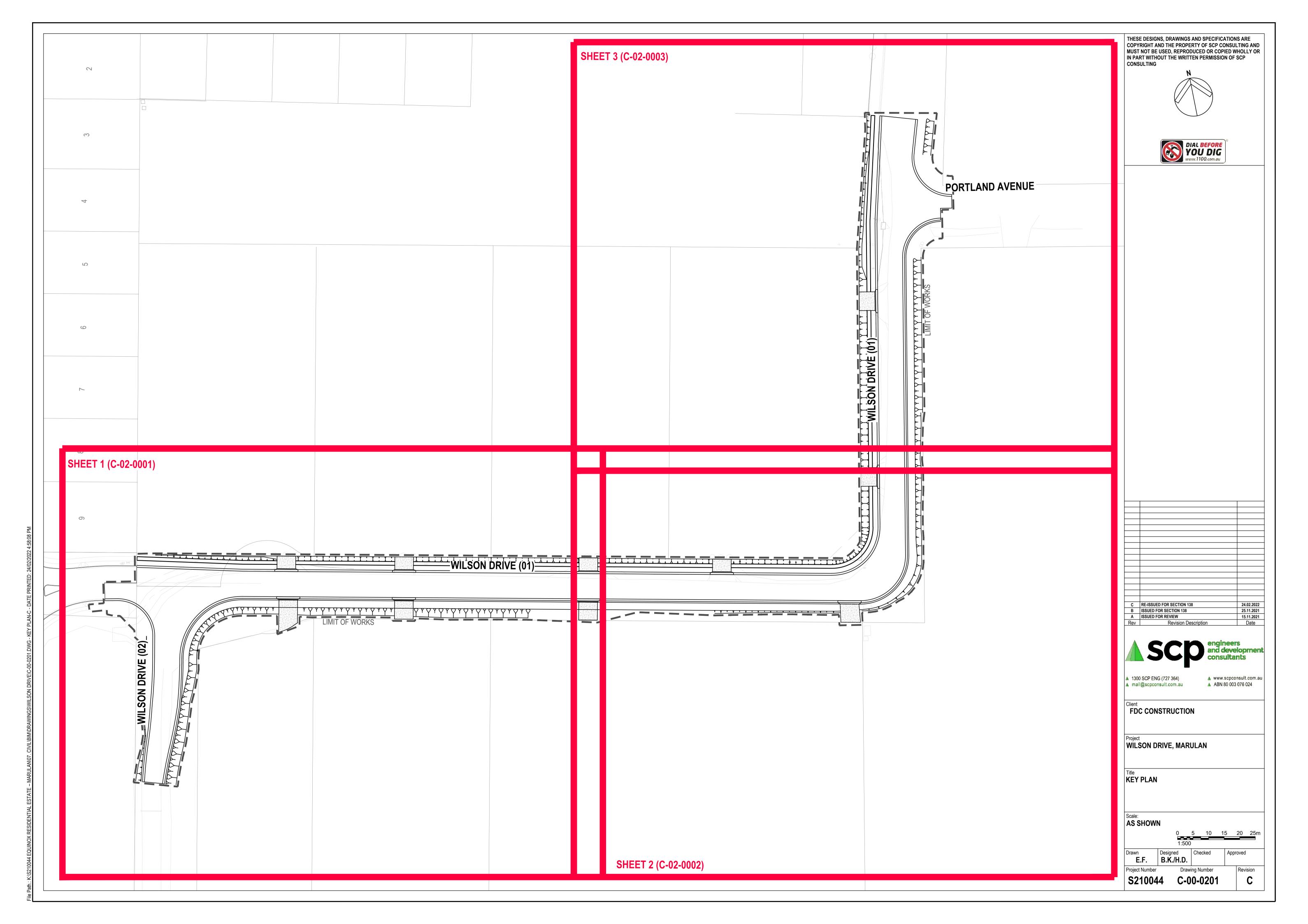
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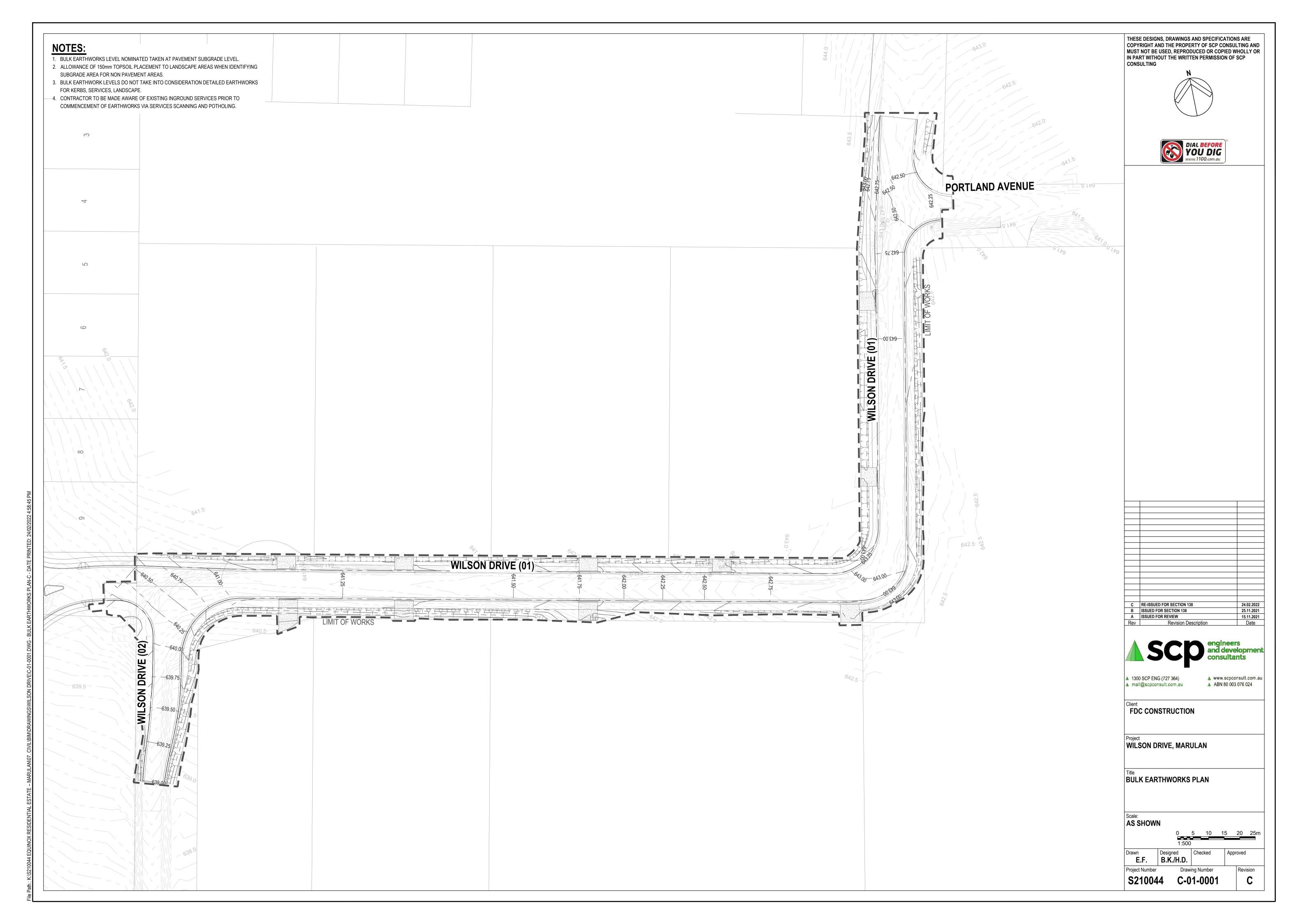
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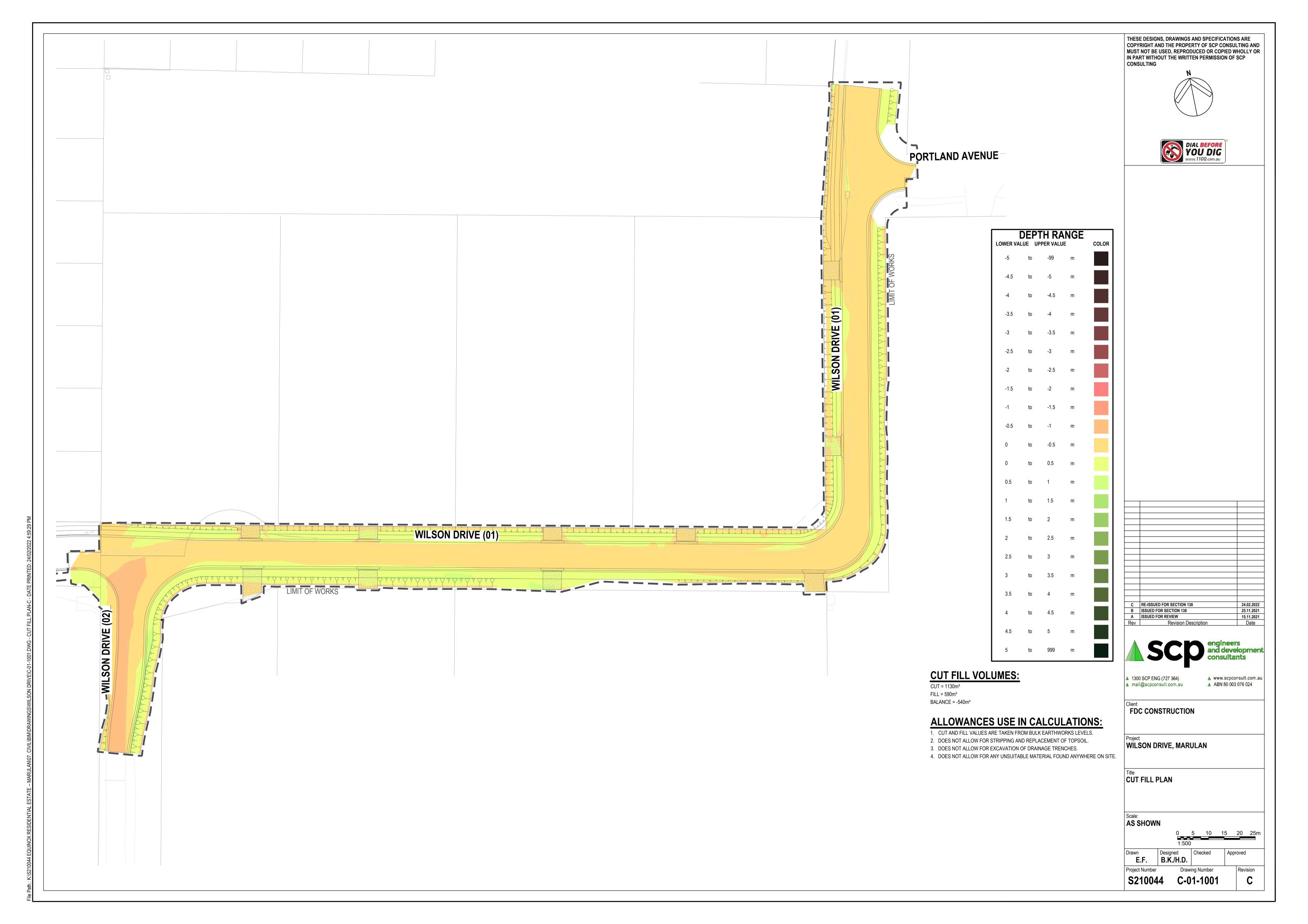
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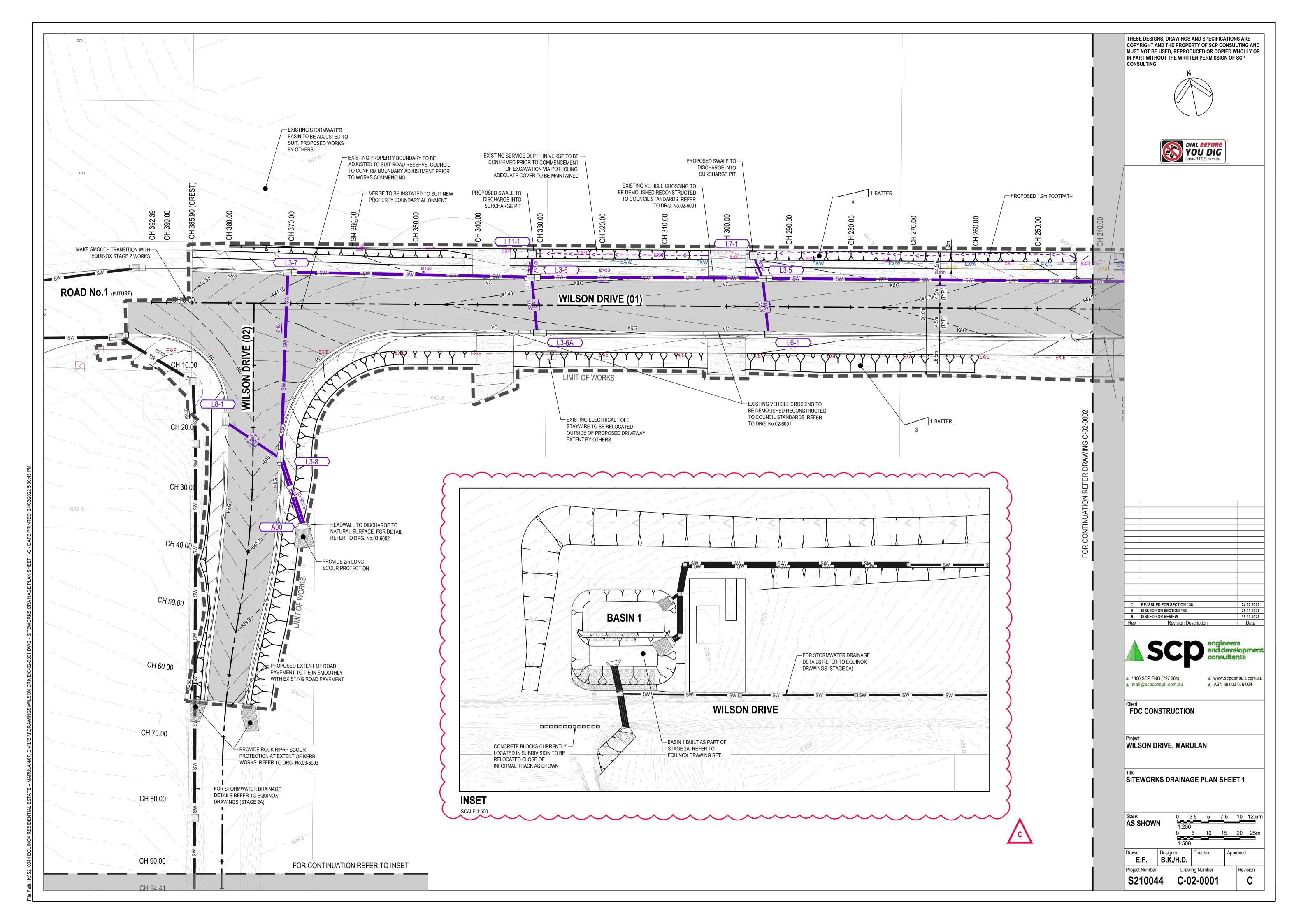
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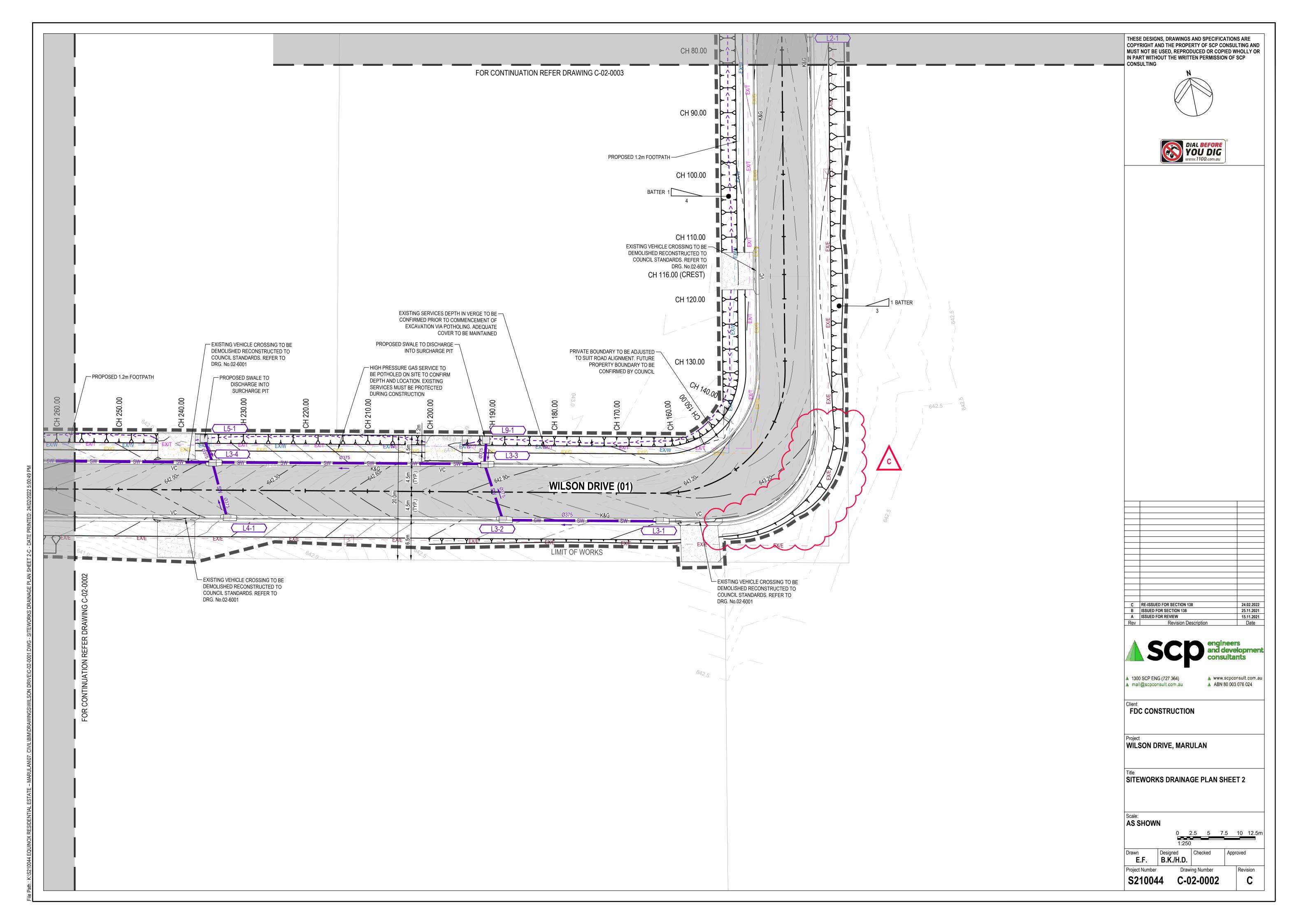
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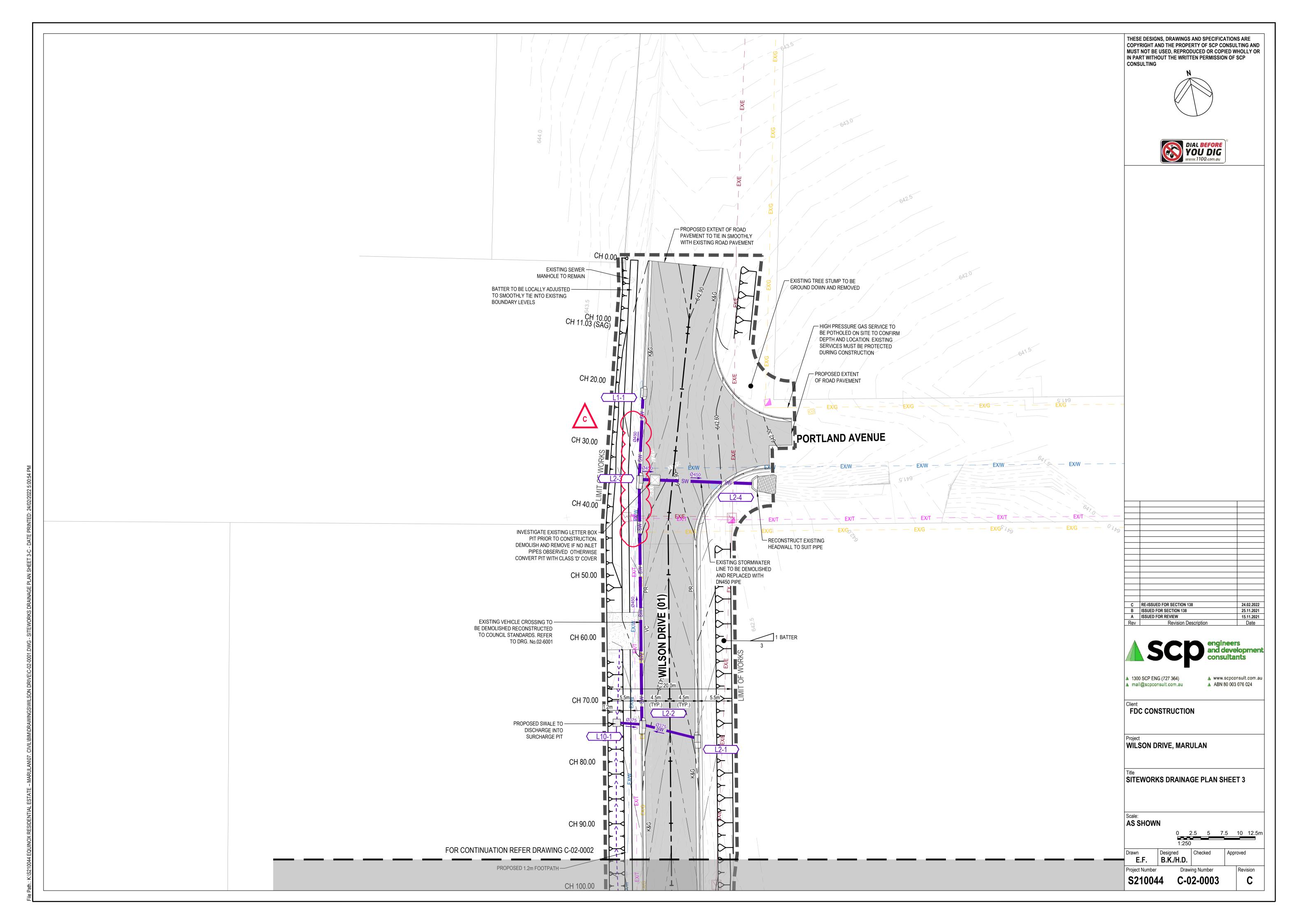


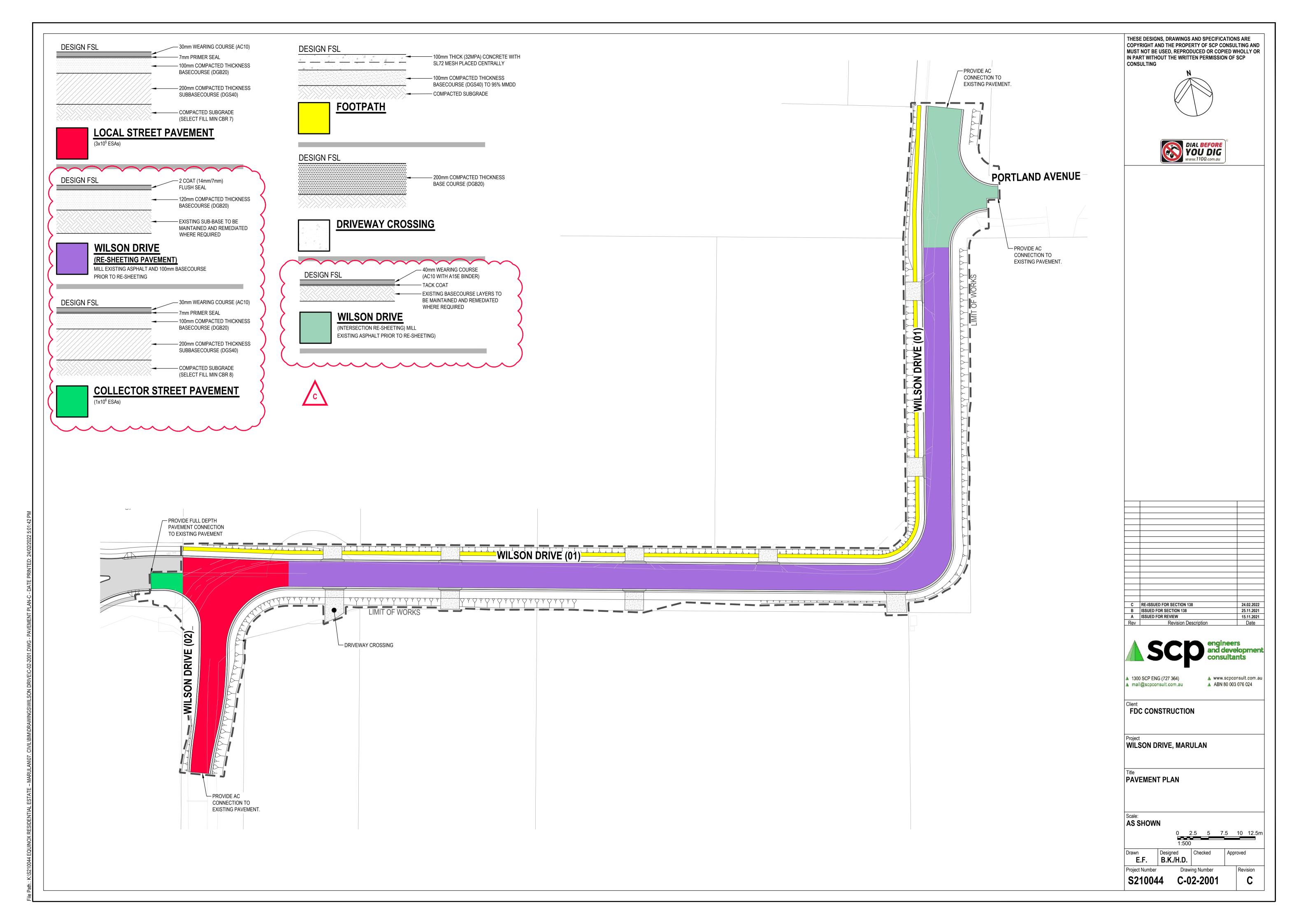


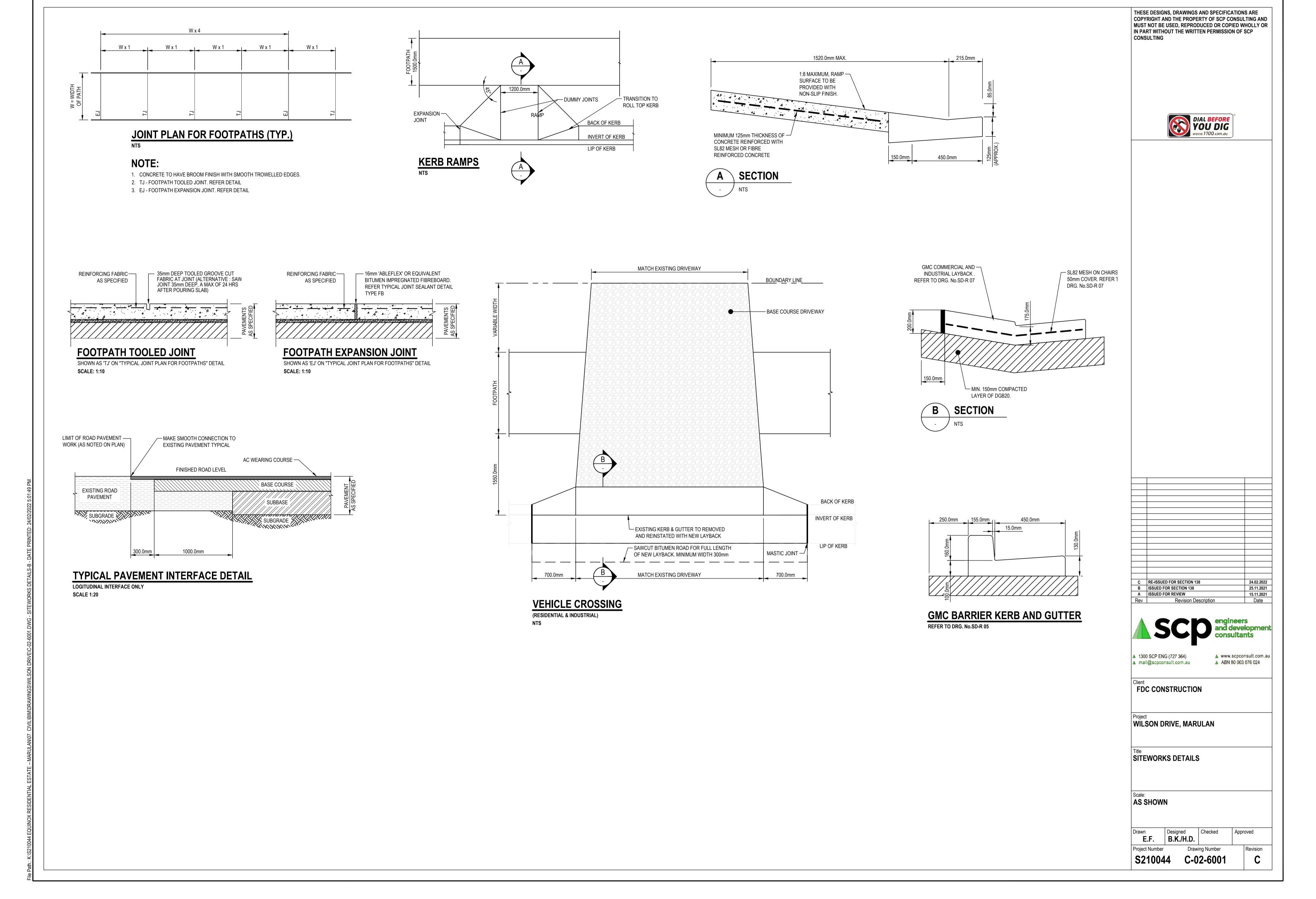








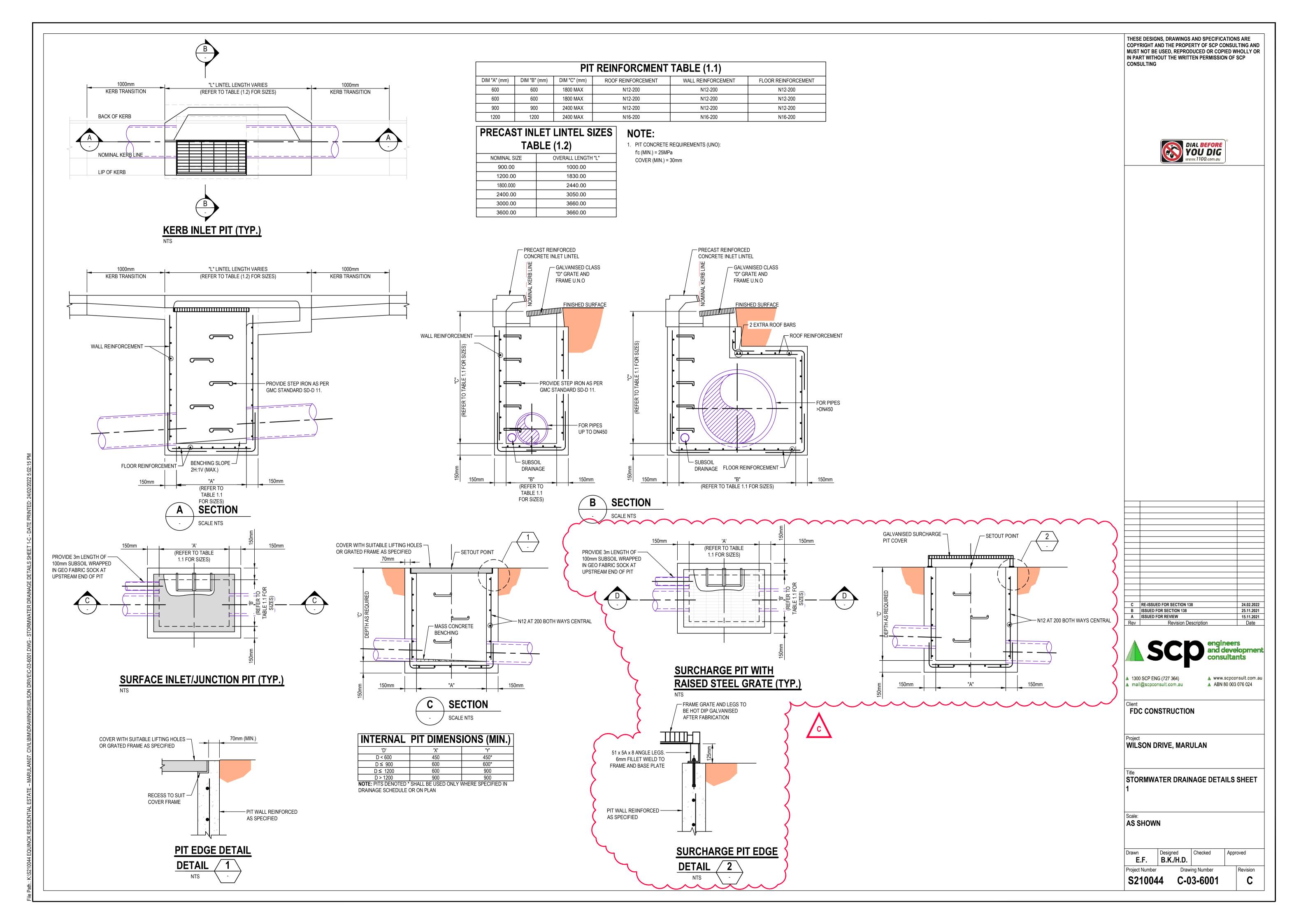


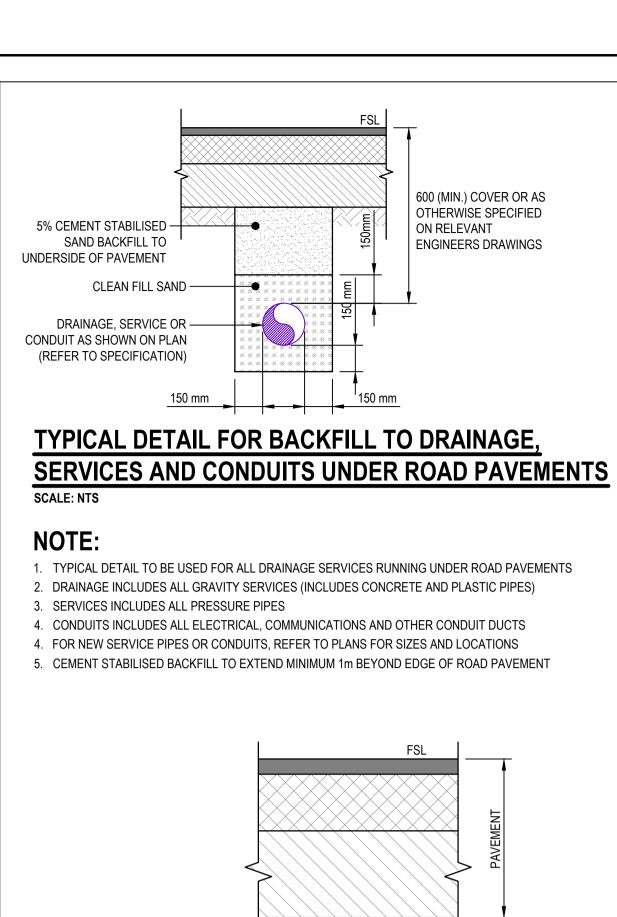


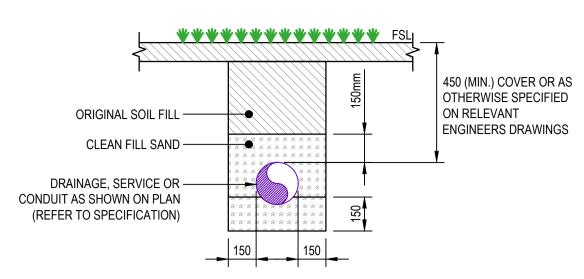
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 0
 2
 4
 6
 8
 10m

 Drawn
 1:200
 Designed
 Checked
 App
 E.F. B.K./H.D. Project Number Revision S210044 C-03-5001 В

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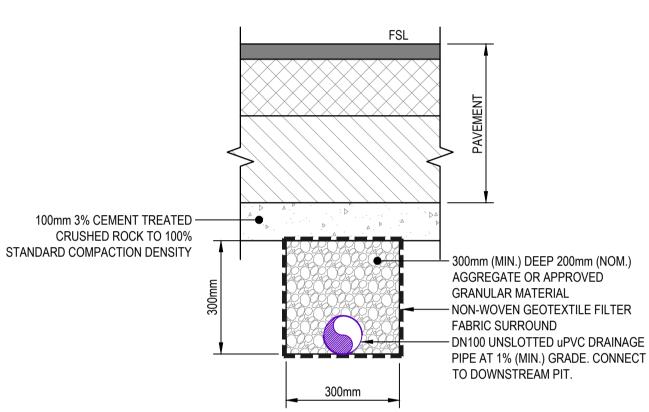




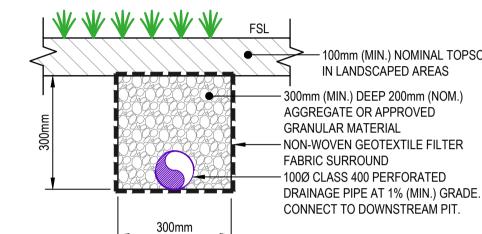
#### TYPICAL DETAIL FOR BACKFILL TO DRAINAGE, SERVICES AND CONDUITS IN LANDSCAPE AREAS SCALE: NTS

#### NOTE:

- 1. TYPICAL DETAIL TO BE USED FOR ALL DRAINAGE SERVICES RUNNING UNDER LANDSCAPE
- 2. DRAINAGE INCLUDES ALL GRAVITY SERVICES (INCLUDES CONCRETE AND PLASTIC PIPES)
- 3. SERVICES INCLUDES ALL PRESSURE PIPES
- 4. FOR NEW SERVICE PIPES OR CONDUITS, REFER TO PLANS FOR SIZES AND LOCATIONS



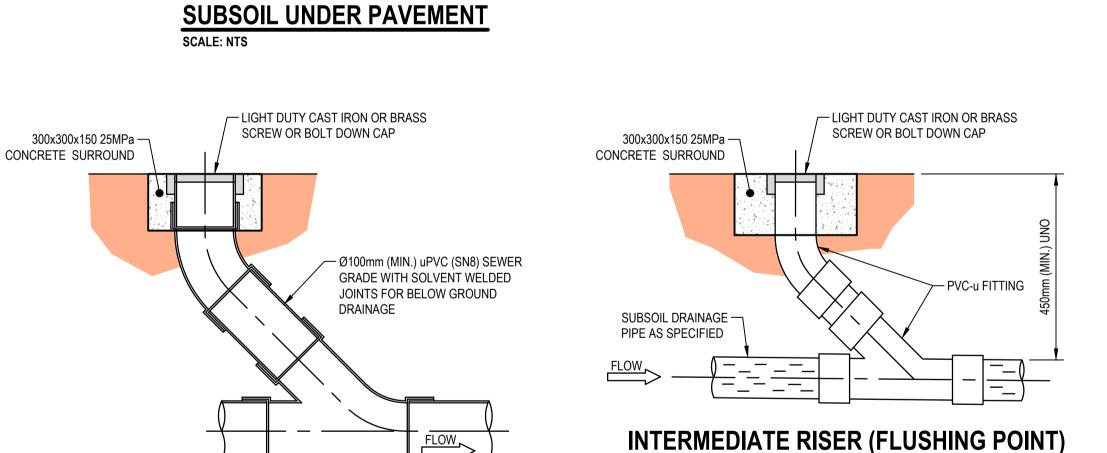
**CLEAR OUT (CO)** 



SUBSOIL IN LANDSCAPING AREAS

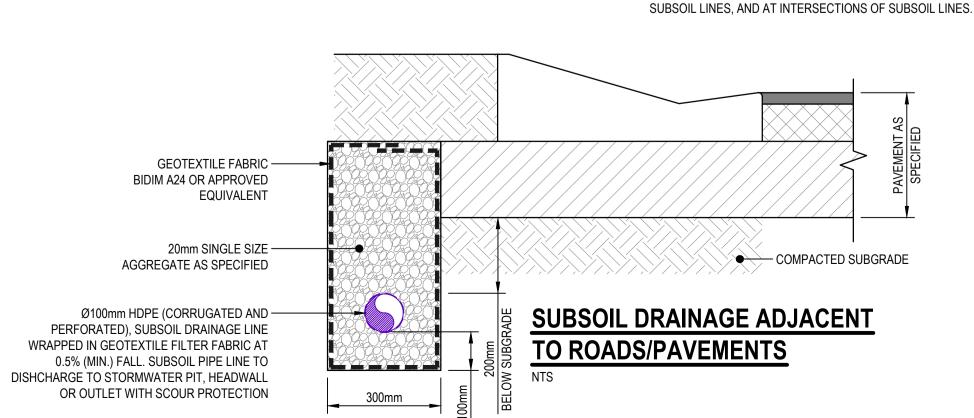
# 100mm (MIN.) NOMINAL TOPSOIL DRAINAGE PIPE AT 1% (MIN.) GRADE.

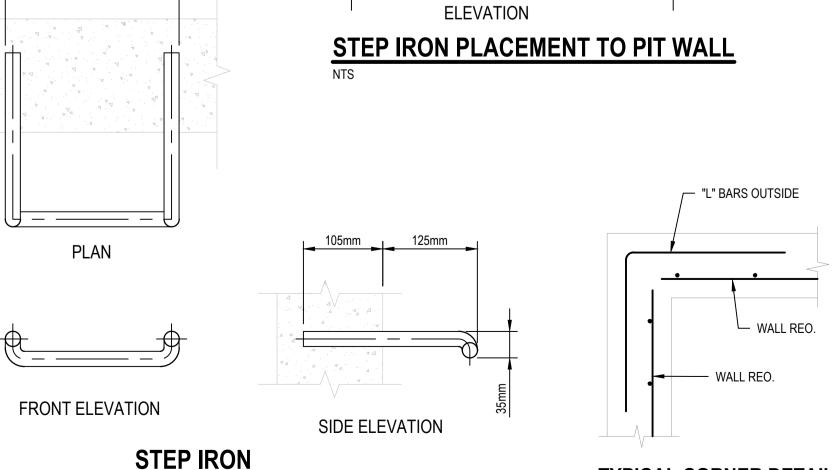




#### NOTE:

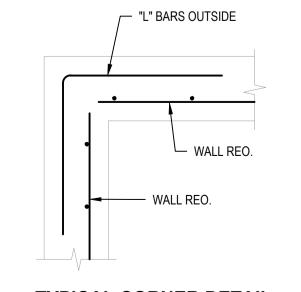
1. SUBSOIL FLUSHING POINTS TO BE PROVIDED AT 50m MAX INTERVALS ALONG SUBSOIL LINES (UNLESS SHOWN OTHERWISE), AT CHANGES IN DIRECTION OF



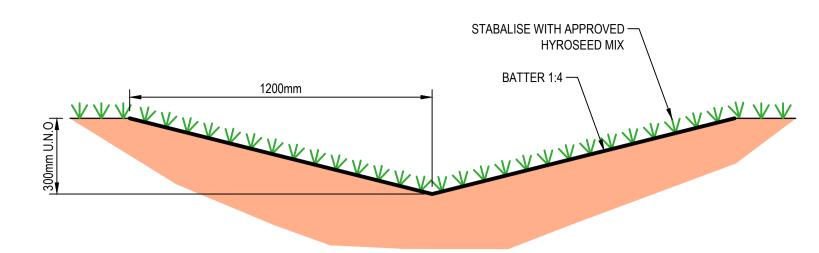


PIT BASE



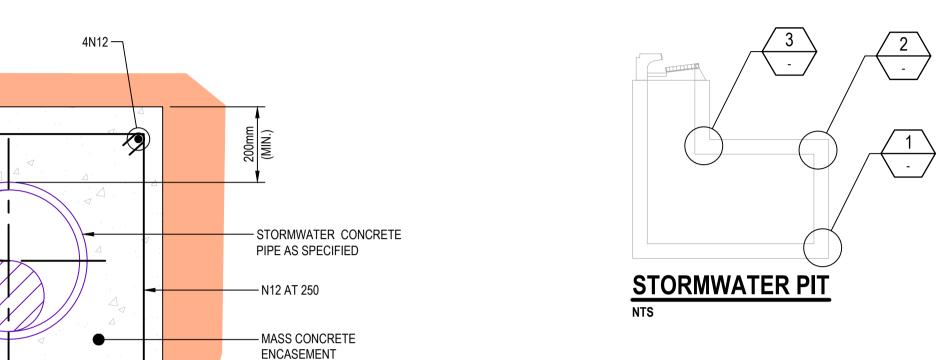


TYPICAL CORNER DETAIL "L" BAR = N12-200 600 LEGS





SCALE: NTS



STORMWATER CONCRETE PIPE ENCASEMENT SCALE: NTS

**\*\*\*\*\*** FSL

TYPICAL SUBSOIL DRAINAGE DETAIL

GEOTEXTILE FABRIC -

APPROVED EQUIVALENT

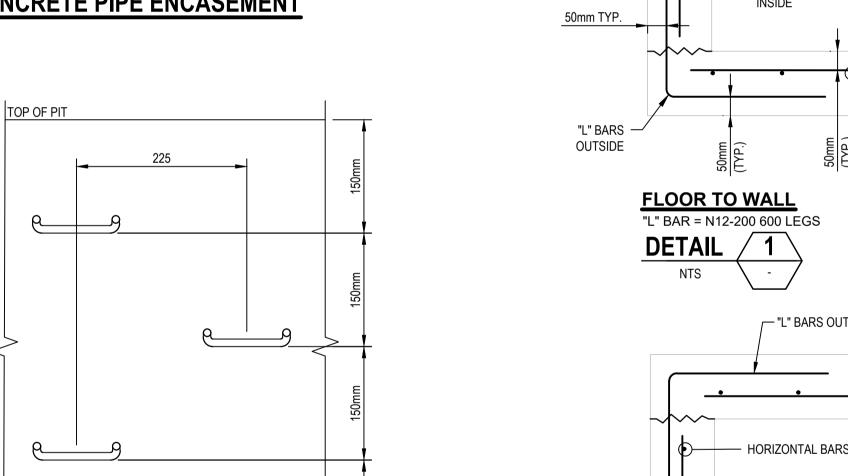
PERFORATED DRAINAGE

PIPE AT 1% (MIN.) GRADE.

BIDIM A24 OR

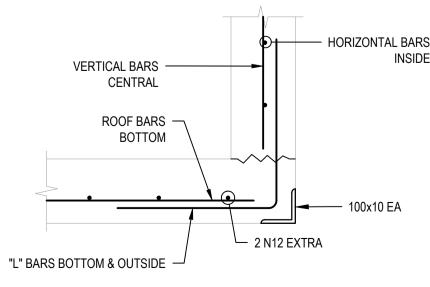
CLEAN FILL SAND -

Ø100mm CLASS 400 -



SURROUND (25MPa)



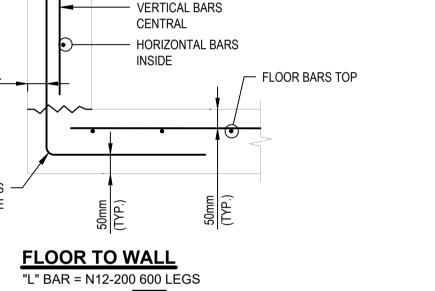


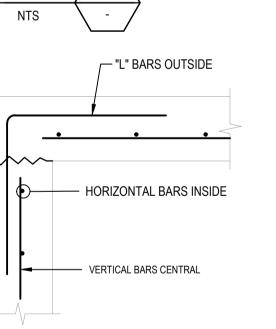


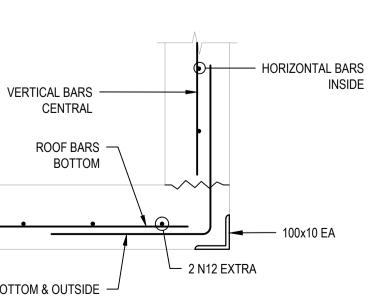
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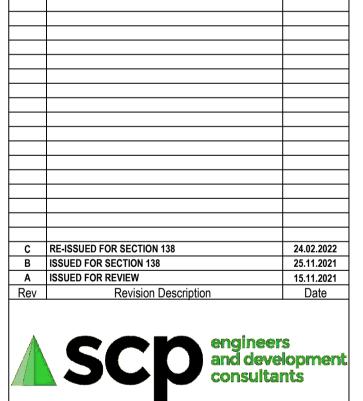
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WILSON DRIVE, MARULAN

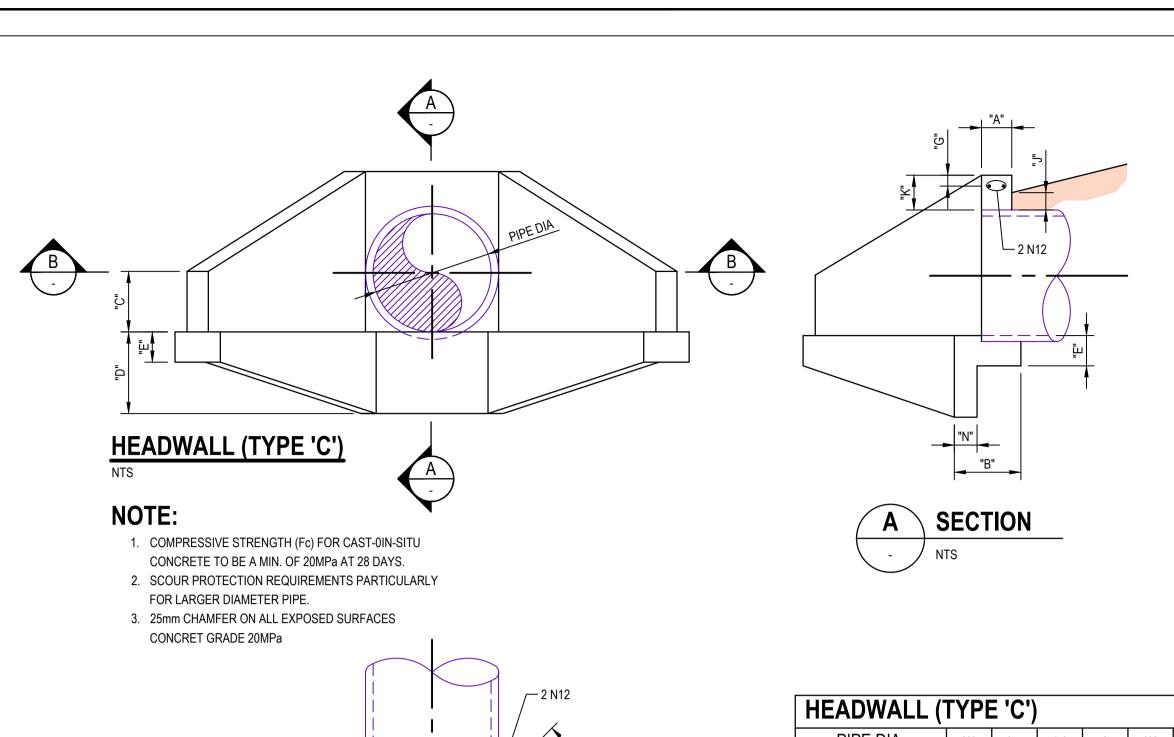
STORMWATER DRAINAGE DETAILS SHEET

AS SHOWN

Checked Approved B.K./H.D. Project Number Drawing Number Revision C-03-6002

**DETAIL** 

230mm

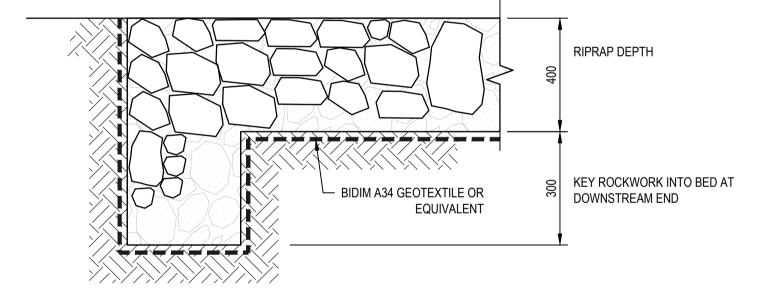


CONCRET GRADE 20MPa
2 N12 /4637mm
150mm CURTAIN WALL
B SECTION  - NTS

TO 01 11T DIDE 017E

HEADWALL (	TYPE	E 'C')							
PIPE DIA.	300	375	450	525	600	675	750	825	900
A	150	150	150	150	175	175	200	200	225
В	300	300	300	300	450	450	450	450	450
С	300	300	300	300	300	300	300	300	300
D	375	375	375	375	375	530	530	530	530
Е	150	150	150	150	175	175	175	175	225
F	75	75	75	75	100	100	100	100	100
G	40	40	40	40	50	50	50	50	50
Н	70	70	70	70	100	100	100	100	125
J	100	100	100	100	100	100	100	100	100
K	200	200	200	200	300	300	300	300	300
W	700	700	850	1000	1100	1300	1450	1600	1750
L	800	850	950	1000	1100	1200	1250	1350	1450
REINFORCEMENT DIA.	12	12	12	12	12	12	12	12	12
LENGTH	1600	1700	1900	2000	2200	2400	2500	2700	2800

— PRECAST CONCRETE HEADWALL 4m (MIN.) PROVIDE RANDOMLY SPACED DN = -500mm BOULDERS FOR ENERGY DISSIPATER WITHIN OUTLET BIDIM A34 GEOTEXTILE OR -**EQUIVALENT** ROCK RIP RAP D50=400mm AT 400mm THICKNESS ON GEOTEXTILE BIDIM A24 SCOUR PROTECTION (TYP.)
SCALE 1:20 FABRIC (OR EQUIVALENT) FOR SCOUR PROTECTION



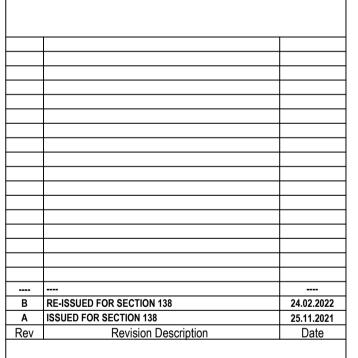
#### **ROCK RIPRAP SPECIFICATIONS**

#### NOTE:

- 1. ALL RIPRAP MUST CONSIST OF ANGULAR RUN-OF-QUARRY DURABLE ROCK
- 2. ALL ROCK IS TO BE A HARD DURABLE ROCK WITH A POINT INDEX LOAD Is50 GREATER THAN 1.0MPa AS DETERMINED IN ACCORDANCE WITH AS4133.4.1 -2007
- 3. RIPRAP IS TO BE PLACED BY HAND AND CROW-BARRED INTO PLACE TO ENSURE FIT WITH
- 4. BASE OF RIPRAP RO BE LINED WITH GEOTEXTILE FABRIC (BIDIM A34)
- 5. ROCK SIZE D<sub>50</sub> 150mm
  - Dmax = 300mm

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FDC CONSTRUCTION

WILSON DRIVE, MARULAN

STORMWATER DRAINAGE DETAILS SHEET

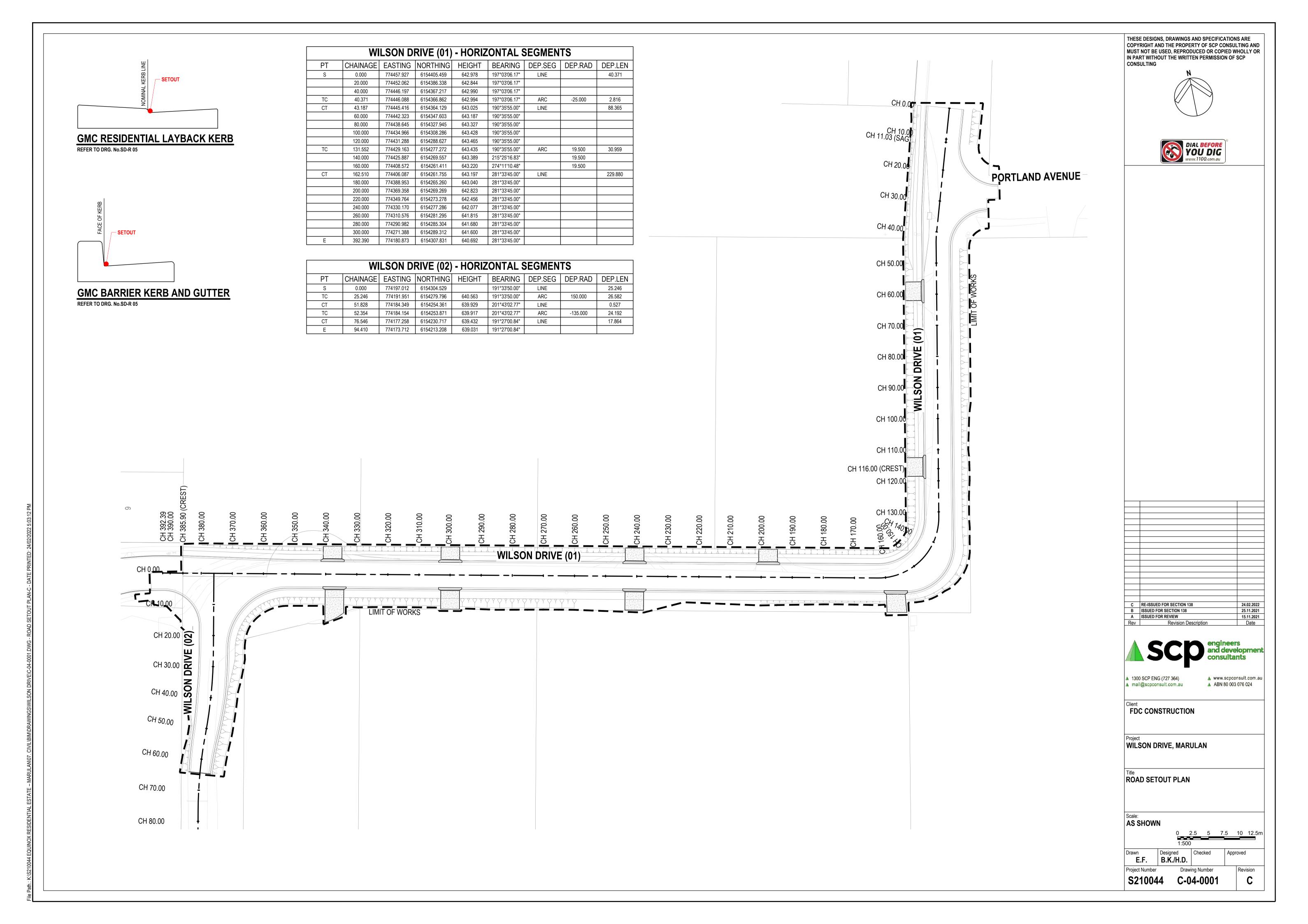
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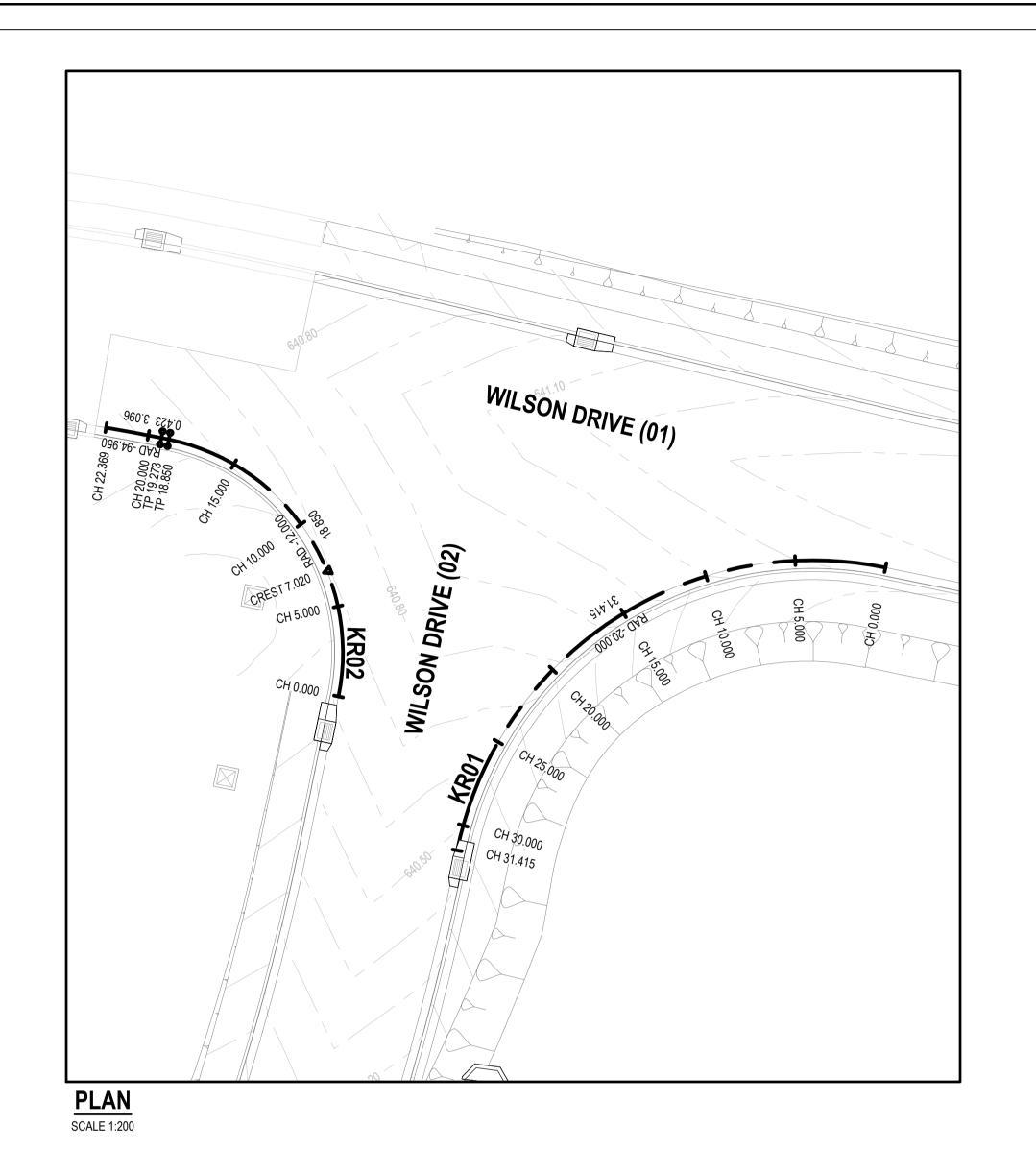
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Project Number Drawing Number

C-03-6003

Revision

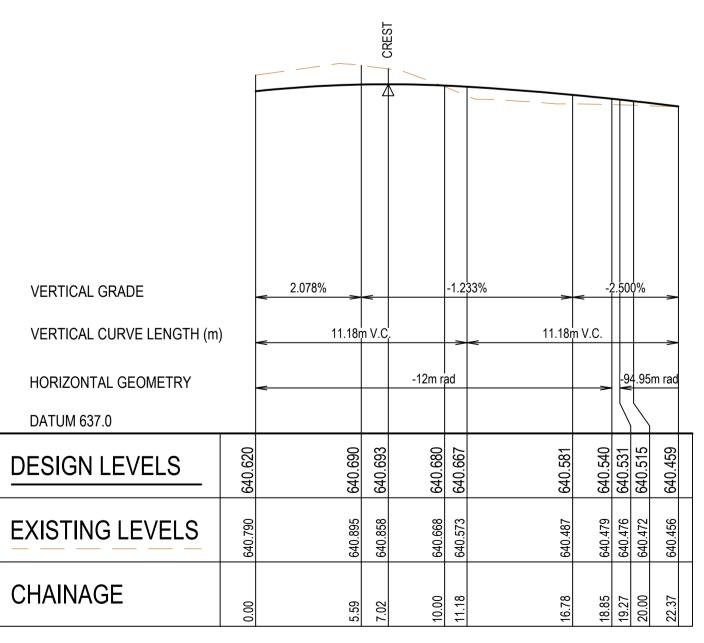




-0.500% -2.253% **VERTICAL GRADE** VERTICAL CURVE LENGTH (m) 15.71m V.C. 15.71m V.C. -20m rad HORIZONTAL GEOMETRY **DATUM 637.0 DESIGN LEVELS EXISTING LEVELS** CHAINAGE

#### KERB RETURN - KR 01 WILSON DR

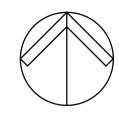
Vertical scale 1:40 Horizontal scale 1:200



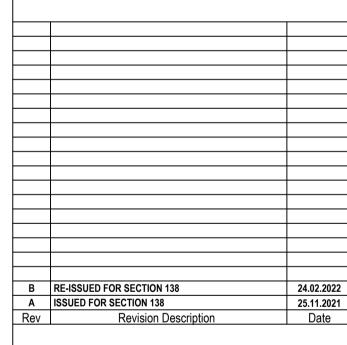
#### KERB RETURN - KR 02 WILSON DR

Vertical scale 1:40 Horizontal scale 1:200

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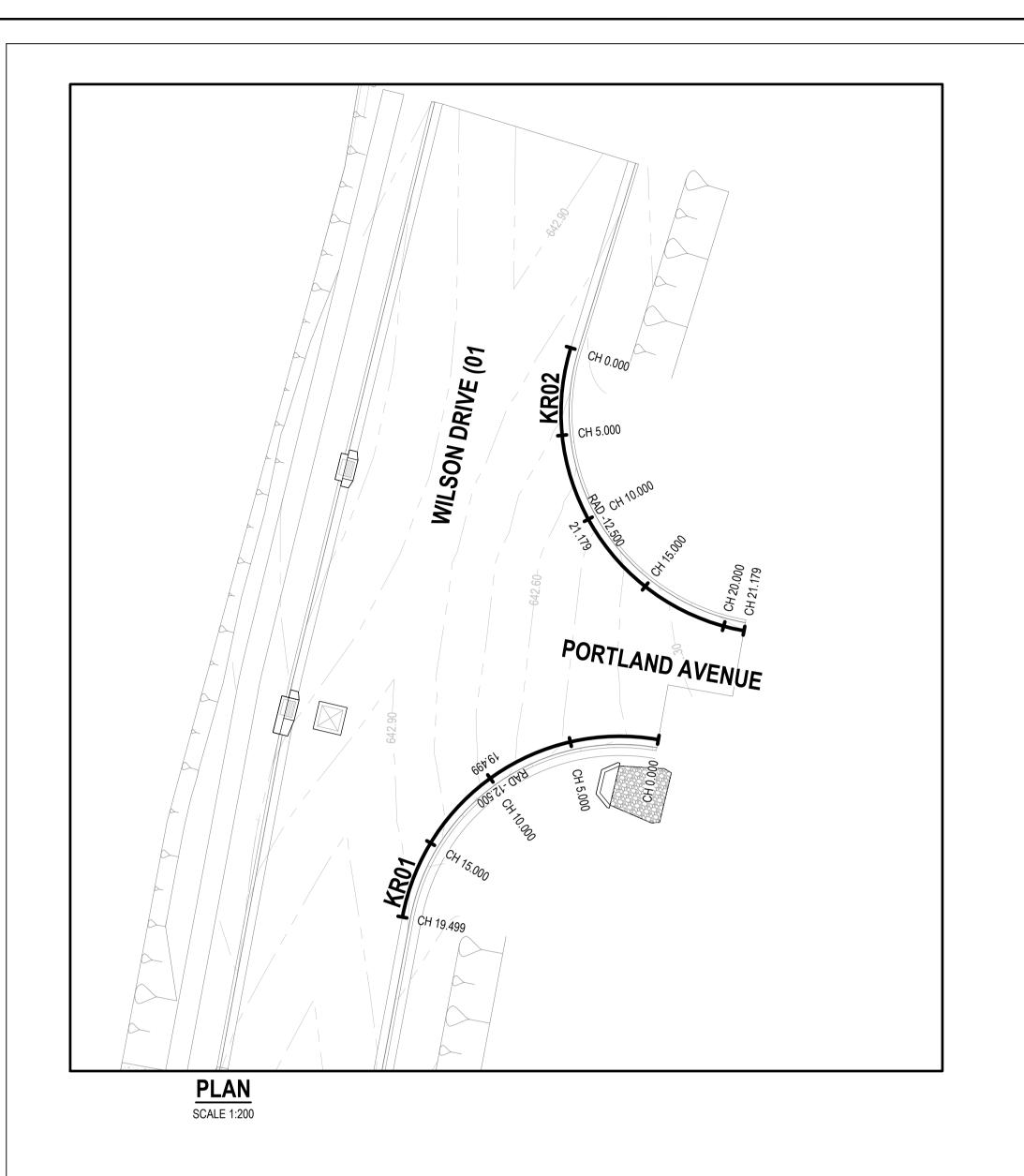
FDC CONSTRUCTION

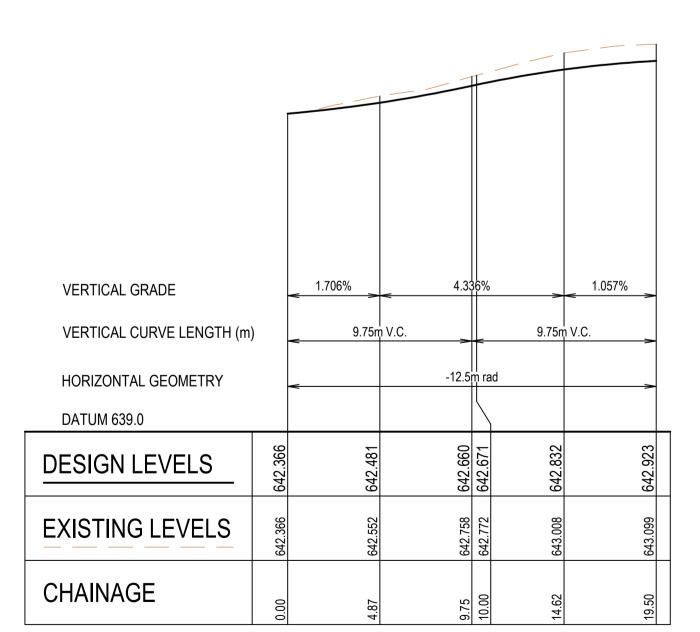
WILSON DRIVE, MARULAN

ROAD KERB RETURNS - SHEET 1

0 0.4 0.8 1.2 1.6 2m 1:40 AS SHOWN Designed Checked B.K./H.D. Drawing Number

S210044 C-04-1001





KERB RETURN - KR01 PORTLAND AVE

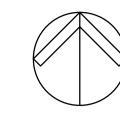
Vertical scale 1:40
Horizontal scale 1:200

	1							
				_				
				-   -			_	
VERTICAL GRADE		-0.787%	-4	.038	3%	-1.706%	->	
VEDTICAL CUDVE LENGTH (***)		10.59r	- V C		10.59r	- V C		
VERTICAL CURVE LENGTH (m)	)	<u>&lt; 10.591</u>	II V.C.	><	= 10.59i =	II V.C.	->	
HORIZONTAL GEOMETRY		<	-12	.5m	rad		->	
DATUM 639.0				$\setminus$				
DATON 039.0								7
DESIGN LEVELS	642.760	642.675	.527	642.504	642.321	642.222	642.200	
	642	642	642	642	642	642	642	
EXISTING LEVELS	28	46	12	90	03	27	10	
AISTING LEVELS	642.758	642.646	642.412	642.408	642.303	642.227	642.201	
CHAINAGE	0.00	5.29	10.00	10.59	15.88	20.00	21.18	
		47	`	,	`		` '	L

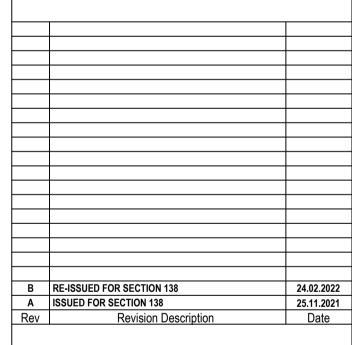
KERB RETURN - KR02 PORTLAND AVE

Vertical scale 1:40
Horizontal scale 1:200

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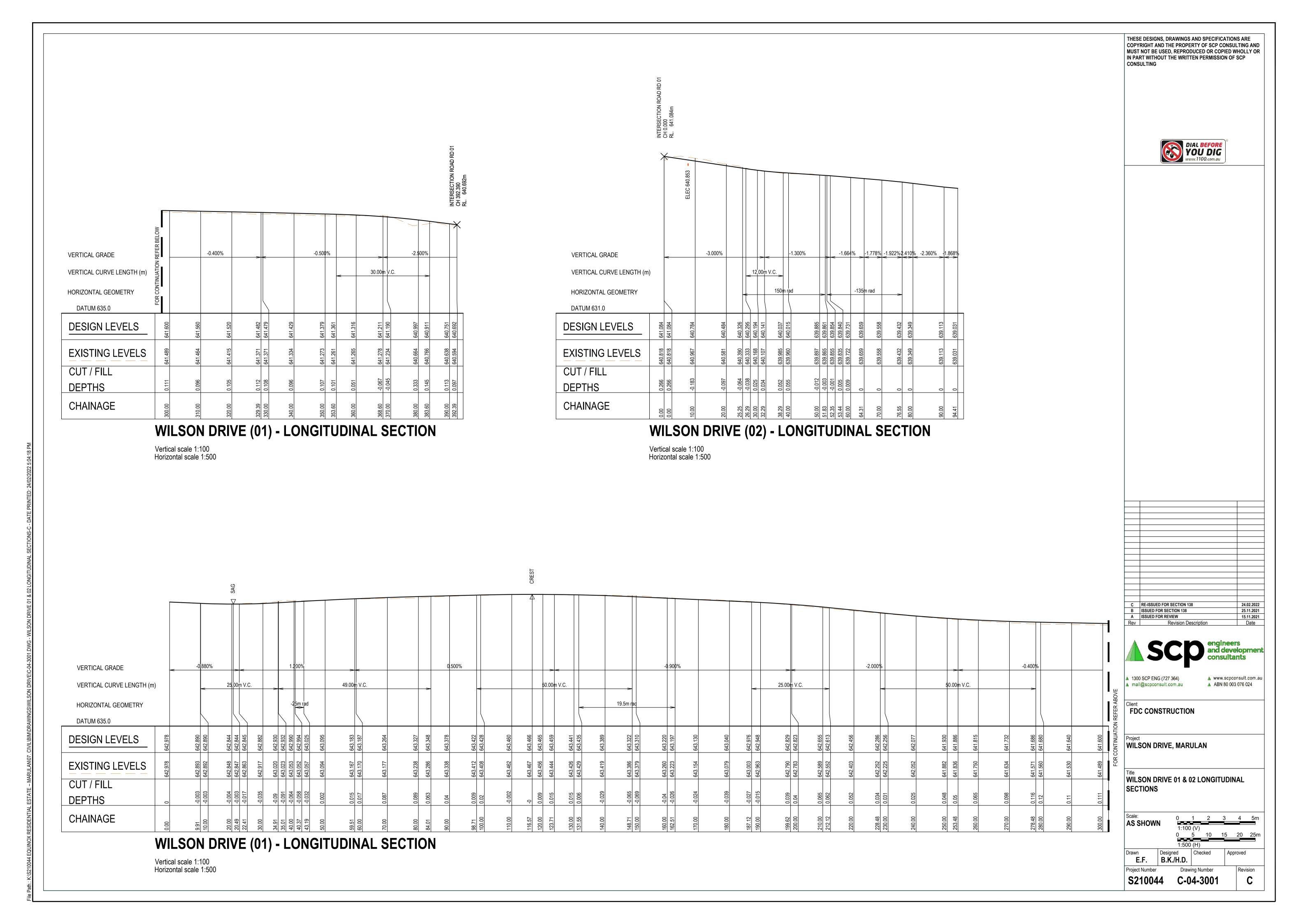
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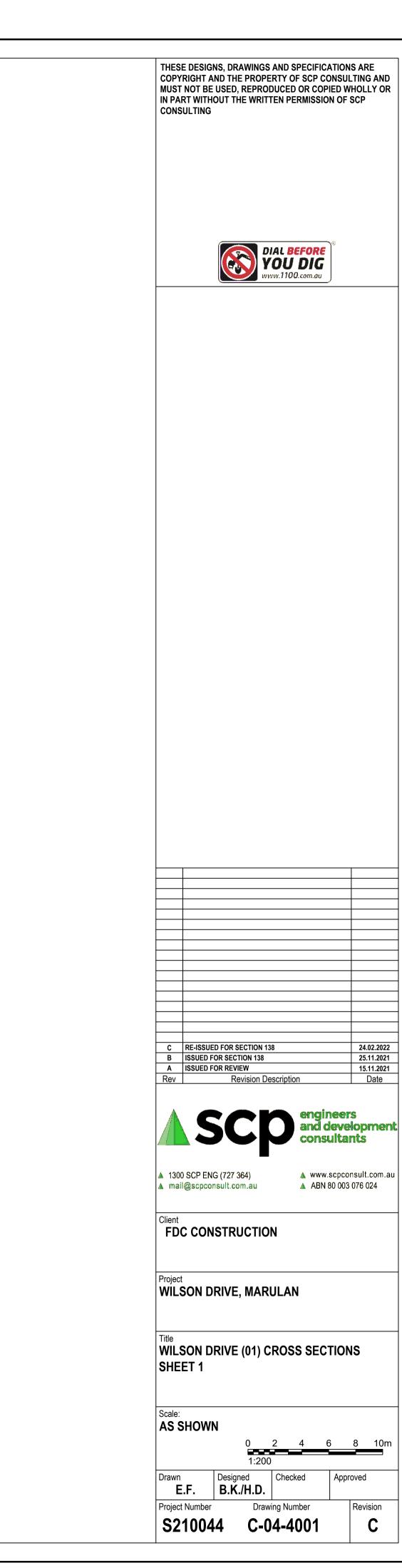
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FDC CONSTRUCTION

Project
WILSON DRIVE, MARULAN

ROAD KERB RETURNS - SHEET 2



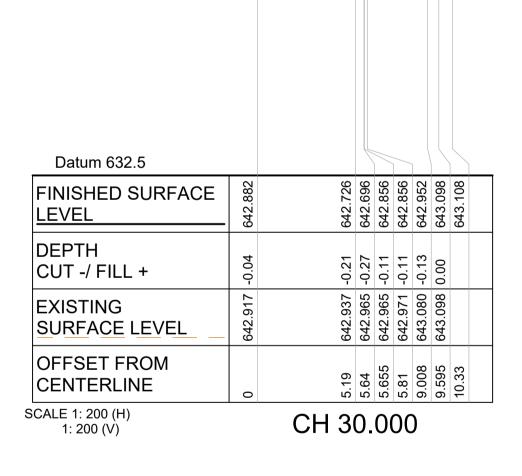


		-8%	2.5%			-3%		-3%			3%					_
Datum 632.5																
FINISHED SURFACE LEVEL	642.76	642.96	642.89	642.89	642.73	642.76	642.881		642.693	642.663	642.823	642.823	642.887	643.079	643.208	643.619
DEPTH CUT -/ FILL +		0.40	0.16	0.15	-0.01	0.00	-0.00		-0.02	-0.03	0.13	0.11	-0.10	0.00	0.00	0.36
EXISTING SURFACE LEVEL		642.560	642.732	642.738			642.885		642.713	642.695		642.717	642.985	_	643.208	643.259
		-7.5	-4.67	-4.515	-4.5				6.271	6.721	6.736	6.891	9.016	9.784	10.948	11.946

Datum 632.5									
FINISHED SURFACE LEVEL	642.99	642.851	642.821	642.981	642.981	643.094	643.105	643.115	
DEPTH CUT -/ FILL +	-0.06	-0.29	-0.39	-0.23	-0.25	-0.01	0.00		
EXISTING SURFACE LEVEL	643.053	643.144	643.211	643.213	643.230	643.108	643.105		
OFFSET FROM CENTERLINE	0	4.62	5.07	5.085	5.24	9.004	9.049	10.005	
SCALE 1: 200 (H) 1: 200 (V)		CH 4	ŀO	.C	0	0			

		-26.01%	2.5%	<b>—</b>		-3%		-3%	_	2.	<u>5</u> %	2.5%	
									GAS 643.035 •				
Datum 632													
FINISHED SURFACE LEVEL	642.616	643.266	643.195	643.195	643.035	643.065	643.187	643 065	643.035	643.16	643.224	643.287	
DEPTH CUT -/ FILL +	00.00	0.39	0.07	90.0	-0.10		0.02	000	-0.25	-0.14	-0.10		
EXISTING SURFACE LEVEL	642.616	642.872	643.123	643.131	643.132	643.156	643.170	990 879	643.281	643.297	643.321		
OFFSET FROM CENTERLINE	-10	-7.5	-4.67	-4.515	-4.5	-4.05	0	ر در	4.5	4.95	7.5	10	
SCALE 1: 200 (H) 1: 200 (V)	,				C	<b>∵</b>	160	0.000					

	8%	2.5%		_	-3%	-3%			3%		10.9	7%		_
Datum 632.5														
FINISHED SURFACE LEVEL	642.969	642.898	642.898	642.738	642.768 642.89		642.7	642.67	642.83	642.83	642.892	643.061	643.202	643.589
DEPTH CUT -/ FILL +	0.40	0.16	0.15	-0.01	00:00		-0.03	-0.04			-0.08	0.00	0.00	0.34
EXISTING SURFACE LEVEL	642.564	642.743	642.748	642.749	642.766		642.726	642.706		_	642.971	643.061	643.202	643.251
OFFSET FROM CENTERLINE	-10	4.67	4.515	4.5	4.05		6.33	6.78	6.795	6.95	9.016	9.691	10.981	11.806
SCALE 1: 200 (H) 1: 200 (V)				·	СН	10.000								



		-24.79%	2.5%			-3%		-3%	_		3%	, 0		,
Datum 632									GAS 642.944					
FINISHED SURFACE LEVEL	642.555	643.175	643.104	643.104	642.944	642.974	643.095	642.974	642.944	643.104	643.104	643.234	643.226	643.236
DEPTH CUT -/ FILL +	0.00	0.35	0.03	0.02	-0.14	-0.14	00.00	0.01			0.03	0.01	0.00	
EXISTING SURFACE LEVEL	642.555	642.828	643.076	643.086	643.087	643.115	643.094	642.969	_	643.046	643.075	643.225	643.226	
OFFSET FROM CENTERLINE	-10	-7.5	-4.67	-4.515	-4.5	-4.05	0	4.05	4.5	4.515	4.67	6	9.033	10
SCALE 1: 200 (H) 1: 200 (V)					C	CH	1 50	0.000						

	Ī	-8%	2.5%			-3%		-3%		_	3%	2!	5%	_		
Datum 632.5																
FINISHED SURFACE LEVEL	642.857	643.057	642.986	642.986	642.826	642.856	642.978		642.771	642.741	642.901	642.901	642.946	643.055	643.391	643.166
DEPTH CUT -/ FILL +			0.11	0.10	-0.06	-0.05	00.00		-0.03	-0.05	0.10	0.09	-0.06	0.00	0.26	0.00
EXISTING SURFACE LEVEL			642.877	642.884	642.885	642.907	642.978		642.804	642.795	642.796	642.815	643.001	643.055	643.130	643.166
OFFSET FROM CENTERLINE	-10	-7.5	-4.67	-4.515	-4.5	-4.05	0		6.9	7.35	7.365	7.52	9.02	9.457	10.802	11.802

1: 200 (V)

CH 0.000

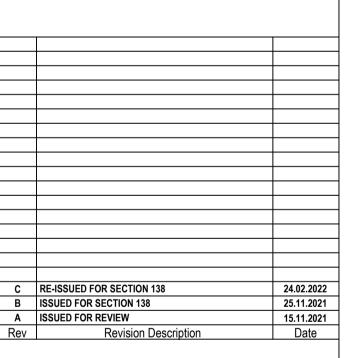
Datum 632.5									_
FINISHED SURFACE LEVEL	642.844	642.671	642.641	642.801	642.801	642.88	643.158	643.191	
DEPTH CUT -/ FILL +	-0.00	-0.07	-0.20	-0.04	-0.07	-0.22	0.00	0.00	
EXISTING SURFACE LEVEL	642.848	642.744	642.840	642.843	642.876	643.102	643.158	643.191	
OFFSET FROM CENTERLINE	0	5.76	6.21	6.225	6.38	9.012	10.124	10.656	
CALE 1: 200 (H) 1: 200 (V)	,	CH 20	).(	00	0				

		-23.85%	2.5%			-3%	0	-3%		GAS 642.904		3%				
Datum 632																
FINISHED SURFACE LEVEL	642.528	643.124	643.053	643.053	642.893	642.923	643.045		642.915	642.885	643.045	643.045	643.166	643.163	643.173	
DEPTH CUT -/ FILL +	00.00	0.32	-0.01	-0.02	-0.18	-0.18	-0.02		-0.11	-0.19	-0.04	-0.05	0.00	0.00		
EXISTING SURFACE LEVEL	642.528	642.802	643.060	643.070	643.071	643.099	643.068		643.029	643.078	643.080	643.097	643.163	643.163		
OFFSET FROM CENTERLINE	-10	-7.5	-4.67	-4.515	-4.5	-4.05	0		4.334	4.784	4.799	4.954	9.002	9.012	10	
CALE 1: 200 (H) 1: 200 (V)					C	CH	14	5.022								



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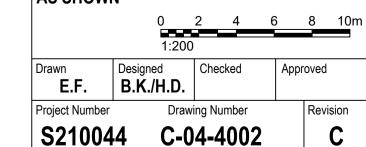
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▲ ABN 80 003 076 024

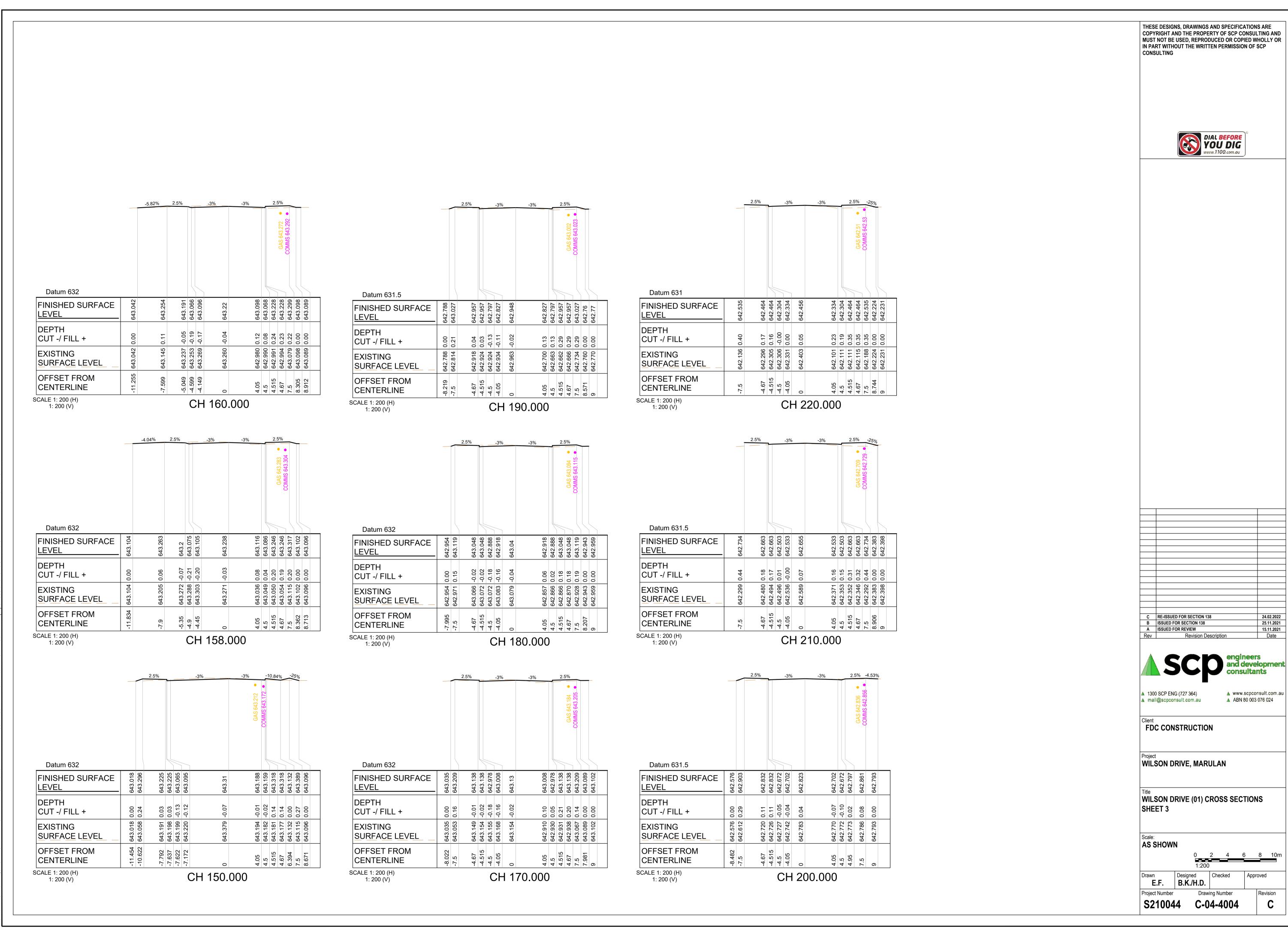
FDC CONSTRUCTION

WILSON DRIVE, MARULAN

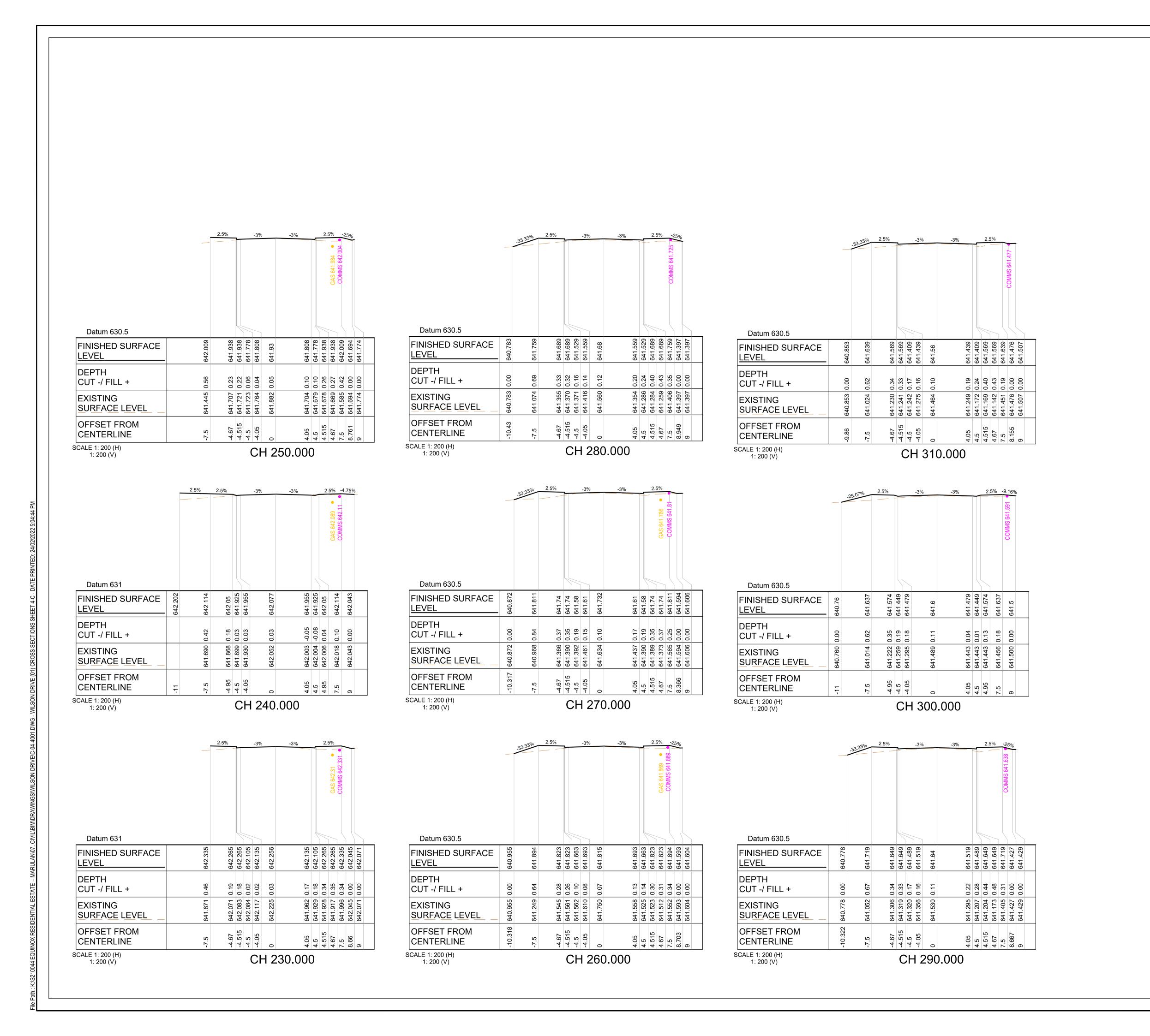
WILSON DRIVE (01) CROSS SECTIONS
SHEET 2

Scale:
AS SHOWN



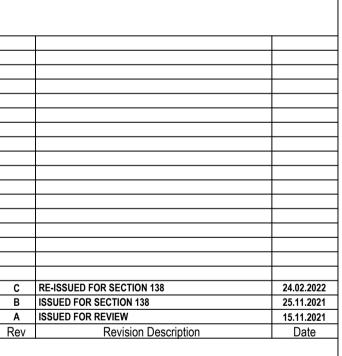


44 EQUINOX RESIDENTIAL ESTATE - MARULANIO7, CIVIL\BIM\DRAWINGS\WILSON DRIVE\C-04-4001.DWG - WILSON DRIVE (01) CROSS SECTIONS SHEET 3-C - DATE PRINTED: 24/02/2



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FDC CONSTRUCTION

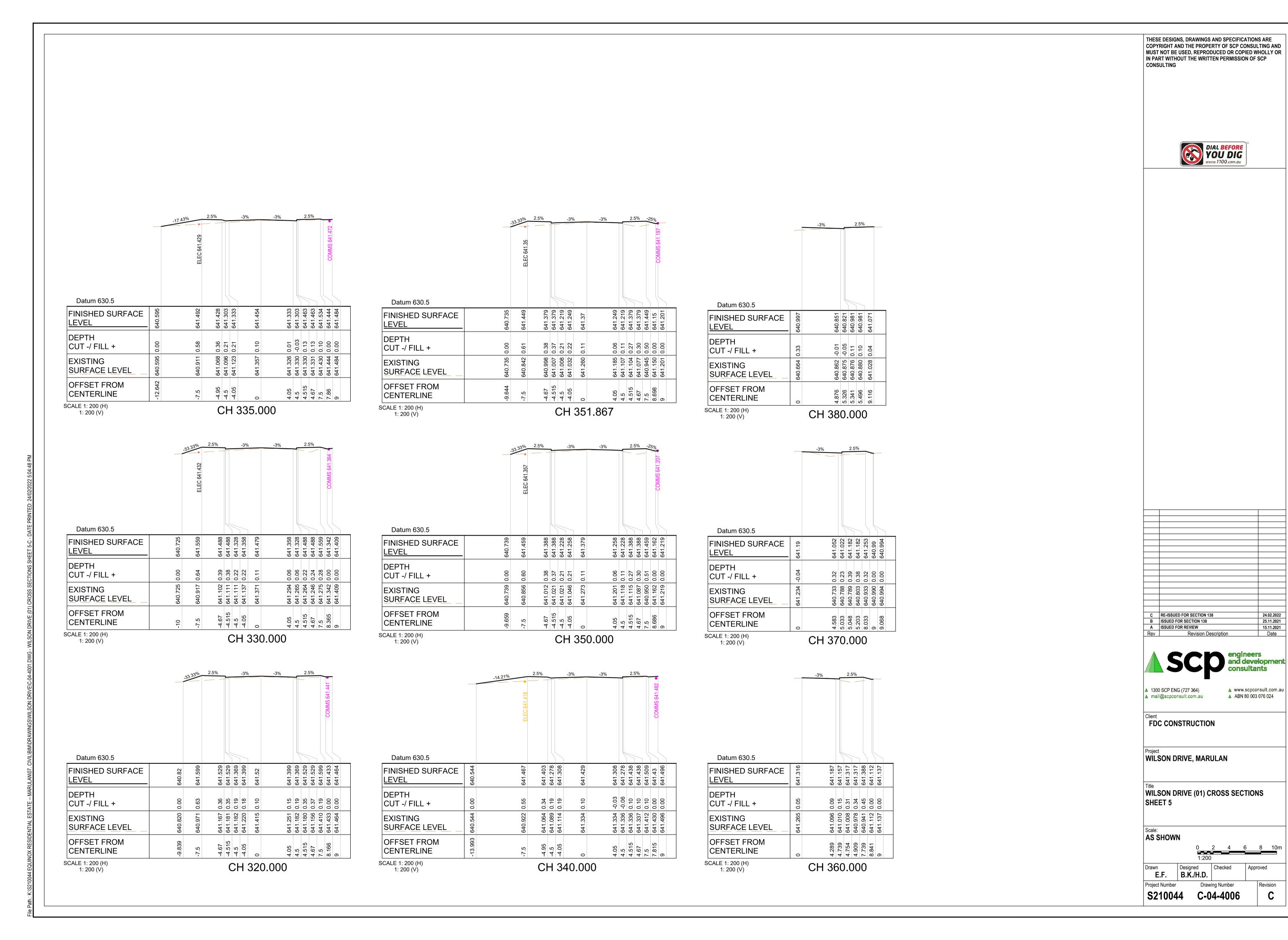
WILSON DRIVE, MARULAN

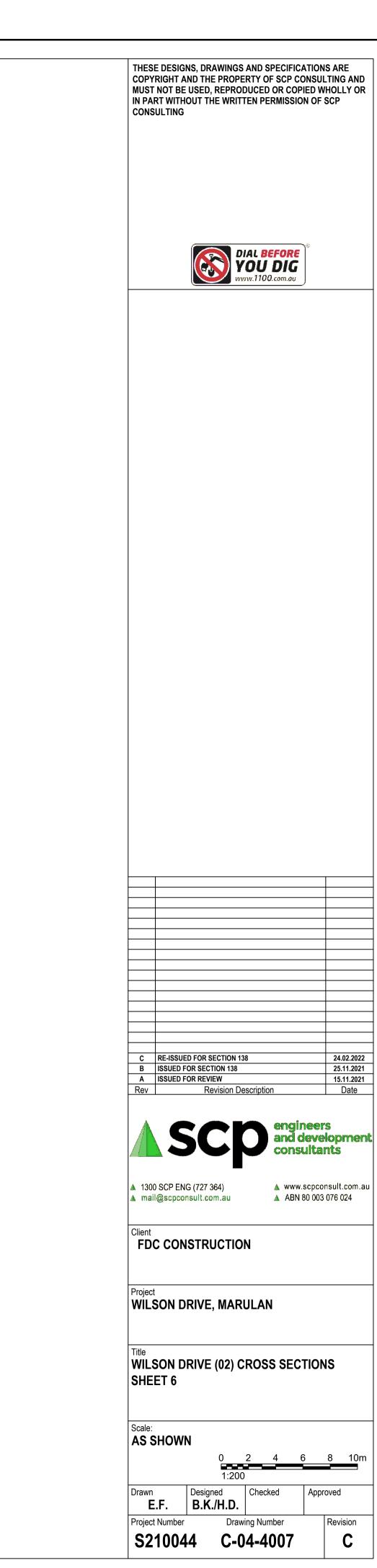
WILSON DRIVE (01) CROSS SECTIONS SHEET 4

AS SHOWN

Designed Checked Approved B.K./H.D. Project Number Drawing Number Revision

C-04-4005





Datum 635.5		-25%		-3%	6		-3%		33.	33%			
FINISHED SURFACE LEVEL	639.015	639.806	639.791	639.631	639.661	639.752	639.657	639.627	639.787	639.787	639.799	639.371	
DEPTH CUT -/ FILL +	0.00	99.0	0.53	0.33	0.25	0.03	-0.04	-0.06	0.10	0.11	0.18	0.00	
EXISTING SURFACE LEVEL	639.015	639.145	639.264	639.304	639.409	639.722	639.700	639.685	639.685	639.680	639.620	639.371	
OFFSET FROM CENTERLINE	-7.322	-4.157	-3.657	-3.487	-3.037	0	3.185	3.635	3.65	3.805	4.305	5.591	
SCALE 1: 200 (H) 1: 200 (V)			CH	1 6	0	.0	00		•		•		

Datum 637 DEPTH -0.18 -0.21 -0.05 -0.05 0.04 0.48 0.30 0.30 0.12 CUT -/ FILL + EXISTING SURFACE LEVEL OFFSET FROM CENTERLINE

SCALE 1: 200 (H) 1: 200 (V) CH 24.050

	_	<del>3</del> %	2.5	%		
Datum 637.5						
FINISHED SURFACE LEVEL	640.679	640.558	640.528 640.688	640.688	640.738	640.721
DEPTH CUT -/ FILL +	0.10	-0.17	-0.20	-0.04	0.02	0.00
EXISTING SURFACE LEVEL	640.581	640.726	640.725 640.725	640.725	640.721	640.721
OFFSET FROM CENTERLINE	0	4.05	4.5	4.67	6.67	6.719
SCALE 1: 200 (H) 1: 200 (V)		CH 20	.00	0		

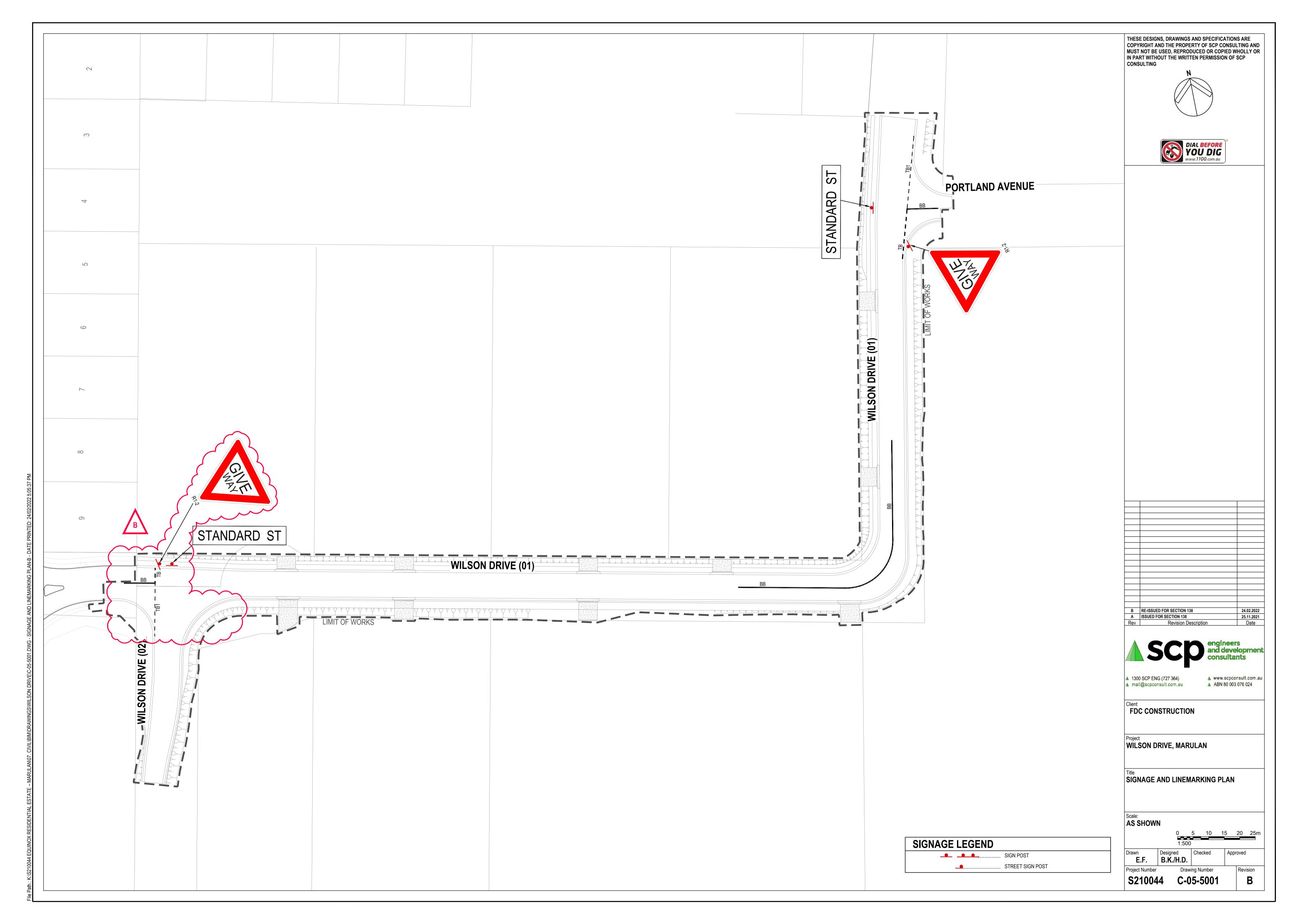
Datum 637.5	
FINISHED SURFACE LEVEL	640.742 640.62 640.59 640.75 640.75 640.78
DEPTH CUT -/ FILL +	0.05 -0.17 -0.20 -0.04 0.02 0.00
EXISTING SURFACE LEVEL	640.688 640.790 640.792 640.793 640.793 640.783
OFFSET FROM CENTERLINE	0 4.05 4.5 4.515 4.67 6.67
SCALE 1: 200 (H) 1: 200 (V)	CH 17.051

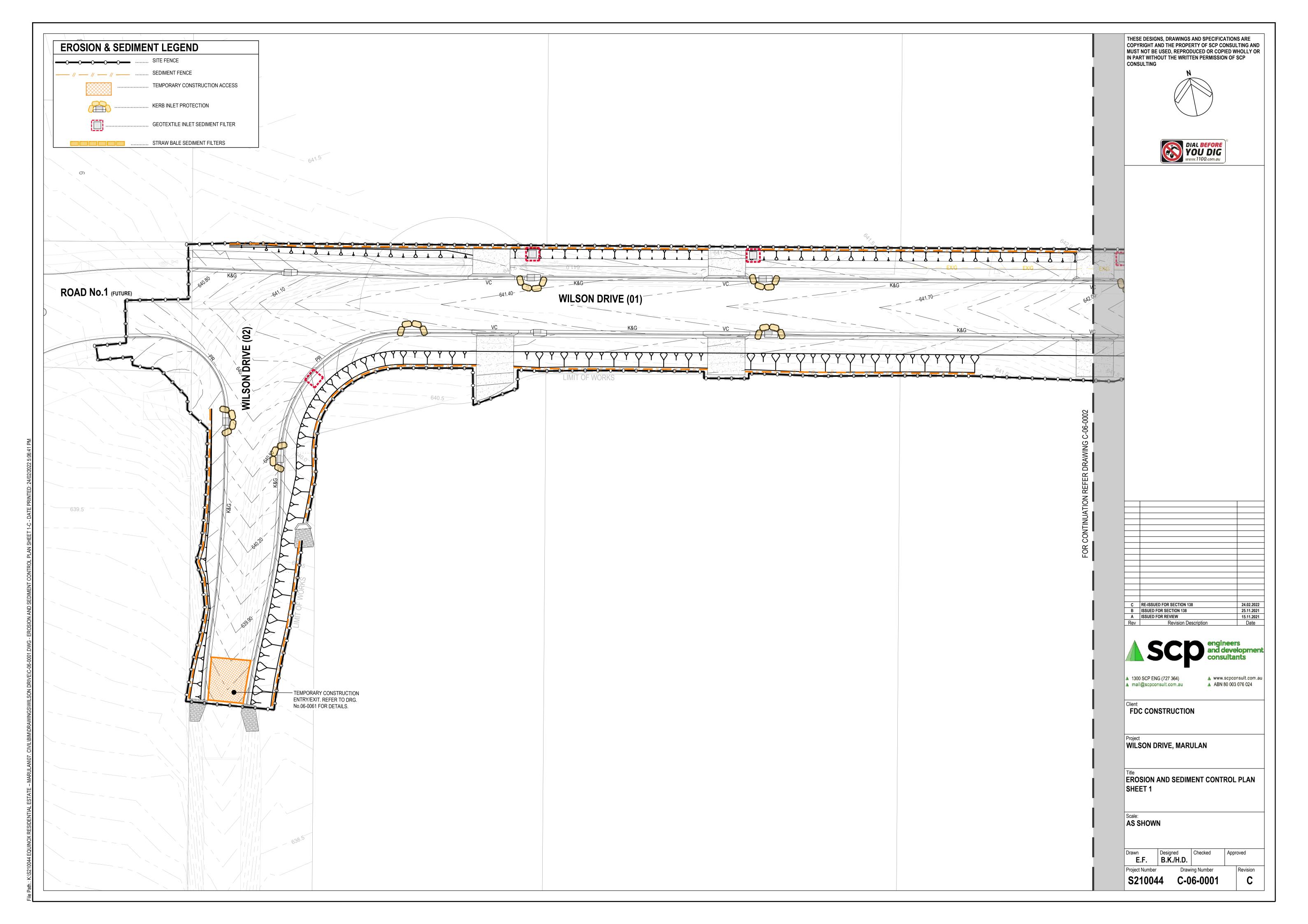
		-25%		-3	%		-3% _						
Datum 636								1					
FINISHED SURFACE LEVEL	639.212	640.007	639.992	639.992 639.832	639.862	639.968	90 000	639.83	639.99	639.99	640.003	639.906	
DEPTH CUT -/ FILL +	00.00	0.74	0.71	0.71	0.57	0.07	6	0.15	0.05	90.0	60.0	0.00	
EXISTING SURFACE LEVEL	639.212	639.271	639.280	639.284 639.284	639.293	639.897	0000	639.940	639.939	639.933	639.913	639.906	
OFFSET FROM CENTERLINE	-7.83	-4.649	-4.149	-3.994	-3.529	0	200	4.055	4.07	4.225	4.725	5.016	
SCALE 1: 200 (H) 1: 200 (V)			Cł	H 5	0	.0	00						

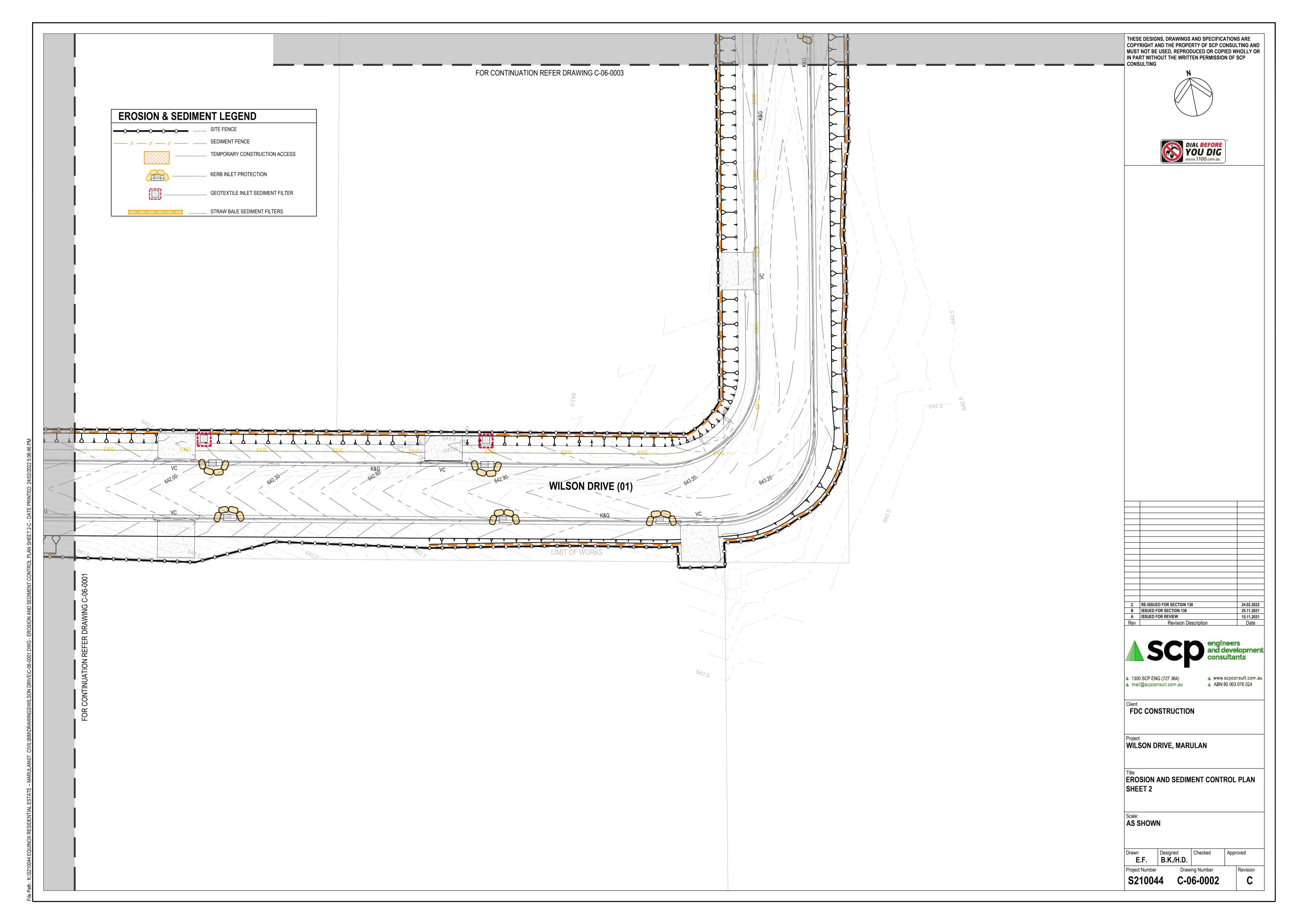
		-25%			3	3%		-3%		2.59	%			
Datum 636.5			(						1					
FINISHED SURFACE LEVEL	639.533	640.228	640.213	640.213	640.053	640.083	640.203	640.083	640.053	640.213	640.213	640.263	640.125	
DEPTH CUT -/ FILL +	00.00	0.70	69.0	69.0	0.53	0.56	0.24	-0.16	-0.18	-0.02	-0.01	0.11	0.00	
EXISTING SURFACE LEVEL	639.533	639.525	639.524	639.523	639.523	639.522	639.960	640.244	640.228	640.228	640.222	640.152	640.125	
OFFSET FROM CENTERLINE	-7.921	-5.141	-4.641	-4.486	-4.471	-4.021	0	4.025	4.475	4.49	4.645	6.645	7.059	
SCALE 1: 200 (H) 1: 200 (V)				C	CH	┨ 4	<b>4</b> 0	0.000						

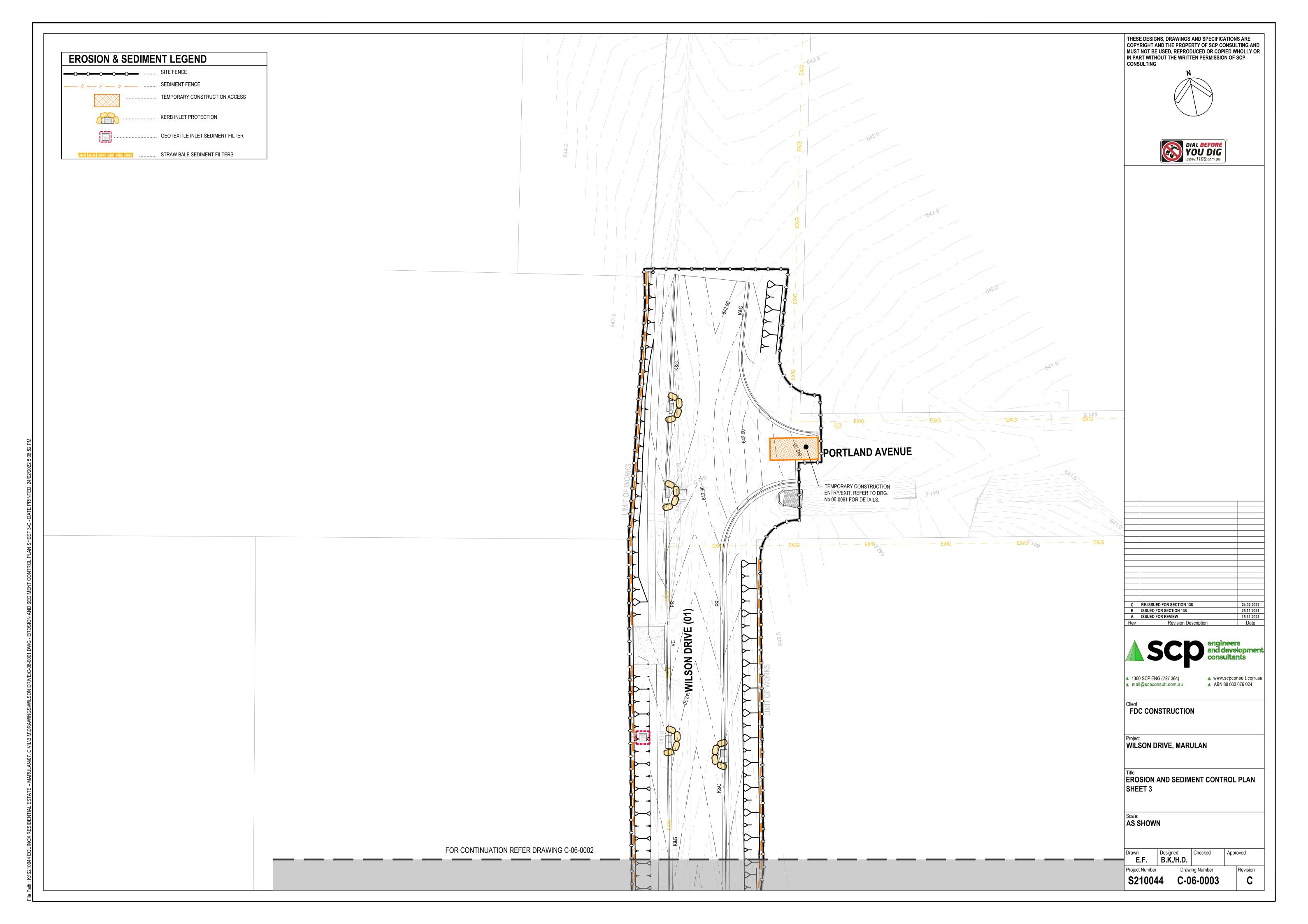
		-25%	3%			-3%	. —	-3%			2.5%	6			
Datum 636.5															
FINISHED SURFACE LEVEL	639.848	640.515	640.46	640.46	640.3	640.33	640.451		640.33	640.3	640.46	640.46	640.51	640.458	
DEPTH CUT -/ FILL +	00.0	0.72	0.67	0.67	0.51	0.53	0.28		-0.15	-0.20	-0.04	-0.05	0.05	0.00	
EXISTING SURFACE LEVEL	639.848	639.797	639.790	639.792	639.792	639.798	640.168		640.485	640.501	640.502	640.507	640.463	640.458	
OFFSET FROM CENTERLINE	-9.191	-6.52	-4.67	-4.515	-4.5	-4.05	0		4.05	4.5	4.515	4.67	29.9	6.826	
SCALE 1: 200 (H) 1: 200 (V)	CH 30.000														

		-25%			-3%	•		-3%		-33.	33%	5		
Datum 635.5			L											
FINISHED SURFACE LEVEL	638.904	639.718	639.703	639.703	639.543	639.573	639.658	639.568	639.538	639.698	639.698	639.71	639.137	
DEPTH CUT -/ FILL +	00.0	0.50	0.38	0.34	0.18	0.10	-0.00	-0.03	-0.05	0.11	0.13	0.24	0.00	
EXISTING SURFACE LEVEL	638.904	639.214	639.327	639.362	639.366	639.468	639.658	639.602	639.585	639.585	639.564	639.468	639.137	
OFFSET FROM CENTERLINE	-7.196	-3.94	-3.44	-3.285	-3.27	-2.82	0	က	3.45	3.465	3.62	4.12	5.84	
SCALE 1: 200 (H) 1: 200 (V)			C	Н	6	4	.4	10						

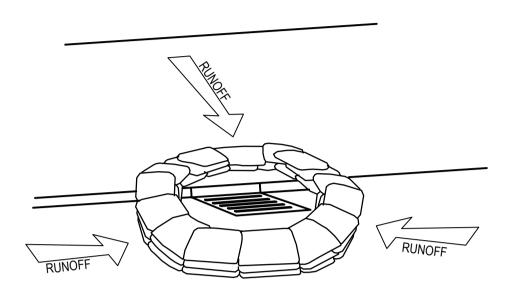








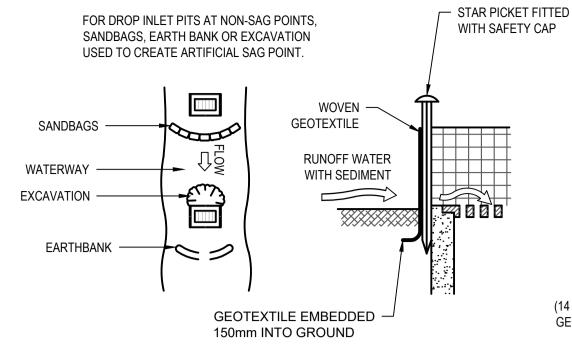
#### KERB INLET SEDIMENT FILTER (ON GRADE)

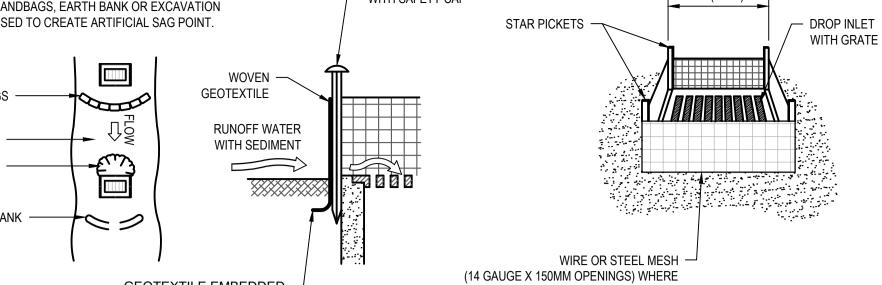


### KERB INLET SEDIMENT FILTER - SANDBAG SURROUND

#### KERB INLET SEDIMENT FILTER

- 1. REFER TO APPROVED PLANS FOR LOCATIONAND INSTALLATION DETAILS. IF THERE ARE QUESTIONS OR PROBLEMS WITH THE LOCATION, DIMENSIONS, OR METHOD OF INSTALLATION, CONTACT THE ENGINEER OR RESPONSIBLE ON-SITE OFFICER FOR ASSISTANCE.
- 2. ENSURE THAT THE INSTALLATION OF THE SEDIMENT TRAP WILL NOT CAUSE UNDESIRABLE SAFETY OR FLOODING ISSUES.
- 3. INSTALL SEDIMENT TRAP IN ACCORDANCE WITH STANDARD DRAWING SUPPLIED WITH THE APPROVED PLAN, OR AS DIRECTED BY THESITE SUPERVISOR.
- 4. ENSURE THE SEDIMENT TRAP IS CONSTRUCTED UP-SLOPE OF AN ON-GRADE KERB INLET. THE SEDIMENT TRAP MUST NOT SURROUND THE KERB INLET UNLESS SPECIFICALLY DIRECTED BY THE
- 5. ENSURE THE SEDIMENT TRAP FULLY ENCLOSES THE KERB INLET. USE APPROPRIATE SPACERS TO
- ENSURE THE SEDIMENT TRAP DOES NOT BLOCK THE SIDE-ENTRY INLET. 6. TAKE ALL NECESSARY MEASURE TO MINIMISE THE SAFETY RISK CAUSED BY THE STRUCTURE





GEOTEXTILE IS NOT SELF-SUPPORTING

**STOCKPILE** 

LARGE STONE OR FOREIGN MATERIAL.

MOISTURE CONTENT FOLLOWING THE SWMP.

1. MAINTAIN THE TRENCH FREE OF WATER AND RECOMPACT THE MATERIALS WITH

2. SELECT FILL FOLLOWING THE SWMP THAT IS FREE OF ROOTS, WOOD, ROCK

3. SPREAD THE FILL IN 100mm TO 150mm LAYERS AND COMPACT IT AT OPTIMUM

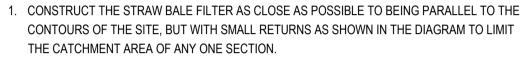
EQUIPMENT AS SPECIFIED IN THE SWMP TO 95% STANDARD PROCTOR DENSITY.

#### DROP INLET FILTERS

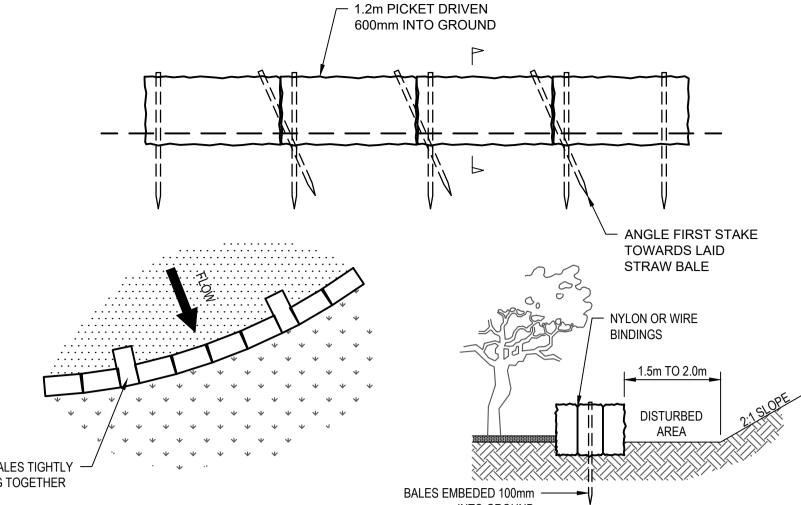
EARTH BANK

- 1. FABRICATE A SEDIMENT BARRIER MADE FROM GEOTEXTILE OF STRAW BALES. 2. FOLLOW STANDARD DRAWINGS OF STRAW BALE FILTERS AND SEDIMENT FENCES FOR INSTALLATION PROCEDURES FOR THE STRAW BALES OR GEOFABRIC. REDUCE THE PICKET SPACING TO 1m CENTERS.
- 3. IN WATERWAYS, ARTIFICIAL SAG POINTS CAN BE CREATED WITH SANDBAGS OR EARTH BANKS AS SHOWN IN THE DRAWING.
- 4. DO NOT COVER THE INLET WITH GEOTEXTILE UNLESS THE DESIGN IS ADEQUATE TO ALLOW FOR ALL WATERS TO BYPASS IT.

# STRAW BALES TIGHTLY -ABUTTING TOGETHER STRAW BALE FILTERS



- 2. PLACE BALES LENGTHWISE IN A ROW WITH ENDS TIGHTLY ABUTTING. USE STRAW TO FILL ANY GAPS BETWEEN THE BALES. THE STRAWS IN EACH BALE ATE TO BE ALIGNED PARALLEL
- 3. ENSURE THAT THE MAXIMUM HEIGHT OF THE FILTER IS ONE BALE.
- 4. EMBED EACH BALE IN THE GROUND 75mm TO 100mm AND ANCHOR WITH 1.2m STAR PICKETS OR STAKES. ANGLE THE FIRST STAR PICKET OR STAKE IN EACH BALE TOWARDS THE PREVIOUSLY LAID BALE. DRIVE THEM 600mm INTO THE GROUND AND, IF POSSIBLE, FLUSH WITH THE TOP OF THE BALES. WHERE STAR PICKETS ARE USED AND THEY PROTRUDE
- 5. WHERE A STRAW BALE FILTER IS CONSTRUCTED DOWNSLOPE FROM A DISTURBED BATTER,
- RETAINED THEY COULD REQUIRE REPLACEMENT EACH TWO TO FOUR MONTHS.





SETION A - A



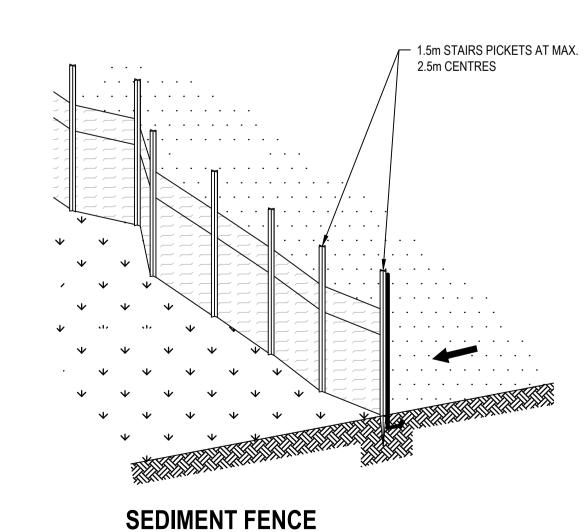
- TO THE GROUND.
- ABOVE THE BALES, ENSURE THEY ARE FITTED WITH SAFETY CAPS.
- ENSURE BALES ARE PLACED 1m TO 2m DOWNSLOPE FROM THE TOE.
- 6. ESTABLISH A MAINTENANCE PROGRAM THE ENSURES THE INTEGRITY OF THE BALES IS

#### **KERBSIDE TURF STRIP**

THE ESCP/SWMP.

- 1. INSTALL A 400mm MINIMUM WIDE ROLL OF TURF ON THE FOOTPATH NEXT TO THE KERB AND AT THE SAME LEVEL AS THE TOP OF THE KERB.
- 2. LAY 1.4m LONG TURF STRIPS NORMAL TO THE KERB EVERY 10m. 3. REHABILITATE DISTURBED SOIL BEHIND THE TURF STRIP FOLLOWING

## **SEDIMENT FENCE - PLAN** STAR PICKETS AT MAX 2.5m SPACING



#### SEDIMENT FENCE

1. CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BE PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING, TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 litres/sec IN THE DESIGN STORM EVENT, USUALLY THE 10 YEAR EVENT.

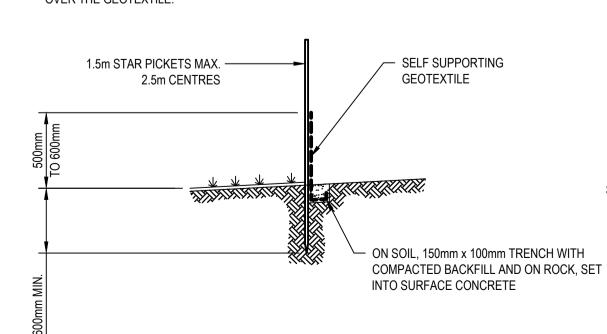
- STABILISE STOCKPILE

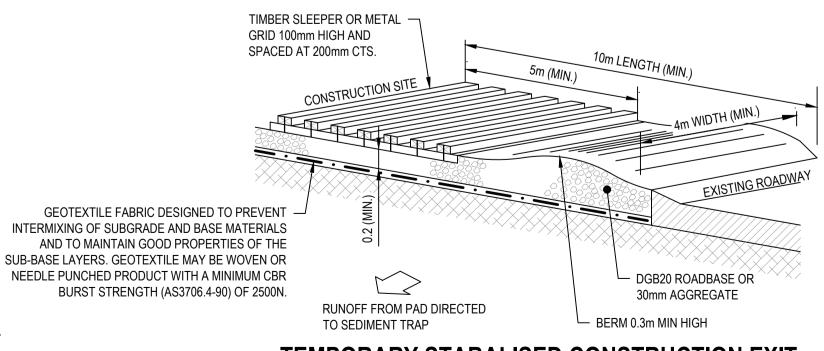
SEDIMENT FENCE -

SURFACE

**STOCKPILE** 

- 2. CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED.
- 3. DRIVE 1.5 METER LONG STAR PICKETS INTO GROUND AT 2.5 METER INTERVALS (MAX.) AT THE DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH
- 4. FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS, ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES, OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- 5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
- 6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.





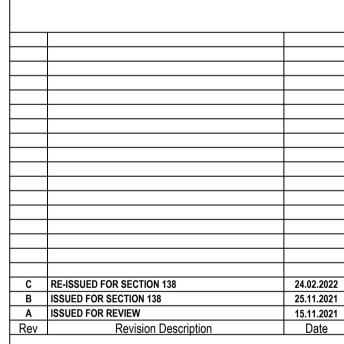
#### TEMPORARY STABALISED CONSTRUCTION EXIT

#### **CONSTRUCTION NOTES**

- 1. CONTRACTOR SHALL CONDUCT A DIAL BEFORE YOU DIG SEARCH PRIOR TO COMMENCEMENT OF ANY WORK.
- 2. ENSURE THAT ALL COUNCIL AND PUBLIC UTILITY ASSETS ARE MAINTAINED AND PROTECTED AT ALL TIMES IN THE VICINITY OF THE TEMPORARY CONSTRUCTION EXIT.
- 3. STRIP TOPSOIL AND LEVEL SITE.
- 4. COMPACT SUBGRADE.
- 5. COVER AREA WITH NEEDLE-PUNCHED GEOTEXTILE. 6. CONSTRUCT 200mm THICK PAD OVER GEOTEXTILE USING
- ROADBASE OR 30mm AGGREGATE. 7. CONSTRUCT HUMP IMMEDIATELY WITHIN BOUNDARY TO DIVERT WATER TO A SEDIMENT FENCE OR OTHER SEDIMENT
- TRAP WHERE THE SEDIMENT IS COLLECTED AND REMOVED.

#### MAINTENANCE NOTES

THE EXIT SHALL BE MAINTAINED IN A CONDITION WHICH PREVENTS TRACKING OR FLOWING OF SEDIMENT OFF THE CONSTRUCTION SITE. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL GRAVEL AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED OFF THE CONSTRUCTION SITE MUST BE REMOVED IMMEDIATELY.



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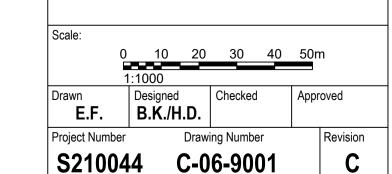
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FDC CONSTRUCTION

WILSON DRIVE, MARULAN

EROSION AND SEDIMENT CONTROL DETAILS



#### **APPENDIX B**

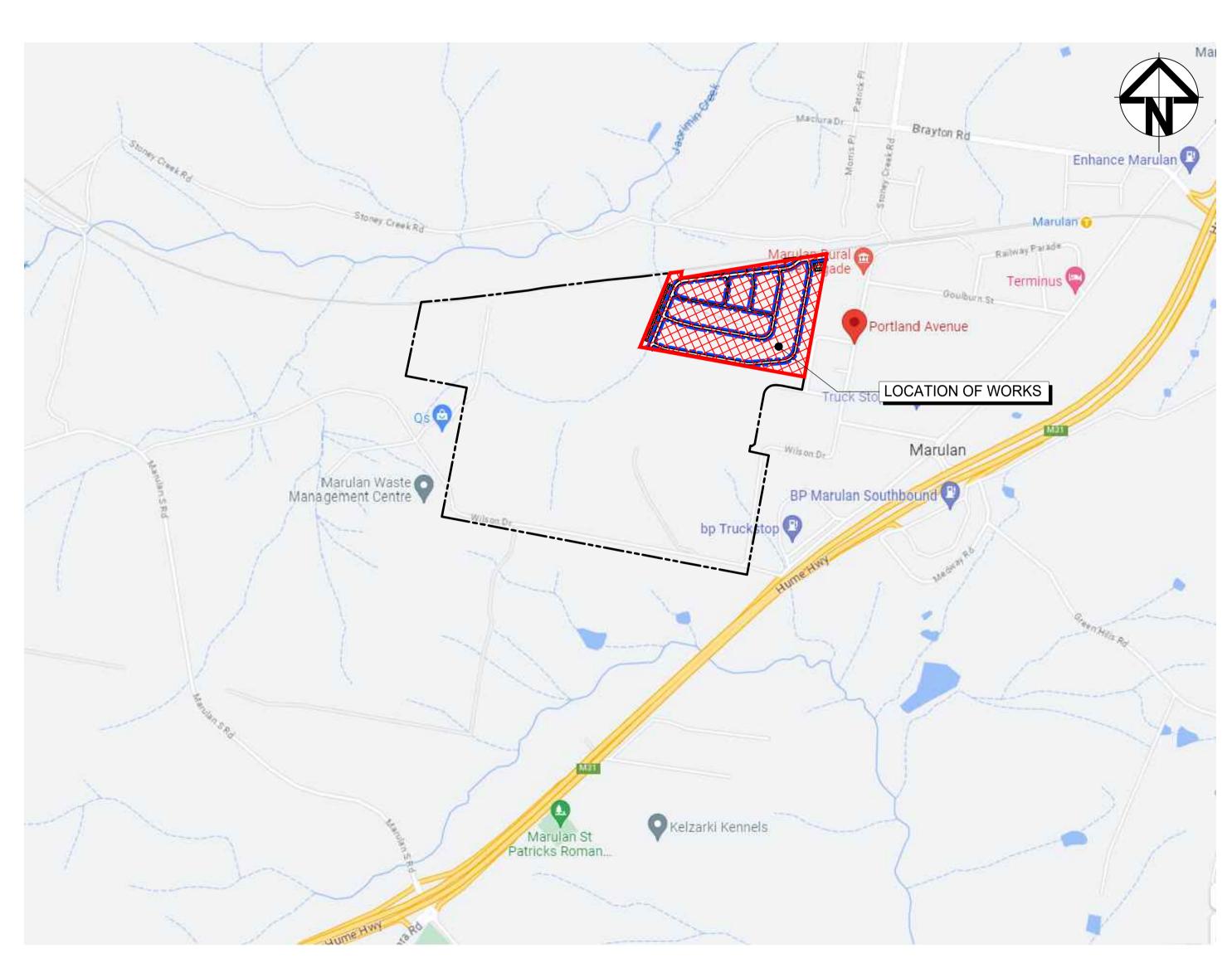
Stage 3 Subdivision Plans (Proposed)

# EQUINOX - STAGE 3

# WILSON DRIVE, MARULAN DEVELOPMENT APPLICATION, CIVIL WORKS

#### **DRAWING LIST**

GENERAL	
23-1098-C1000	COVER SHEET, DRAWING LIST AND LOCALITY PLAN
23-1098-C1001	GENERAL NOTES SHEET 1
23-1098-C1002	GENERAL NOTES SHEET 2
23-1098-C1010	GENERAL ARRANGEMENT PLAN
23-1098-C1015	DEMOLITION PLAN
23-1098-C1020	TYPICAL SECTIONS SHEET 1
23-1098-C1021	TYPICAL SECTIONS SHEET 2
23-1098-C1030	CIVIL DETAILS SHEET 1
23-1098-C1031	CIVIL DETAILS SHEET 2
BULK EARTHWORKS	
23-1098-C1040	BULK EARTHWORKS CUT/FILL PLAN
23-1098-C1041	BULK EARTHWORKS SECTION
23-1098-C1042	DEMOLITION PLAN
SITEWORKS AND STORMWATER DRAINAGE	
23-1098-C1050	SITEWORKS AND STORMWATER PLAN SHEET 1
23-1098-C1051	SITEWORKS AND STORMWATER PLAN SHEET 2
23-1098-C1052	SITEWORKS AND STORMWATER PLAN SHEET 3
ROADWORKS LONGITUDINAL SECTIONS	
23-1098-C1060	ROADWORKS LONGITUDINAL SECTIONS SHEET 1
23-1098-C1061	ROADWORKS LONGITUDINAL SECTIONS SHEET 2
23-1098-C1062	ROADWORKS LONGITUDINAL SECTIONS SHEET 3
23-1098-C1063	ROADWORKS LONGITUDINAL SECTIONS SHEET 4
23-1098-C1064	ROADWORKS LONGITUDINAL SECTIONS SHEET 5
23-1098-C1065	ROADWORKS LONGITUDINAL SECTIONS SHEET 6
23-1098-C1066	ROADWORKS LONGITUDINAL SECTIONS SHEET 7
23-1098-C1067	ROADWORKS LONGITUDINAL SECTIONS SHEET 8
23-1098-C1068	ROADWORKS LONGITUDINAL SECTIONS SHEET 9
23-1098-C1069	ROADWORKS LONGITUDINAL SECTIONS SHEET 10
STORMWATER DRAINAGE	
23-1098-C1070	STORMWATER DRAINAGE CATCHMENT PLAN (PRE-DEVELOPED)
23-1098-C1071	STORMWATER DRAINAGE CATCHMENT PLAN (POST-DEVELOPED)
23-1098-C1072	BASIN 3A PLAN
23-1098-C1073	BASIN 3A SECTION
23-1098-C1074	BASIN 3B PLAN AND SECTION
WATER AND SEWER RETICULATION	
23-1098-C1080	WATER AND SEWER RETICULATION PLAN
EROSION AND SEDIMENT CONTROL	
23-1098-C1090	EROSION AND SEDIMENT CONTROL PLAN
23-1098-C1091	EROSION AND SEDIMENT DETAILS
GOULBURN STREET EXTENSION	
23-1098-C2000	GOULBURN STREET SITEWORKS AND STORMWATER PLAN
23-1098-C2001	GOULBURN STREET TYPICAL SECTIONS



LOCALITY PLAN

		Bar Scales	Client	Scales		Drawn	NT	Project EQUINOX - STAGE 3	Civil Engineers and Project Managers
		-			NTS	Designed	BK/JC	WILSON DRIVE,	Level 7, 153 Walker Street North Sydney NSW 2060
				Grid	GDA2020	Checked	AMc	MARULAN	ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9923 1055
				Height Datum	AHD	Approved		Title	www.atl.net.au info@atl.net.au
					GD/	A2020		COVER SHEET,	PRELIMINARY ONLY A1
P1	DRAFT ISSUE 09-06-23			THIS DRAWIN	NG CANNOT BE COPIE		ED IN ANY FORM	DDAMINO LIGTAND	NOT TO BE USED FOR CONSTRUCTION Project - Drawing No. Issue
Issu	Description Date			OR USED FOR	R ANY OTHER PURPOS DED WITHOUT THE WR	SE OTHER THAN T	HAT ORIGINALL	LOCALITY PLAN	23-1098-C1000 P1

#### SITEWORKS NOTES

- 1. ORIGIN OF LEVELS:- REFER SURVEY NOTES.
- 2. CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO COMMENCEMENT OF WORK. ANY DISCREPANCIES TO BE REPORTED TO AT & L.
- 3. MAKE SMOOTH CONNECTION WITH EXISTING WORKS.
- 4. ALL TRENCH BACKFILL MATERIAL SHALL BE COMPACTED TO THE SAME DENSITY AS THE ADJACENT MATERIAL.
- 5. ALL SERVICE TRENCHES UNDER VEHICULAR PAVEMENTS SHALL BE BACKFILLED WITH SAND TO 300mm ABOVE PIPE. WHERE PIPE IS UNDER PAVEMENTS BACKFILL REMAINDER OF TRENCH TO UNDERSIDE OF PAVEMENT WITH SAND OR APPROVED GRANULAR MATERIAL COMAPACTED IN 150mm LAYERS TO MINIMUM 98% MODIFIED MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.2.1. (OR A DENSITY INDEX OF NOT LESS THAN 75)
- 6. PROVIDE 10mm WIDE EXPANSION JOINTS BETWEEN BUILDINGS AND ALL CONCRETE OR UNIT PAVEMENTS.
- 7. ASPHALTIC CONCRETE SHALL CONFORM TO R.M.S SPECIFICATION R116.
- 8. ALL BASECOURSE MATERIAL SHALL BE IGNEOUS ROCK QUARRIED MATERIAL TO COMPLY WITH R.M.S FORM 3051 (UNBOUND), R.M.S FORM 3052 (BOUND) COMPACTED TO MINIMUM 98% MODIFIED DENSITY IN ACCORDANCE WITH AS 1289 5.2.1 FREQUENCY OF COMPACTION TESTING SHALL NOT BE LESS THAN 1
- TEST PER 50m OF BASECOURSE MATERIAL PLACED.
- 9. ALL SUB-BASE COURSE MATERIAL SHALL BE IGNEOUS ROCK QUARRIED MATERIAL TO COMPLY WITH R.M.S FORM 3051, 3051.1 AND COMPACTED TO MINIMUM 95% MODIFIED DENSITY IN ACCORDANCE WITH A.S 1289 5.2.1 FREQUENCY OF COMPACTION TESTING SHALL NOT BE LESS THAN 1 TEST PER 50m OF SUB-BASE COURSE MATERIAL PLACED.
- 10. AS AN ALTERNATIVE TO THE USE OF IGNEOUS ROCK AS A SUB-BASE MATERIAL IN (9) A CERTIFIED RECYCLED CONCRETE MATERIAL COMPLYING WITH R.M.S FORM 3051 AND 3051.1 WILL BE CONSIDERED. SUBJECT TO MATERIAL SAMPLES AND APPROPRIATE CERTIFICATIONS BEING PROVIDED TO THE SATISFACTION OF AT & L.
- 11. SHOULD THE CONTRACTOR WISH TO USE A RECYCLED PRODUCT THE CONTRACTOR IS TO SEEK ACCEPTANCE OF THE PRODUCT FROM AT&L. THE PRICE DIFFERENCE BETWEEN AN IGNEOUS PRODUCT AND A RECYCLED PRODUCT SHALL BE CLEARLY INDICATED.
- 12. WHERE NOTED ON THE DRAWINGS THAT WORKS ARE TO BE CARRIED BY OTHERS, (eg. ADJUSTMENT OF SERVICES), THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CO-ORDINATION OF THESE WORKS.

#### **SURVEY NOTES**

THE EXISTING SITE CONDITIONS SHOWN ON THE FOLLOWING DRAWINGS HAVE BEEN INVESTIGATED BY LANDPARTNERS, BEING REGISTERED SURVEYORS. THE INFORMATION IS SHOWN TO PROVIDE A

BASIS FOR DESIGN. AT & L DOES NOT GUARANTEE THE ACCURACY OR COMPLETENESS OF THE SURVEY BASE OR ITS SUITABILITY AS A BASIS FOR CONSTRUCTION DRAWINGS.

SHOULD DISCREPANCIES BE ENCOUNTERED DURING CONSTRUCTION BETWEEN THE SURVEY DATA AND ACTUAL FIELD DATA, CONTACT AT & L.

THE FOLLOWING NOTES HAVE BEEN TAKEN DIRECTLY FROM THE ORIGINAL SURVEY DOCUMENTS.

#### IMPORTANT NOTE:

TREE SIZES ARE ESTIMATES ONLY. ONLY VISIBLE SERVICES HAVE BEEN LOCATED IN THIS SURVEY. SERVICE AND UTILITIES SHOWN ON PLAN HAVE BEEN LOCATED BY PHYSICAL EVIDENCE ON SITE ONLY AND MAY NOT HAVE BEEN OPENED TO VERIFY THE TYPE OF UTILITY, NEITHER EXCAVATION NOR POTHOLING HAVE BEEN

CARRIED OUT TO CONFIRM UNDERGROUND LOCATION. SERVICE DETAILS SHOULD BE CONFIRMED WITH THE RELEVANT SERVICE AUTHORITY DURING DESIGN AND PRIOR TO ANY CONSTRUCTION. ALL DIMENSIONS MUST BE VERIFIED ON SITE PRIOR TO ANY CONSTRUCTION.

THE POSITION OF SURVEYED DATA HAS BEEN LOCATED AND IS SHOWN TO TOPOGRAPHIC ACCURACIES. IF CLEARANCES TO BOUNDARIES OR OTHER FEATURES ARE CRITICAL AND DIMENSIONS ARE NOT SHOWN FURTHER SURVEY MAY BE REQUIRED.

THE TITLE BOUNDARIES SHOWN HEREON WERE NOT MARKED AT THE TIME OF SURVEY AND HAVE BEEN DETERMINED BY PLAN DIMENSIONS ONLY AND NOT BY FIELD SURVEY.

ANY CONSTRUCTION ON OR NEAR BOUNDARIES WILL REQUIRE FURTHER SURVEY IN ORDER THAT MARKS DEFINING BOUNDARIES CAN BE PLACED.

#### **EXISTING UNDERGROUND SERVICES**

THE LOCATIONS OF UNDERGROUND SERVICES SHOWN IN THIS SET OF DRAWINGS HAVE BEEN PLOTTED FROM SURVEY INFORMATION AND SERVICE AUTHORITY INFORMATION. THE SERVICE INFORMATION HAS BEEN PREPARED ONLY TO SHOW THE APPROXIMATE POSITIONS OF ANY KNOWN SERVICES AND MAY NOT BE AS CONSTRUCTED OR ACCURATE.

AT & L CAN NOT GUARANTEE THAT THE SERVICES INFORMATION SHOWN ON THESE DRAWINGS ACCURATELY INDICATES THE PRESENCE OR ABSENCE OF SERVICES OR THEIR LOCATION AND WILL ACCEPT NO LIABILITY FOR INACCURACIES IN THE SERVICES INFORMATION SHOWN FROM ANY CAUSE WHATSOEVER.

CONTRACTORS SHALL TAKE DUE CARE WHEN EXCAVATING ONSITE INCLUDING HAND EXCAVATION WHERE NECESSARY.

CONTRACTORS ARE TO CONTACT THE RELEVANT SERVICE AUTHORITY PRIOR TO COMMENCEMENT OF EXCAVATION WORKS.

CONTRACTORS ARE TO UNDERTAKE A SERVICES SEARCH, PRIOR TO COMMENCEMENT OF WORKS ON SITE. SEARCH RESULTS ARE TO BE KEPT ON SITE AT ALL TIMES.

#### **CONCRETE NOTES**

1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600 CURRENT EDITION WITH AMENDMENTS. EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.

#### 2. CONCRETE QUALITY

ALL REQUIREMENTS OF THE CURRENT ACSE CONCRETE SPECIFICATION DOCUMENT 1 SHALL APPLY TO THE FORMWORK, REINFORCEMENT AND CONCRETE UNLESS NOTED OTHERWISE.

ELEMENT	AS 3600 F'c MPa	SPECIFIED	NOMINAL
	AT 28 DAYS	SLUMP	AGG. SIZE
VEHICULAR BASE KERBS, PATHS, AND PITS	32 25	60 80	20 20

- CEMENT TYPE SHALL BE (ACSE SPECIFICATION) TYPE SL - PROJECT CONTROL TESTING SHALL BE CARRIED OUT IN ACCORDANCE WITH AS 1379.

3. NO ADMIXTURES SHALL BE USED IN CONCRETE UNLESS APPROVED IN WRITING BY AT & L.

- 4. CLEAR CONCRETE COVER TO ALL REINFORCEMENT FOR DURABILITY SHALL BE 40mm TOP AND 70mm FOR EXTERNAL EDGES UNLESS NOTED OTHERWISE.
- 5. ALL REINFORCEMENT SHALL BE FIRMLY SUPPORTED ON MILD STEEL PLASTIC TIPPED CHAIRS, PLASTIC CHAIRS OR CONCRETE CHAIRS AT NOT GREATER THAN 1m CENTRES BOTH WAYS. BARS SHALL BE TIED AT ALTERNATE INTERSECTIONS.
- 6. THE FINISHED CONCRETE SHALL BE A DENSE HOMOGENEOUS MASS, COMPLETELY FILLING THE FORMWORK, THOROUGHLY EMBEDDING THE REINFORCEMENT AND FREE OF STONE POCKETS. ALL CONCRETE INCLUDING SLABS ON GROUND AND FOOTINGS SHALL BE COMPACTED AND CURED IN ACCORDANCE WITH R.M.S SPECIFICATION R83.
- 7. REINFORCEMENT SYMBOLS:
- N DENOTES GRADE 450 N BARS TO AS 1302 GRADE N
- R DENOTES 230 R HOT ROLLED PLAIN BARS TO AS 1302 SL DENOTES HARD-DRAWN WIRE REINFORCING FABRIC TO AS 1304
- NUMBER OF BARS IN GROUP THE BAR GRADE AND TYPE

#### 17 N 20 250

NOMINAL BAR SIZE IN mm U SPACING IN mm

THE FIGURE FOLLOWING THE FABRIC SYMBOL SL IS THE REFERANCE NUMBER FOR FABRIC TO AS 1304.

8. FABRIC SHALL BE LAPPED IN ACCORDANCE WITH THE FOLLOWING

\_LAP TWO WIRES

#### KERBING NOTES

1. ALL CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 25MPa U.N.O IN REINFORCED CONCRETE NOTES.

2. ALL KERBS, GUTTERS, DISH DRAINS AND CROSSINGS TO BE CONSTRUCTED ON MIN. 100mm GRANULAR BASECOURSE COMPACTED TO MINIMUM 95% MODIFIED DRY DENSITY (AS 1289 5.2.1).

3. EXPANSION JOINTS (E.J) TO BE FORMED FROM 10mm COMPRESSIBLE CORK FILLER BOARD FOR THE FULL DEPTH OF THE SECTION AND CUT TO PROFILE. EXPANSION JOINTS TO BE LOCATED AT DRAINAGE PITS, ON TANGENT POINTS OF CURVES AND ELSEWHERE AT MAX 12m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE EXPANSION JOINTS ARE TO MATCH THE JOINT LOCATIONS IN THE SLABS.

4. WEAKENED PLANE JOINTS TO BE MIN 3mm WIDE AND LOCATED AT 3m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE WEAKENED PLANE JOINTS ARE TO MATCH THE JOINT LOCATIONS IN THE SLABS.

5. BROOMED FINISH TO ALL RAMPED AND VEHICULAR CROSSINGS. ALL OTHER KERBING OR DISH DRAINS TO BE STEEL FLOAT FINISHED.

6. IN THE REPLACEMENT OF KERB AND GUTTER:-EXISTING ROAD PAVEMENT IS TO BE SAWCUT 900mm U.N.O FROM THE LIP OF GUTTER. UPON COMPLETION OF THE NEW KERB AND GUTTER NEW BASECOURSE AND SURFACE TO BE LAID 900mm WIDE U.N.O.

EXISTING ALLOTMENT DRAINAGE PIPES ARE TO BE BUILT INTO THE NEW KERB AND GUTTER WITH 100mm DIA HOLE.

EXISTING KERB AND GUTTER IS TO BE COMPLETELY REMOVED WHERE NEW KERB AND GUTTER IS SHOWN.

#### STORMWATER DRAINAGE NOTES

. STORMWATER DESIGN CRITERIA: (A) AVERAGE RECURRENCE INTERVAL: 1:100 YEARS MAJOR STORM (OVERLAND FLOW) 1:20 YEARS MINOR STORM (PIPED NETWORK) (B) RAINFALL INTENSITIES:

TIME OF CONCENTRATION:5 MINUTES 1:100 YEARS= 182 mm/hr 1:20 YEARS= 138 mm/hr (C) RUNOFF COEFFICIENTS:
ROOF AREAS:
C 100 =1.0

EXTERNAL PAVEMENTS: C 100 =0.9

2. PIPES 300 DIA. AND LARGER TO BE REINFORCED CONCRETE CLASS '3' APPROVED SPIGOT AND SOCKET WITH RUBBER RING JOINTS. U.N.O. ALL ROAD CROSSINGS TO BE CLASS "4" U.N.O.

3. PIPES UP TO 300 DIA SHALL BE SEWER GRADE uPVC WITH SOLVENT WELDED JOINTS.

. EQUIVALENT STRENGTH VCP OR FRC PIPES MAY BE USED, SUBJECT TO THE APPROVAL.

5. PIPES TO BE INSTALLED TO TYPE HS1 SUPPORT IN ACCORDANCE WITH AS 3725 (2007) IN ALL CASES BACKFILL TRENCH WITH SAND TO 300mm ABOVE PIPE. WHERE PIPE IS UNDER PAVEMENTS BACKFILL REMAINDER OF TRENCH TO UNDERSIDE OF PAVEMENT WITH SAND OR APPROVED GRANULAR MATERIAL COMPACTED IN 150mm LAYERS TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.2.1 (OR A DENSITY INDEX OF NOT LESS THAN 75)

ALL INTERNAL WORKS WITHIN PROPERTY BOUNDARIES ARE TO COMPLY WITH THE REQUIREMENTS OF AS 3500.3 (2021).

7. PRECAST PITS MAY BE USED EXTERNAL TO THE BUILDING SUBJECT TO APPROVAL BY AT & L.

ENLARGERS, CONNECTIONS AND JUNCTIONS TO BE PREFABRICATED FITTINGS WHERE PIPES ARE LESS THAN 300 DIA.

9. WHERE SUBSOIL DRAINS PASS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS, UNSLOTTED uPVC SEWER GRADE PIPE IS TO BE USED.

10. CARE IS TO BE TAKEN WITH LEVELS OF STORMWATER LINES. GRADES

SHOWN ARE NOT TO BE REDUCED WITHOUT APPROVAL. 1. GRATES AND COVERS SHALL CONFORM TO AS 3996.

12. AT ALL TIMES DURING CONSTRUCTION OF STORMWATER PITS, ADEQUATE SAFETY PROCEDURES SHALL BE TAKEN TO ENSURE AGAINST THE POSSIBILITY OF PERSONNEL FALLING DOWN PITS.

13. ALL EXISTING STORMWATER DRAINAGE LINES AND PITS THAT ARE TO REMAIN ARE TO BE INSPECTED AND CLEANED, DURING THIS PROCESS ANY PART OF THE STORMWATER DRAINAGE SYSTEM THAT WARRANTS REPAIR SHALL BE REPORTED TO THE SUPERINTENDENT/ENGINEER FOR FURTHER DIRECTIONS.

#### FINISHED SURFACE LEVELS

1. ALL FINISHED SURFACE LEVELS ARE ±1000mm U.N.O.

#### **EROSION AND SEDIMENT CONTROL NOTES**

#### GENERAL INSTRUCTIONS

1. THE SITE SUPERINTENDENT/ENGINEER WILL ENSURE THAT ALL SOIL AND WATER MANAGEMENT WORKS ARE LOCATED AS DOCUMENTED.

2. ALL WORK SHALL BE GENERALLY CARRIED OUT IN ACCORDANCE WITH a. LOCAL AUTHORITY REQUIREMENTS h EPA REQUIREMENTS

c. NSW DEPARTMENT OF HOUSING MANUAL "MANAGING URBAN STORMWATER, SOILS AND CONSTRUCTION", 4th EDITION, MARCH

3. MAINTAIN THE EROSION CONTROL DEVICES TO THE SATISFACTION OF THE SUPERINTENDENT AND THE LOCAL AUTHORITY.

4. WHEN STORMWATER PITS ARE CONSTRUCTED, PREVENT SITE RUNOFF ENTERING UNLESS SEDIMENT FENCES ARE ERECTED AROUND PITS.

5. CONTRACTOR IS TO ENSURE ALL EROSION & SEDIMENT CONTROL DEVICES ARE MAINTAINED IN GOOD WORKING ORDER AND OPERATE EFFECTIVELY, REPAIRS AND OR MAINTENANCE SHALL BE UNDERTAKEN AS REQUIRED, PARTICULARLY FOLLOWING STORM EVENTS.

#### LAND DISTURBANCE

6. WHERE PRACTICAL, THE SOIL EROSION HAZARD ON THE SITE WILL BE KEPT AS LOW AS POSSIBLE. TO THIS END, WORKS SHOULD BE UNDERTAKEN IN THE FOLLOWING SEQUENCE:

(A) INSTALL A WIND FENCE ALONG THE BOUNDARIES AS SHOWN ON PLAN. REFER DETAIL.

(B) INSTALL A SEDIMENT FENCE ALONG THE BOUNDARIES AS SHOWN ON PLAN. REFER DETAIL.

(C) CONSTRUCT STABILISED CONSTRUCTION ENTRANCE TO LOCATION AS DETERMINED BY SUPERINTENDENT/ENGINEER. REFER

(D) INSTALL SEDIMENT BASIN AS SHOWN ON PLAN

(E) INSTALL SEDIMENT TRAPS AS SHOWN ON PLAN.

(F) UNDERTAKE SITE DEVELOPMENT WORKS IN ACCORDANCE WITH THE ENGINEERING PLANS. WHERE POSSIBLE, PHASE DEVELOPMENT SO THAT LAND DISTURBANCE IS CONFINED TO AREAS OF WORKABLE SIZE.

#### **EROSION CONTROL**

7. DURING WINDY WEATHER, LARGE, UNPROTECTED AREAS WILL BE KEPT MOIST (NOT WET) BY SPRINKLING WITH WATER TO KEEP DUST UNDER

8. FINAL SITE LANDSCAPING WILL BE UNDERTAKEN AS SOON AS POSSIBLE AND WITHIN 20 WORKING DAYS FROM COMPLETION OF CONSTRUCTION ACTIVITIES.

#### SEDIMENT CONTROL

9. STOCKPILES WILL NOT BE LOCATED WITHIN 2 METRES OF HAZARD AREAS, INCLUDING LIKELY AREAS OF CONCENTRATED OR HIGH VELOCITY FLOWS SUCH AS WATERWAYS. WHERE THEY ARE BETWEEN 2 AND 5 METRES FROM SUCH AREAS, SPECIAL SEDIMENT CONTROL MEASURES SHOULD BE TAKEN TO MINIMISE POSSIBLE POLLUTION TO DOWNSLOPE WATERS, E.G. THROUGH INSTALLATION OF SEDIMENT

10. ANY SAND USED IN THE CONCRETE CURING PROCESS (SPREAD OVER THE SURFACE) WILL BE REMOVED AS SOON AS POSSIBLE AND WITHIN 10 WORKING DAYS FROM PLACEMENT.

11. WATER WILL BE PREVENTED FROM ENTERING THE PERMANENT DRAINAGE SYSTEM UNLESS IT IS RELATIVELY SEDIMENT FREE, I.E. THE CATCHMENT AREA HAS BEEN PERMANENTLY LANDSCAPED AND/OR ANY LIKELY SEDIMENT HAS BEEN FILTERED THROUGH AN APPROVED

12. TEMPORARY SOIL AND WATER MANAGEMENT STRUCTURES WILL BE REMOVED ONLY AFTER THE LANDS THEY ARE PROTECTING ARE REHABILITATED.

#### OTHER MATTERS

13. ACCEPTABLE RECEPTORS WILL BE PROVIDED FOR CONCRETE AND MORTAR SLURRIES, PAINTS, ACID WASHINGS, LIGHT-WEIGHT WASTE MATERIALS AND LITTER.

14. ANY EXISTING TREES WHICH FORM PART OF THE FINAL LANDSCAPING PLAN WILL BE PROTECTED FROM CONSTRUCTION ACTIVITIES BY:

(A) PROTECTING THEM WITH BARRIER FENCING OR SIMILAR MATERIALS INSTALLED OUTSIDE THE DRIP LINE

(B) ENSURING THAT NOTHING IS NAILED TO THEM

(C) PROHIBITING PAVING, GRADING, SEDIMENT WASH OR PLACING OF STOCKPILES WITHIN THE DRIP LINE EXCEPT UNDER THE FOLLOWING CONDITIONS.

(I) ENCROACHMENT ONLY OCCURS ON ONE SIDE AND NO CLOSER TO THE TRUNK THAN EITHER 1.5 METRES OR HALF THE DISTANCE BETWEEN THE OUTER EDGE OF THE DRIP LINE AND THE TRUNK, WHICH EVER IS THE GREATER

(II) A DRAINAGE SYSTEM THAT ALLOWS AIR AND WATER TO CIRCULATE THROUGH THE ROOT ZONE (E.G. A GRAVEL BED) IS PLACED UNDER ALL FILL LAYERS OF MORE THAN 300 MILLIMETRES DEPTH

(III) CARE IS TAKEN NOT TO CUT ROOTS UNNECESSARILY NOR TO COMPACT THE SOIL AROUND THEM.

#### **EROSION AND SEDIMENT CONTROL NOTES**

#### STAGING

SUITABLE EROSION AND SEDIMENT CONTROLS SHALL BE DESIGNED, PROVIDED AND MAINTAINED BY THE CONTRACTOR THROUGHOUT ALL STAGES OF WORKS, INCLUDING AT COMPLETION OF THE BULK EARTHWORKS WHERE SHOWN ON AT&L DRAWINGS OR WHERE DIRECTED BY THE SUPERINTENDENT OR PENRITH CITY COUNCIL'S

ENGINEERS. SEDIMENT AND EROSION CONTROLS ARE TO BE DESIGNED AND DOCUMENTED BY A SUITABLY QUALIFIED EXPERT ENGAGED BY THE CONTRACTOR AND APPROVED AS PART OF THE CONSTRUCTION ENVIRONMENTAL MANAGEMENT PLAN PRIOR TO THE COMMENCEMENT OF CONSTRUCTION. SUCH CONTROLS SHALL BE IN ACCORDANCE WITH THE RELEVANT REQUIREMENTS IN THE LATEST VERSION OF THE MANAGING URBAN STORMWATER: SOILS AND CONSTRUCTION GUIDELINE (LANDCOM).



CONTRACTOR SHALL OBTAIN ALL CURRENT

SERVICE AUTHORITY PLANS PRIOR TO

COMMENCEMENT OF WORK





Client

Scales NT NTS Designed BK/JC Checked GDA2020 AMc Height Datum Approved AHD

**GDA2020** 

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**GENERAL NOTES** SHEET 1

**EQUINOX - STAGE 3** 

WILSON DRIVE,

**MARULAN** 

Fax: 02 9923 1055 www.atl.net.au info@atl.net.au PRELIMINARY ONLY

NOT TO BE USED FOR CONSTRUCTION Project - Drawing No. Issue 23-1098-C1001

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Civil Engineers and Project Managers

09-06-23

Date

Bar Scales

P1 DRAFT ISSUE

100mm on Original

Description

Level 7, 153 Walker Street

North Sydney NSW 2060 ABN 96 130 882 405

Tel: 02 9439 1777

#### GOULBURN MULWAREE SHIRE COUNCIL

STANDARD NOTES FOR SUBDIVISION AND WORKS IN ROAD RESERVES

#### NOTIFICATION AND INSPECTIONS

- 1. THE SUPERVISOR SHALL PROVIDE THE COUNCIL ENGINEER WITH 7-DAYS NOTICE PRIOR TO THE COMMENCEMENT
- 2. AT LEAST 48-HOURS NOTICE TO COUNCIL'S DEVELOPMENT AND ASSET ENGINEER (PHONE NUMBER 48234444) SHOULD BE GIVEN FOR INSPECTIONS. FAILURE TO NOTIFY THE NEED FOR AN INSPECTION MAY LEAD TO THE PORTION OF THE WORK NOT BEING APPROVED BY THE COUNCIL.
- THE SUPERVISOR SHALL NOT PROCEED TO THE NEXT STAGE UNTIL THE COUNCIL ENGINEER HAS APPROVED THE
- 4. IN CASES WHERE A PORTION OF THE WORKS IS NOT APPROVED, NO FURTHER WORK MAY PROCEED WHEREBY THE FAILED PORTION OF THE WORK MAY BECOME INCORPORATED INTO ANY NEW WORKS.
- 5. THE SUPERVISOR MUST ENSURE THAT INSPECTIONS ARE CARRIED OUT AT THE FOLLOWING STAGES OF CONSTRUCTION. COUNCIL WILL ASSUME THE ROLE OF THE PRINCIPAL CERTIFYING/ROADS AUTHORITY. ADDITIONAL INSPECTIONS MAY BE CARRIED OUT AS DIRECTED BY COUNCIL.
- INSPECTIONS BY CERTIFYING AUTHORITY ARE REQUIRED AT THE FOLLOWING STAGES AND THE WORKS APPROVED PRIOR TO CONTINUANCE OF ANY FUTURE WORK:

#### EROSION AND SEDIMENT CONTROL;

a. IMPLEMENTATION OF EROSION AND SEDIMENT CONTROL MEASURES BEFORE CONSTRUCTION;

#### DRAINAGE;

- TRENCH EXCAVATED, BEDDING MATERIAL PLACED AND PIPES/CONDUITS (SHOWING PIPE CLASS) LAID PRIOR TO BACKFILLING AND NON-COHESIVE GRANULAR BACKFILLING MATERIAL UP TO HAUNCH;
- FILTER MATERIAL PLACED IN SUBSOIL DRAINS PRIOR TO BACKFILLING;
- PIT WALLS, WING WALLS AND HEAD WALLS WITH REINFORCEMENT AND PLACE PRIOR TO CASTING;
- CONNECTION TO EXISTING SYSTEM PRIOR TO BACKFILLING; e. CHANNEL/WATERCOURSE TAIL OUT WORKS AFTER CONSTRUCTION;
- PAVEMENT CONSTRUCTION;
- a. PRIOR TO WORKS COMMENCING APPROVAL OF THE MATERIAL USED IN ROAD PAVEMENTS SHALL BE OBTAINED FROM COUNCIL, INCLUDING THE SOURCE OF THE MATERIAL AND MOST RECENT MATERIAL TESTING. SUB-GRADE ROLLER TEST (A VISUAL CHECK AND ROLLER TEST WITH 10T SMOOTH DRUM ROLLER) OR IN
- ACCORDANCE WITH TEST METHOD T198. SUB BASE ROLLER TEST AT KERB ALIGNMENT (A VISUAL CHECK AND ROLLER TEST WITH 10T SMOOTH DRUM
- ROLLER) OR IN ACCORDANCE WITH TEST METHOD T198. BASE COURSE BENKELMAN BEAN TEST CARRIED OUT BY NATA REGISTERED GEOTECHNICAL ENGINEER AT 10M
- WEARING SURFACE

INTERVALS.

- APPLICATION OF THE WEARING SURFACE SHALL NOT OCCUR WITHOUT THE APPROVAL OF COUNCIL. THE FOLLOWING SHALL BE SUBMITTED TO COUNCIL TO DEMONSTRATE COMPLIANCE WITH COUNCIL'S
- SPECIFICATION PRIOR TO APPROVAL TO SEAL i) COMPACTION TEST FOR SUBGRADE, SUB BASE AND BASE
- PRIOR TO THE PLANTING OF ANY PLANTS, APPROVAL SHALL BE OBTAINED BY COUNCIL. ii) MATERIALS TESTING AND SAMPLING FOR SUB BASE AND BASE
- iii) BENKELMAN BEAM TEST RESULTS
- b. VISUAL INSPECTION OF THE INITIAL SEAL/PRIMER SEAL PRIOR TO APPLICATION OF THE FINAL WEARING
- c. CORE DRILLS OF THE WEARING SURFACE WILL BE UNDERTAKEN BY COUNCIL
- FOOTPATH, OFF-ROAD CYCLEWAY AND SHARED WAY WORKS;
- CONCRETE FOOTPATHS, CYCLEWAYS, SHARED WAYS AND PATHWAYS FORMED AND REINFORCEMENT IN PLACE PRIOR TO PLACEMENT OF CONCRETE:
- 6) ON-SITE DETENTION SYSTEM (OSD):
- STEEL AND FORMWORK FOR TANK/PIT PRIOR TO PLACEMENT OF CONCRETE;
- PIPES UPSTREAM/DOWNSTREAM OF TANK/PIT PRIOR TO BACKFILLING;
- INSTALLATION OF STORMWATER QUALITY DEVICES;
- PRIOR TO WORKS COMMENCING APPROVAL OF THE MATERIAL USED IN STORMWATER QUALITY DEVICES SUCH AS STORMWATER BIO FILTRATION SYSTEMS SHALL BE OBTAINED FROM COUNCIL, INCLUDING THE SOURCE OF THE MATERIAL AND MOST RECENT MATERIAL TESTING.
- INSPECTION OF EACH LAYER IN THE STORMWATER QUALITY DEVICE PRIOR TO PLACEMENT OF THE NEXT
- PRIOR TO THE PLANTING OF ANY PLANTS, APPROVAL SHALL BE OBTAINED BY COUNCIL.

#### GENERAL NOTES

P1 DRAFT ISSUE

- 1. ALL WORK TO BE CARRIED OUT IN ACCORDANCE WITH GOULBURN MULWAREE COUNCIL'S ENGINEERING DESIGN AND ENGINEERING CONSTRUCTION SPECIFICATIONS AND TO THE REQUIREMENTS OF THE CERTIFYING
- AUTHORITY NO TREES ARE TO BE REMOVED UNLESS APPROVAL IS GRANTED BY COUNCIL.
- MAKE SMOOTH JUNCTIONS WITH EXISTING WORKS.

Description

- 4. NO WORK IS TO BE CARRIED OUT ON COUNCIL PROPERTY OR ADJOINING PROPERTIES WITHOUT THE WRITTEN PERMISSION FROM THE OWNER/S.
- 5. VEHICULAR ACCESS AND ALL UTILITIES/SERVICES ARE TO BE MAINTAINED AT ALL TIMES TO ADJOINING
- PROPERTIES AFFECTED BY CONSTRUCTION. 6. ALL RUBBISH, BUILDINGS, SHEDS AND FENCES TO BE REMOVED TO SATISFACTION OF COUNCIL'S ENGINEER.
- 7. SHOULD ANY UNEXPECTED FINDS OF CONTAMINATION BE IDENTIFIED ON THE SITE, THE UNEXPECTED FINDS
- PROCEDURES AND PROTOCOLS SHOULD BE FOLLOWED. 8. IF ANY PREVIOUSLY UNRECORDED OR UNANTICIPATED ABORIGINAL OBJECTS ARE ENCOUNTERED DURING THE DEVELOPMENT ACTIVITIES - WORK MUST CEASE IMMEDIATELY AND THE NATURE AND EXTENT OF THE OBJECTS ASSESSED. THE APPLICANT/OWNER/BUILDER SHALL CEASE WORK IMMEDIATELY IN THE VICINITY OF THE ARTEFACT/S OR OBJECT/S AND CONTACT THE OFFICE OF ENVIRONMENT & HERITAGE AT QUEANBEYAN (COUNTRY, CULTURE & HERITAGE) AND PEJAR LOCAL ABORIGINAL LAND COUNCIL TO ARRANGE FOR THE
- ASSESSMENT OF THE ARTEFACTS. COUNCIL IS TO BE NOTIFIED OF THE OUTCOME OF THE INSPECTION. IF ABORIGINAL OBJECTS AND/OR PLACES WILL BE DIRECTLY OR INDIRECTLY ADVERSELY AFFECTED, THE PROPONENT WILL NEED TO APPLY FOR, AND BE ISSUED WITH, AN ABORIGINAL HERITAGE IMPACT PERMIT (AHIP) BY OFFICE OF ENVIRONMENT & HERITAGE.
- 10. AT THE CONCLUSION OF THE CONSTRUCTION WORKS, WORK-AS-EXECUTED (WAE) DRAWINGS SHALL BE SUBMITTED, IN ACCORDANCE WITH COUNCIL'S ENGINEERING STANDARDS. WAE DRAWINGS SHALL BE PROVIDED IN HARD COPY, PDF AND DWG FORMAT ON USB (PREFERABLY) OR CD. AT THE CONCLUSION OF THE FINAL STAGE OF THIS DEVELOPMENT, A CONSOLIDATED SET OF WORK AS EXECUTED DRAWINGS AND GPS DATA SHALL BE PROVIDED ENCOMPASSING ALL THE STAGES.

#### EARTHWORKS NOTES

- 1. EARTHWORKS ARE TO BE CARRIED OUT TO THE SATISFACTION OF THE COUNCIL. UNSUITABLE MATERIALS ARE TO BE REMOVED FROM ROADS AND LOTS PRIOR TO FILLING. THE CONTRACTOR IS TO ARRANGE AND SUBMIT TO COUNCIL, COMPACTION TESTING RESULTS FOR ALL AREAS THAT CONTAIN FILL IN EXCESS OF 200 MM.
- 2. COMPACTION OF EARTHWORKS SHALL CONTINUE UNTIL A DRY DENSITY RATIO OF 95% FOR SITE FILLING AND 100% FOR ROAD PAVEMENT SUBGRADES HAS BEEN ACHIEVED IN ACCORDANCE WITH TEST METHOD AS1289.5.3.1 OR AS.1289.5.1.1. THE CONTROL TESTING OF EARTHWORKS SHALL BE IN ACCORDANCE WITH THE GUIDELINES IN AS3798 'GUIDELINES ON EARTHWORKS FOR COMMERCIAL AND RESIDENTIAL DEVELOPMENTS'. WHERE IT IS PROPOSED TO USE TEST METHOD AS1289.5.8.1 TO DETERMINE THE FIELD DENSITY, A SAND REPLACEMENT METHOD SHALL BE USED TO CONFIRM THE RESULTS.
- THE SUITABLE QUALIFIED GEOTECHNICAL ENGINEER, SHALL HAVE A LEVEL 1 RESPONSIBILITY FOR ALL FILLING AS DEFINED IN APPENDIX B AS3798 'GUIDELINES ON EARTHWORKS FOR COMMERCIAL AND RESIDENTIAL DEVELOPMENTS', AND AT THE END OF THE WORKS SHALL CONFIRM THE EARTHWORKS COMPLY WITH THE REQUIREMENTS OF THE SPECIFICATION AND DRAWINGS IN A WRITTEN REPORT SUBMITTED TO COUNCIL.
- 4. IN THE EVENT THAT ANY AREAS OF POTENTIAL SITE CONTAMINATION ARE DISCOVERED DURING WORKS, WORK SHALL CEASE AND THE APPLICANT SHALL CONTACT THE COUNCIL AND ANY OTHER RELEVANT AUTHORITY. A SUITABLY QUALIFIED CONSULTANT SHALL BE ENGAGED TO INVESTIGATE THE LIKELIHOOD AND/OR EXTENT OF SITE CONTAMINATION, AND A CONTAMINATION REPORT SHALL BE PREPARED IN ACCORDANCE WITH THE PUBLICATION "GUIDELINES FOR CONSULTANTS REPORTING ON CONTAMINATED SITES" BY THE NSW OFFICE OF ENVIRONMENT AND HERITAGE).
- ALL BATTERS ARE TO BE SCARIFIED TO A DEPTH OF 50 MM TO ASSIST WITH ADHESION OF TOP SOIL TO BATTER FACE.
- PROVIDE MINIMUM 150 MM ON FOOTPATHS, FILLED AREAS AND ALL OTHER AREAS DISTURBED DURING CONSTRUCTION. TOP SOILED AREAS TO BE STABILISED WITH APPROVED VEGETATION A MAXIMUM OF 14 DAYS AFTER TOP SOILING AND ARE TO BE WATERED TO ENSURE GERMINATION.
- THE CONTRACTOR SHALL CONTROL SEDIMENTATION, EROSION AND POLLUTION DURING CONSTRUCTION IN ACCORDANCE WITH THE REQUIREMENTS OF THE CURRENT EDITION OF 'MANAGING URBAN STORMWATER: SOILS AND CONSTRUCTION' PRODUCED BY

#### ROADWORKS NOTES

- SUBGRADES AND SUB BASES ARE TO BE COMPACTED IN ACCORDANCE WITH COUNCIL'S CONSTRUCTION SPECIFICATION.
- SUBSOIL DRAINS TO BE PROVIDED ON BOTH SIDES OF ROADS (EXCEPT WHERE THERE IS STORMWATER DRAINAGE). 150 X 50 H.D. GALVANISED STEEL KERB OUTLETS TO BE PLACED IN ALL KERB TYPES ON LOW SIDE OF LOTS. PROVIDE SUITABLE
- ADAPTOR TO ALLOW CONNECTION OF 90 MM DIAMETER STORMWATER PIPE. 4. LIPLESS PERAMBULATOR CROSSINGS ARE TO BE PROVIDED IN ALL KERB RETURNS AND WHERE REQUIRED BY COUNCIL IN
- ACCORDANCE WITH STANDARD DRAWING SD-R 10 A AND SD-R 10 B.
- SERVICE CONDUITS TO BE PLACED AS DIRECTED BY ALL PUBLIC UTILITY AUTHORITIES INCLUDING ESSENTIAL ENERGY AND TELSTRA. PROPOSED UTILITIES AND SERVICES CROSSING EXISTING ROADS SHALL BE PROVIDED FOR USING A TRENCHLESS TECHNIQUE SO AS
- NOT TO DAMAGE THE EXISTING SURFACE. ALL SERVICE CONDUITS UNDER ROADS MUST BE LAID TO A MINIMUM DEPTH OF 750 MM. ALL TEMPORARY ROADS MUST BE TEMPORARILY SEALED WITH A SINGLE COAT FLUSH SEAL.
- SIGNPOSTING AND LINE MARKING SHALL CONFORM TO AS1742.2 'TRAFFIC CONTROL DEVICES FOR GENERAL USE'. RAISED RETRO-REFLECTIVE PAVEMENT MARKERS TO CONFORM TO AS1906 '111RETRO-REFLECTIVE MATERIALS AND DEVICES FOR ROAD TRAFFIC CONTROL PURPOSES'. ALL APRONS AND KERB FACE ON CENTRAL ISLANDS OF ROUNDABOUTS AND ALL OTHER ISLANDS TO BE DELINEATED BY REFLECTIVE WHITE MARKING. INSTALLATION SHALL OCCUR IN ACCORDANCE WITH THE PLAN APPROVED BY THE LOCAL TRAFFIC COMMITTEE.
- 9. STREET SIGNS TO COUNCIL STANDARD MUST BE INSTALLED BY THE CONTRACTOR.

#### STORMWATER NOTES

- ALL PIPES TO BE SPIGOT AND SOCKET, RUBBER RING JOINTED.
- 2. ALL LONGITUDINAL PIPELINES IN ROADS MUST BE LOCATED UNDER KERB AND GUTTER AND BE BACKFILLED WITH APPROVED GRANULAR MATERIAL UNLESS OTHERWISE APPROVED BY THE COUNCIL ENGINEER.
- 3. DRAINAGE LINES MUST BE BACKFILLED WITH APPROVED GRANULAR MATERIAL IN TRAFFICABLE AREAS. THREE (3) METRES OF SUBSOIL DRAINAGE WRAPPED IN GEOTEXTILE STOCKING MUST BE PROVIDED TO ALL DOWNSTREAM PITS.
- 4. ALL GULLY PITS TO COUNCIL'S STANDARD AND LINTELS CENTRALLY PLACED AT SAG PITS.
- 5. ALL PITS MUST BE BENCHED AND STREAMLINED. PROVIDE SL72 REINFORCEMENT AND GALVANISED STEP IRONS IN ALL PITS OVER 1.2-METRES DEEP AS MEASURED FROM THE TOP OF GRATE TO THE INVERT OF THE PIT.
- 6. CONCRETE IS TO HAVE MINIMUM COMPRESSIVE STRENGTH OF 32MPA AT 28-DAYS UNLESS OTHERWISE APPROVED BY THE COUNCIL
- 7. ALL INTERALLOTMENT DRAINAGE MUST HAVE A MINIMUM PIPE DIAMETER OF 150 MM AND A MINIMUM GRADE OF 1% UNLESS OTHERWISE APPROVED BY THE COUNCIL ENGINEER.
- 8. ALL INTERALLOTMENT DRAINAGE LINES MUST BE LAID CENTRALLY WITHIN DRAINAGE EASEMENTS. INSPECTION PITS MUST BE PROVIDED AT ALL CHANGES OF GRADE AND DIRECTION.
- 9. INTERALLOTMENT DRAINAGE LINES MUST BE INSTALLED AFTER SEWERAGE LINES HAVE BEEN INSTALLED WHERE SEWER IS PROPOSED
- ADJACENT TO INTERALLOTMENT DRAINAGE LINES.
- 10. 1% AEP OVERLAND FLOW PATHS MUST BE FORMED AND SHOWN ON 'WORKS AS EXECUTED' DRAWINGS.
- 11. ALL PLANS (BOTH DESIGN AND WAE) ARE TO CLEARLY DELINEATE THE EXTENT/LOCATION OF FLOOD LINES INCLUDING THE 5% AEP, 1% AEP, FLOOD PLANNING LEVEL AND PROBABLE MAXIMUM FLOOD.
- 12. ADEQUATE PERMANENT SCOUR PROTECTION AND SEDIMENTATION CONTROL SHALL BE PROVIDED FOR ALL DRAINAGE WORKS IN ACCORDANCE WITH COUNCIL'S REQUIREMENTS.
- 13. SOIL AND WATER MANAGEMENT PLANS ARE TO BE PREPARED FOR ALL DISTURBED SITES AND ADHERED TO AT ALL TIMES DURING THE CONSTRUCTION AND MAINTENANCE PERIODS.

#### LANDSCAPING NOTES

- THE DEVELOPER IS REQUIRED TO PLANT STREET TREES BEING A MAXIMUM OF ONE TREE FOR EACH LOT AND TWO TREES FOR CORNER THE SPECIES OF THE TREE SHOULD BE IN ACCORDANCE WITH THE THEMES CONTAINED IN THE GOULBURN STREET TREE MASTER PLAN AND BE APPROVED BY COUNCIL'S LANDSCAPE & HERITAGE PLANNER
- STREET TREES SHALL BE ADVANCED SPECIMENS HAVING A CONTAINER VOLUME OF 25 LITRES AND A HEIGHT OF AT LEAST 1.5M. THE TREES SHALL BE STAKED WITH THREE 1800 MM X 50 MM SQUARE HARDWOOD STAKES AND LOOSELY SECURED WITH HESSIAN WEBBING. THE PLANTING HOLE IS TO BE TWICE THE WIDTH AND ONE AND A HALF TIMES THE DEPTH OF THE POT OF THE TREE TO BE PLANTED. THE HOLE SHALL BE FILLED WITH SOIL SUITABLE FOR TREE GROWTH BEING MADE UP OF FREE DRAINING COARSE SAND 50%.
- LOAM 20% AND COMPOSTED ORGANIC MATTER 30% AND HAVING A NEUTRAL PH. A MINIMUM 600MM WIDE, CONTINUOUS STRIP OF CANTURF CANBERRA BLEND OR EQUIVALENT (75% TALL FINE FESCUE, 25% KENTUCKY BLUEGRASS) TURF SHALL BE PLACED BEHIND THE BACK OF ALL KERBS AND ADJACENT TO OTHER CONCRETE STRUCTURES IMMEDIATELY AFTER THE COMPLETION OF THE FOOTPATH GRADING OR OTHER ELEMENTS AS APPLICABLE. AND SHALL BE MAINTAINED AND REPLACED AS REQUIRED DURING THE CONSTRUCTION MAINTENANCE PERIOD.
- ALL DISTURBED AREAS (INCLUDING NATURE STRIPS, BATTERS, ALLOTMENTS WITH GRADES OF 1:5 AND GREATER, AND RESERVES) ARE TO BE REINSTATED TO A CLEAN, TIDY CONDITION, TOP DRESSED (100MM LOAMY TOP SOIL FREE OF RUBBISH, DEBRIS, CLUMPS, SODS AND CLAY LUMPS), LIGHTLY COMPACTED AND HYDRO MULCHED.

HYDROMULCH AND SEED MIX TO BE:

- CANTURF CANBERRA BLEND OR EQUIVALENT (75% TALL FINE FESCUE, 25% KENTUCKY BLUEGRASS) IN THE ROAD RESERVE AND OTHER PUBLIC OPEN SPACES
- 1,500KG/HA OF CELLULOSE FIBRE SOIL BINDER, SPECIFICALLY MANUFACTURED FOR HYDROMULCHING, USED AT MANUFACTURERS RECOMMENDED RATES. (E.G. ORGANIC GAUR TACKIFIERS @ 20 - 30 KG/HA, BASED ON, SITE CONDITIONS).

GRASS IS TO BE ESTABLISHED PRIOR TO THE END OF THE MAINTENANCE PERIOD, UNLESS OTHERWISE AGREED IN WRITING.

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Client

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SHEET 2

**EQUINOX - STAGE 3** WILSON DRIVE, **MARULAN** 

**GENERAL NOTES** 

Civil Engineers and Project Managers Level 7, 153 Walker Street

North Sydney NSW 2060 ABN 96 130 882 405

Issue

Tel: 02 9439 1777 Fax: 02 9923 1055 www.atl.net.au info@atl.net.au

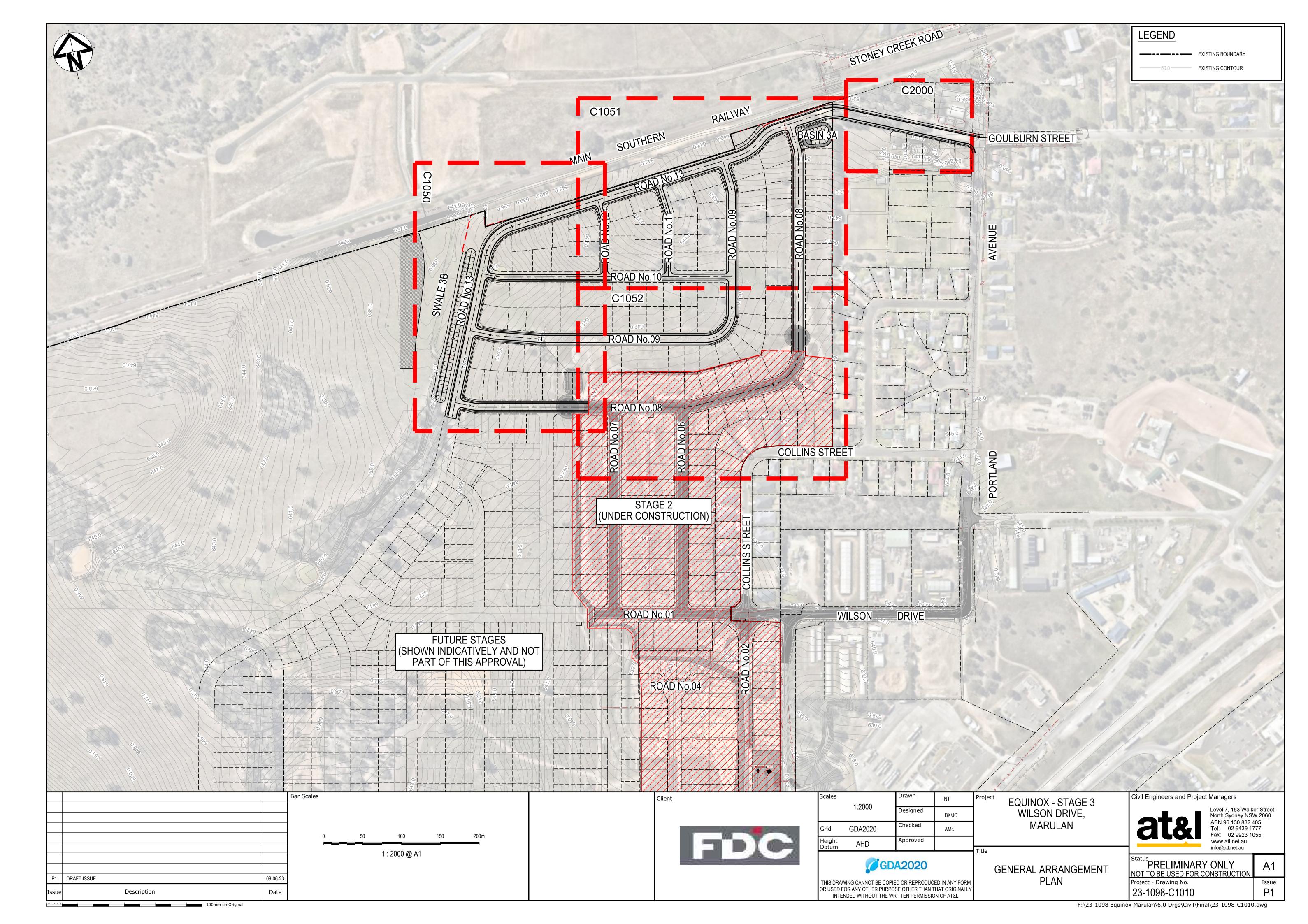
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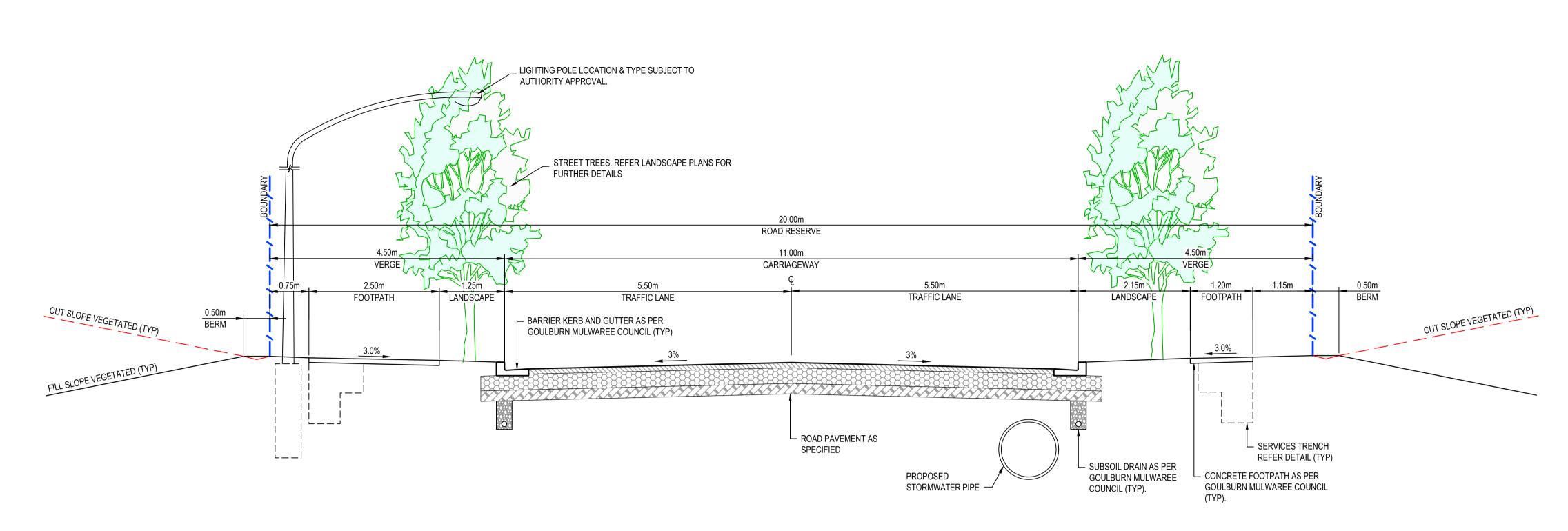
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Bar Scales

09-06-23

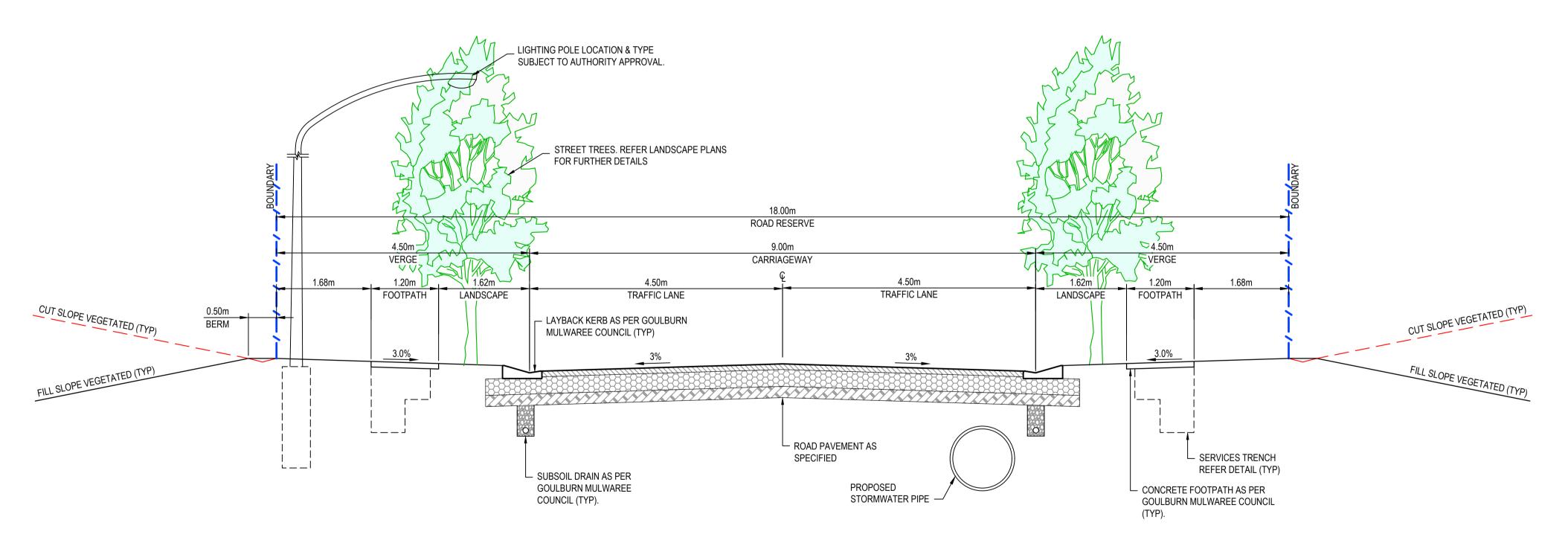
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#### TYPICAL SECTION (COLLECTOR STREET)

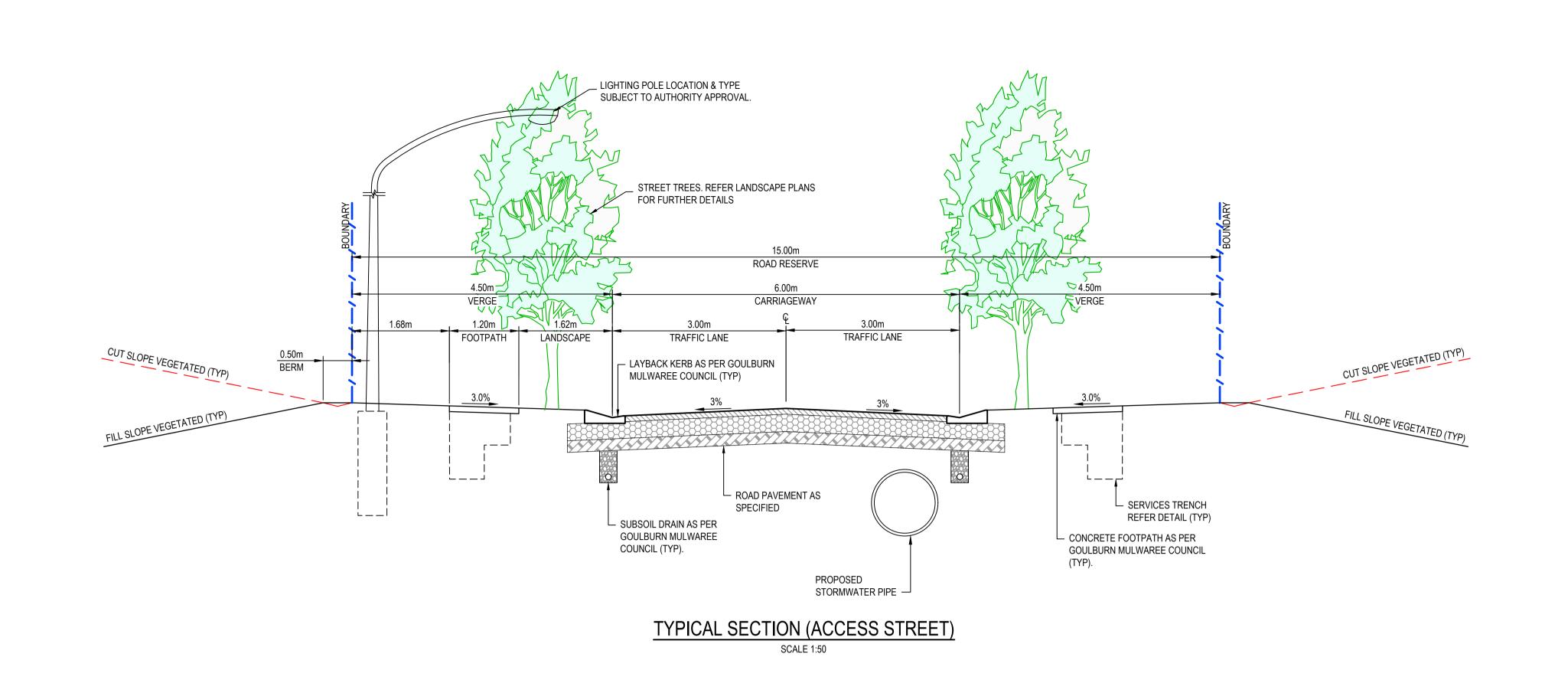
SCALE 1:50



## TYPICAL SECTION (LOCAL STREET) SCALE 1:50

Bar Scales Civil Engineers and Project Managers Client NT EQUINOX - STAGE 3 WILSON DRIVE, AS SHOWN Level 7, 153 Walker Street North Sydney NSW 2060 Designed BK/JC ABN 96 130 882 405 MARULAN Checked GDA2020 Tel: 02 9439 1777 AMc Fax: 02 9923 1055 Height Datum Approved www.atl.net.au AHD info@atl.net.au 1:50 @ A1 1:100 @ A3 Title PRELIMINARY ONLY **GDA2020** TYPICAL SECTIONS NOT TO BE USED FOR CONSTRUCTION SHEET 1 P1 DRAFT ISSUE 09-06-23 Project - Drawing No. THIS DRAWING CANNOT BE COPIED OR REPRODUCED IN ANY FORM Issue OR USED FOR ANY OTHER PURPOSE OTHER THAN THAT ORIGINALLY 23-1098-C1020 Date Description INTENDED WITHOUT THE WRITTEN PERMISSION OF AT&L

100mm on Original



Bar Scales 1:50 @ A1 1:100 @ A3 09-06-23 P1 DRAFT ISSUE Date Description



Client

NT AS SHOWN Designed BK/JC Checked GDA2020 Height Datum Approved

**GDA2020** 

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EQUINOX - STAGE 3 WILSON DRIVE, MARULAN

TYPICAL SECTIONS SHEET 2

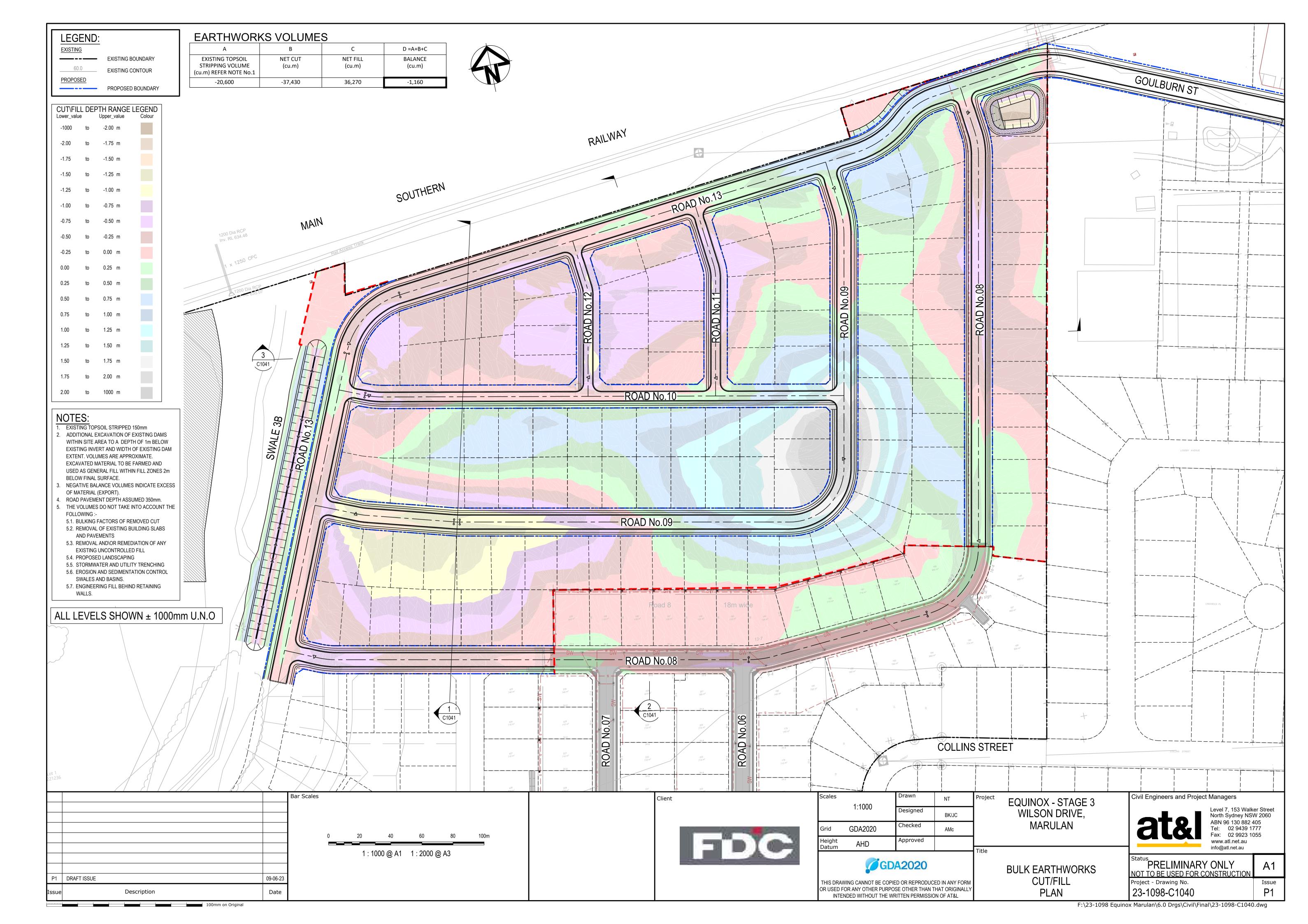
Civil Engineers and Project Managers

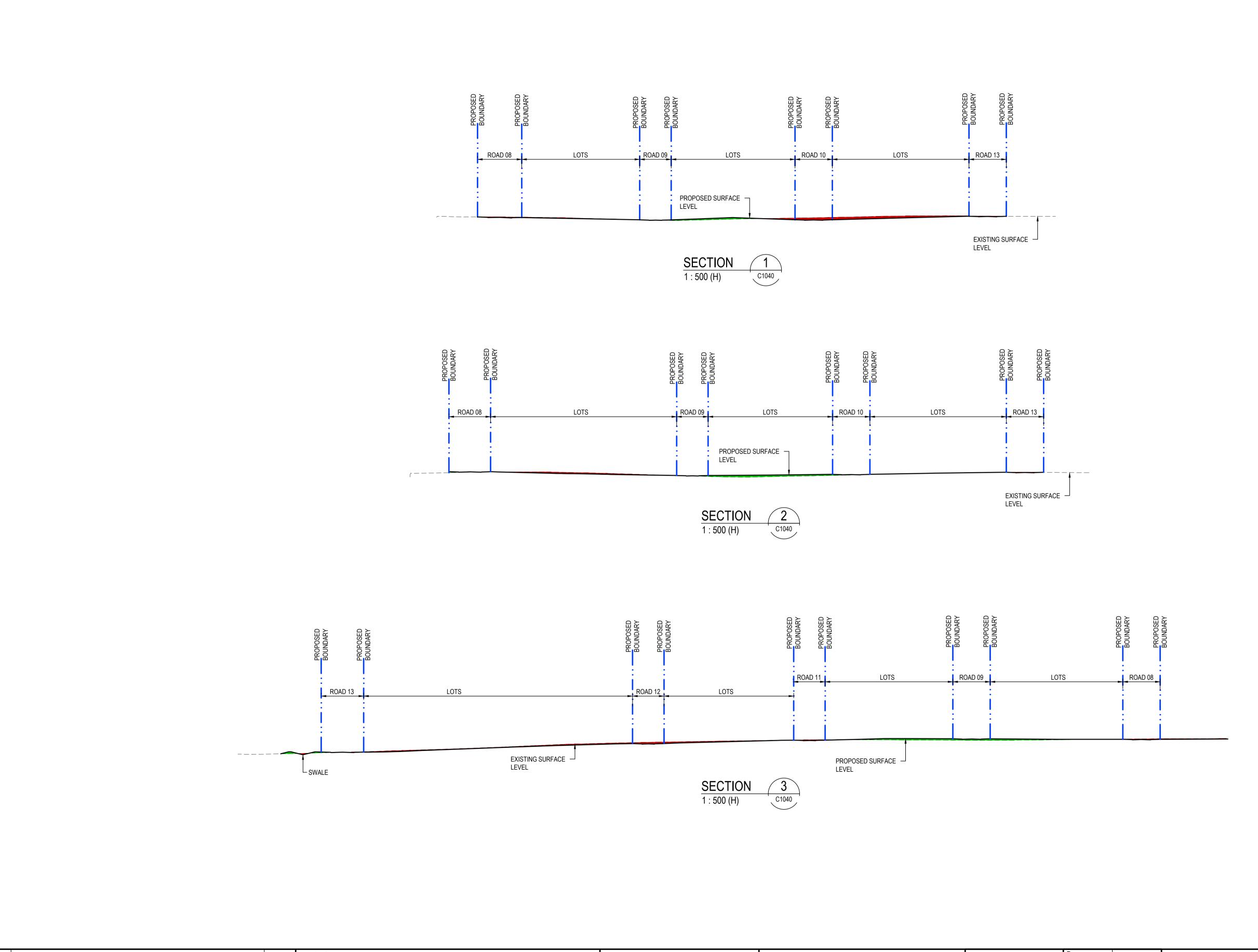


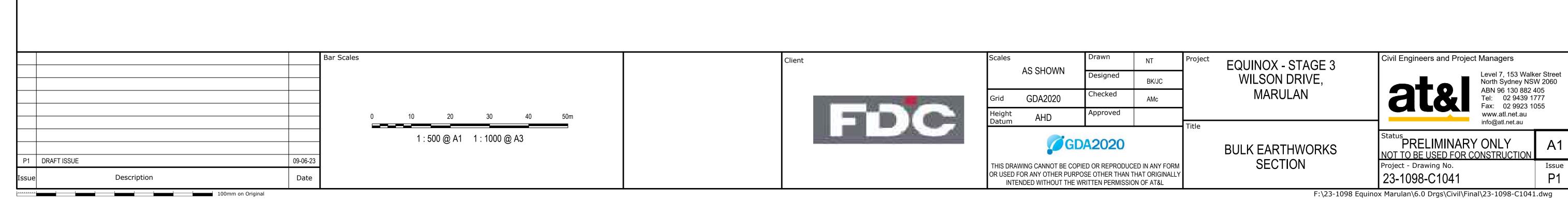
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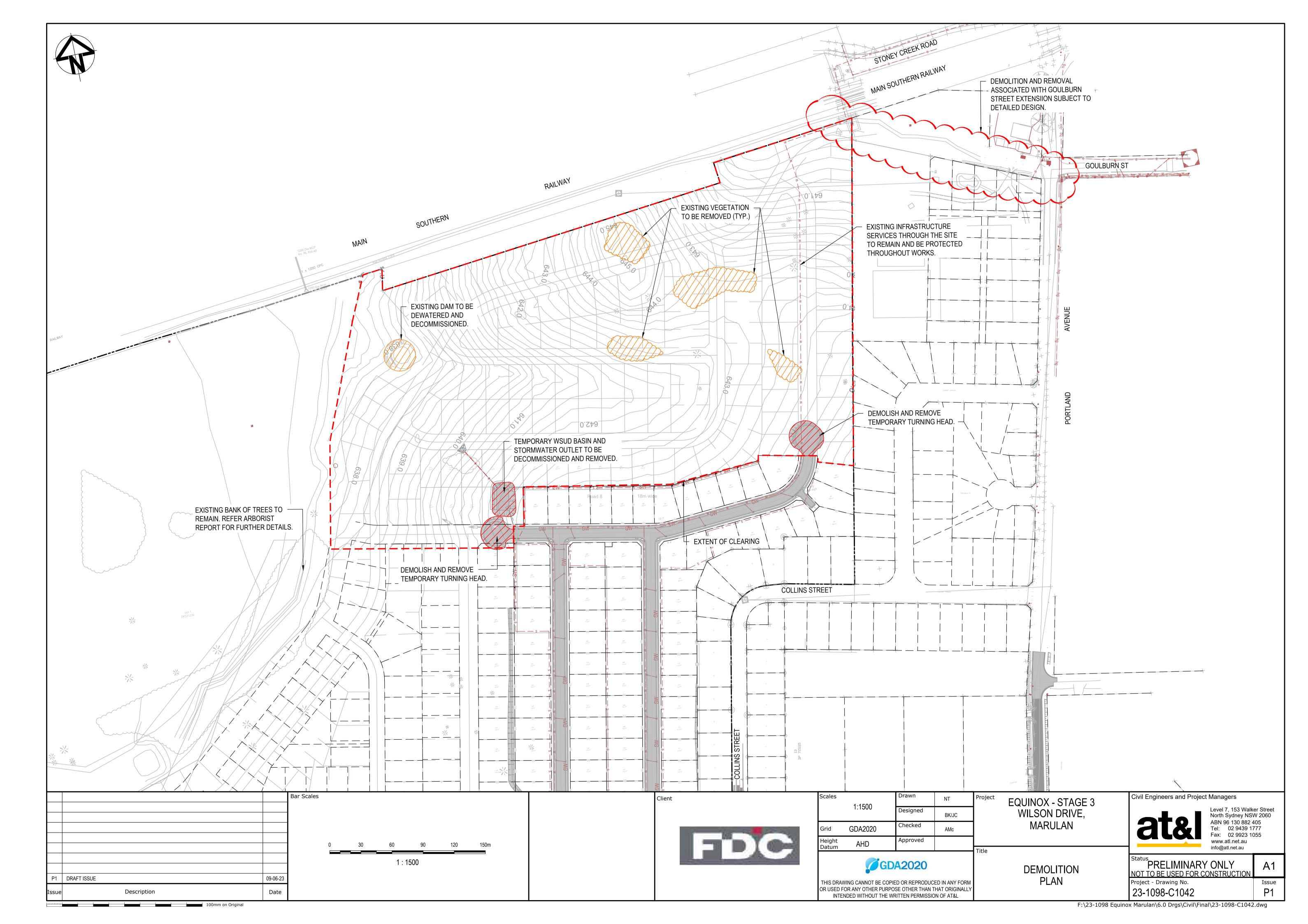
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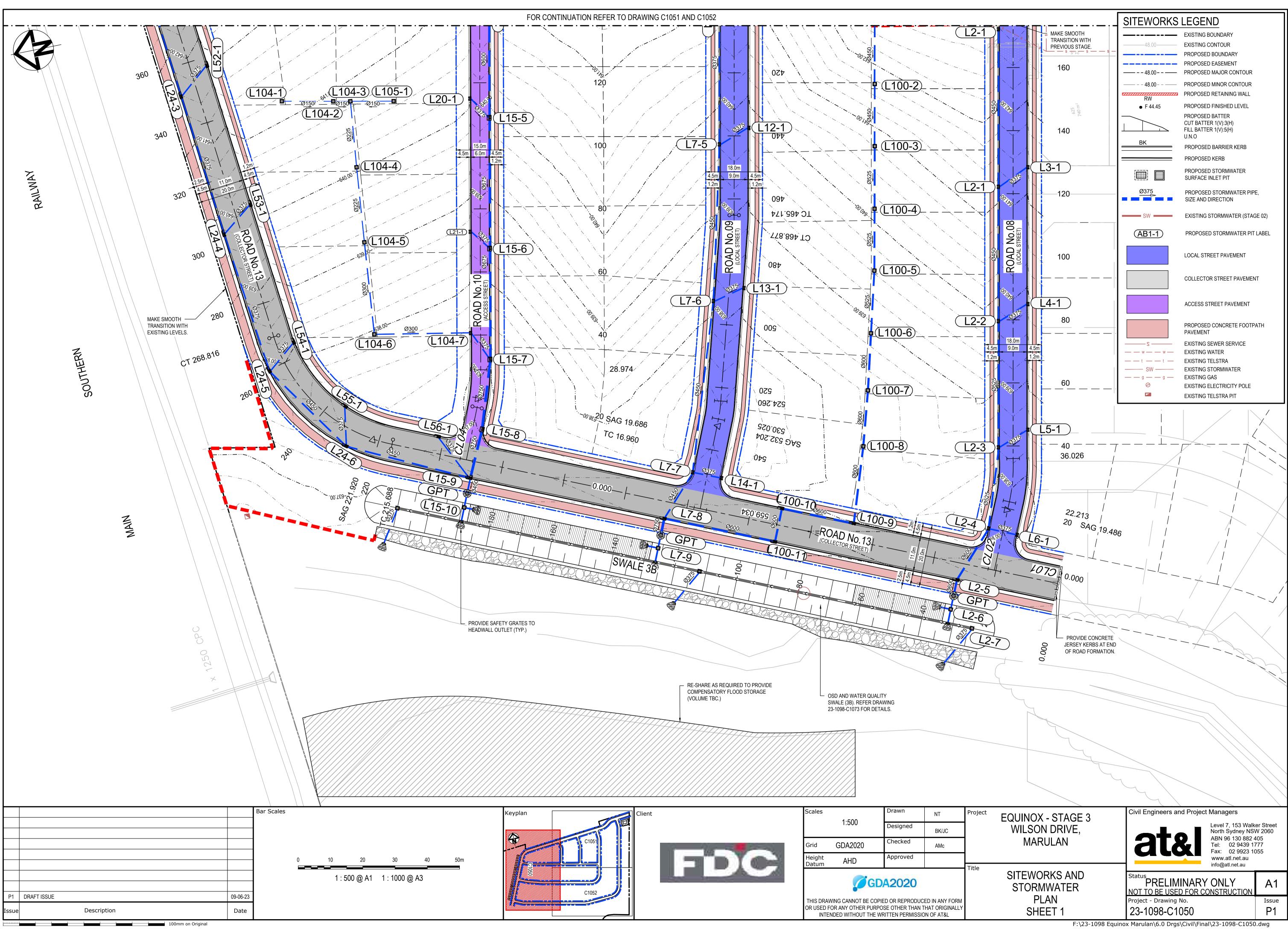
Issue P1

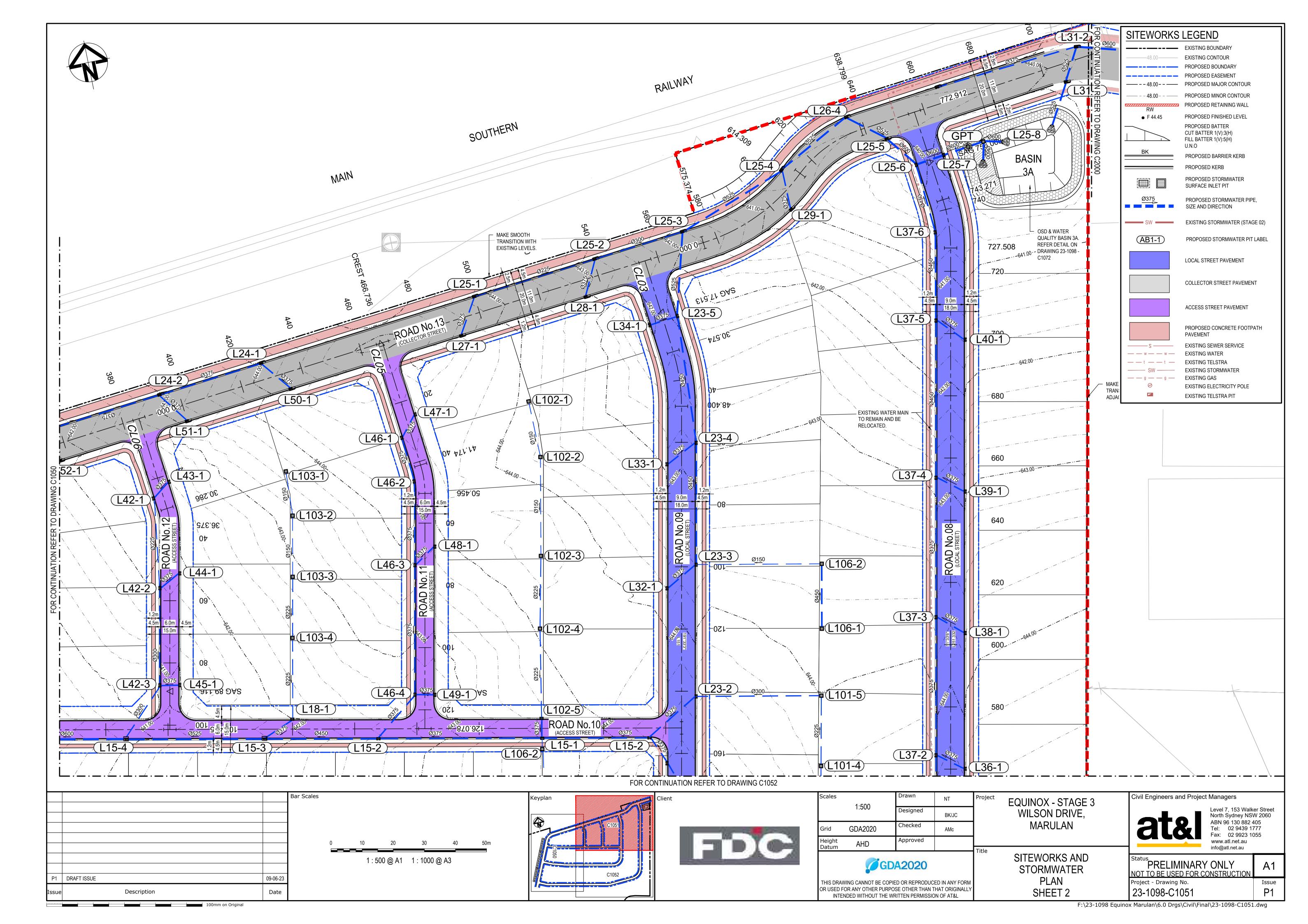


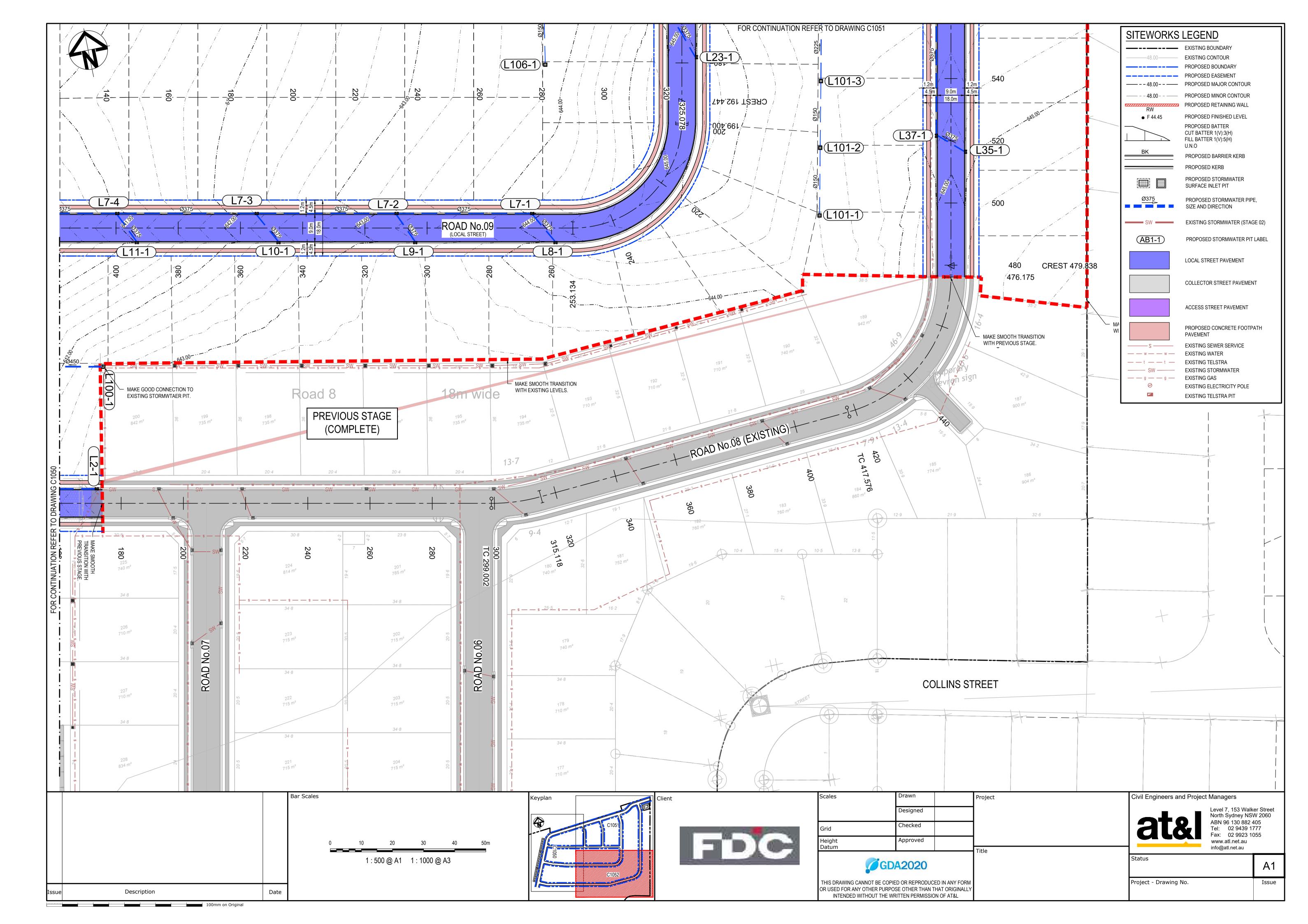


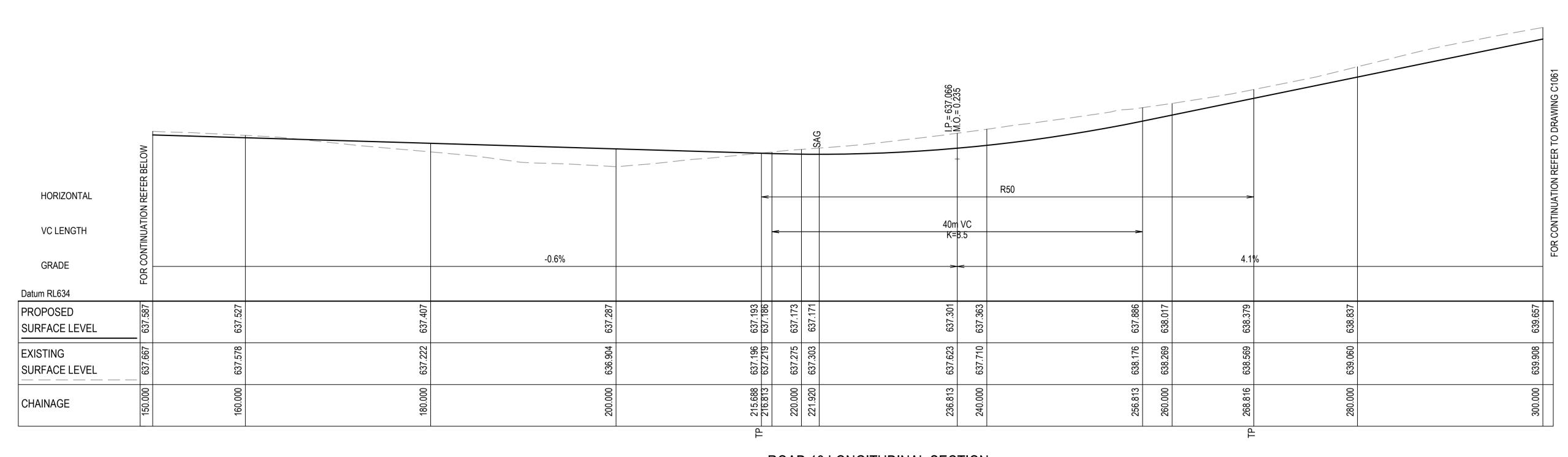






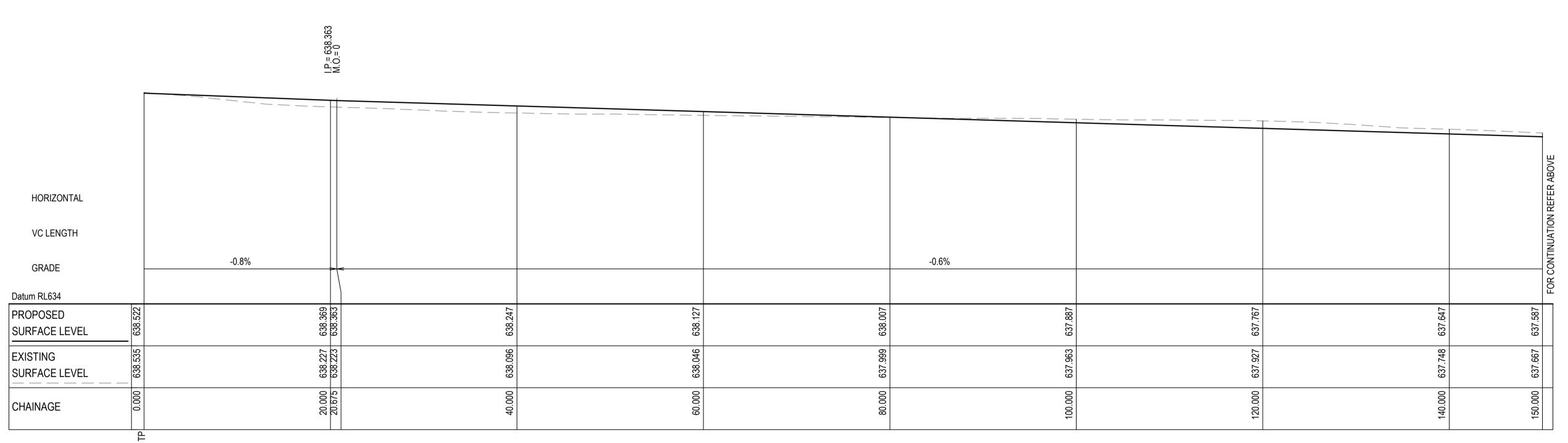






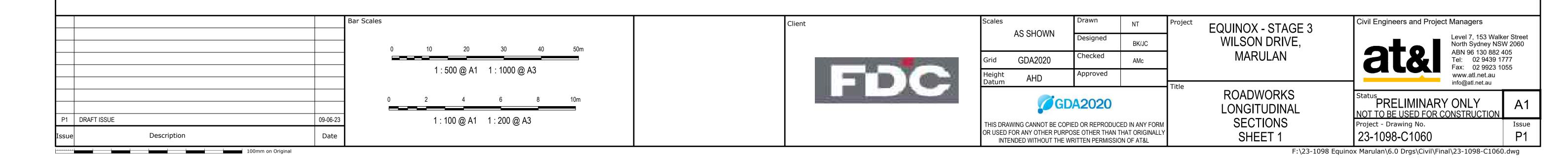
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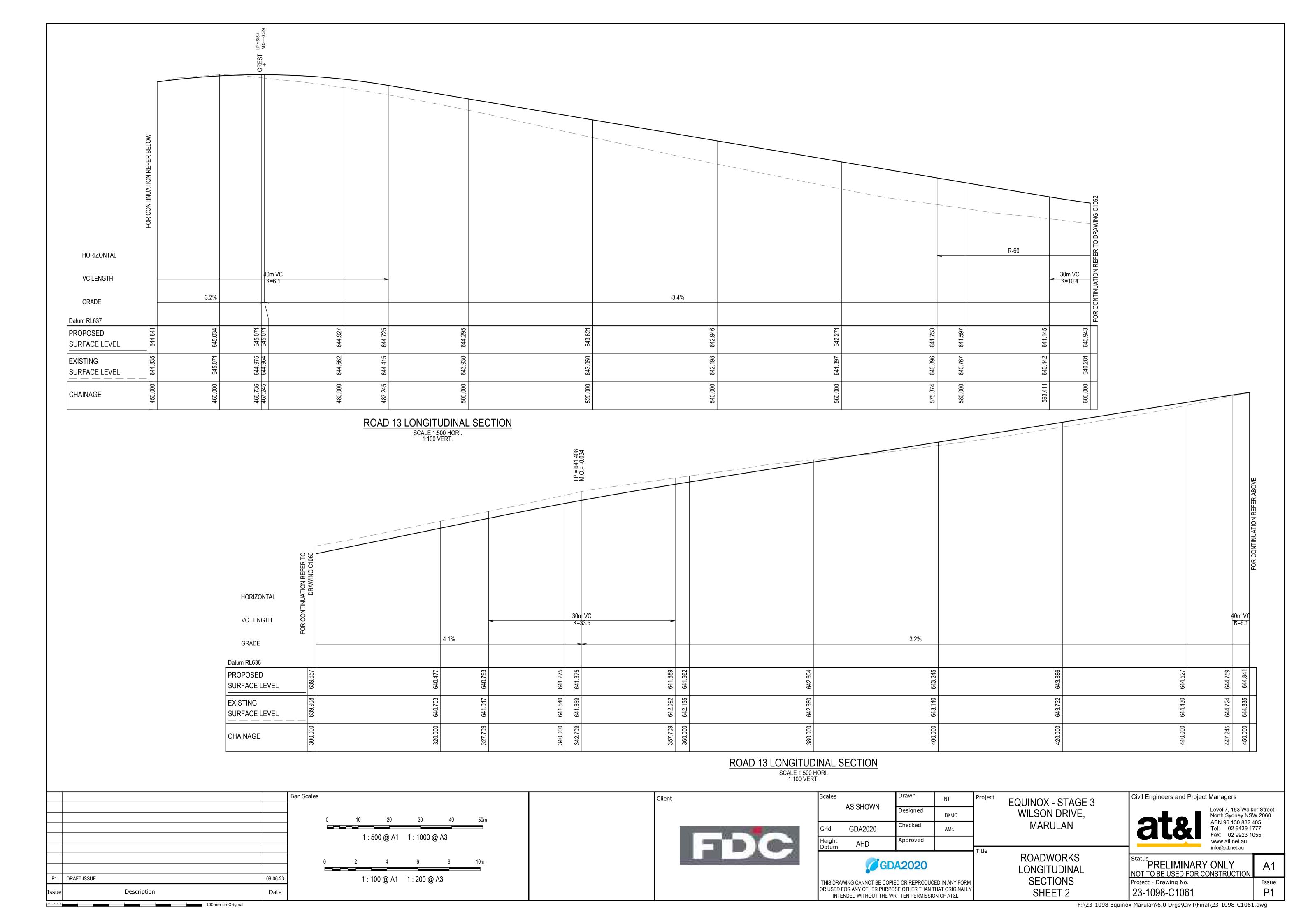
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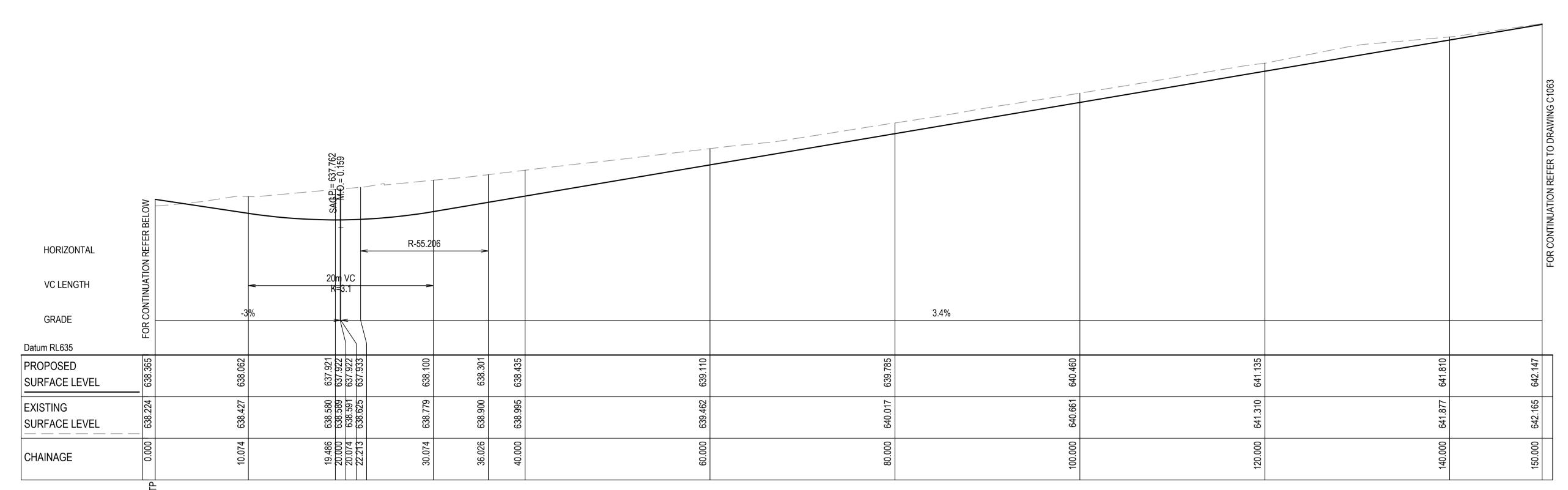


ROAD 13 LONGITUDINAL SECTION

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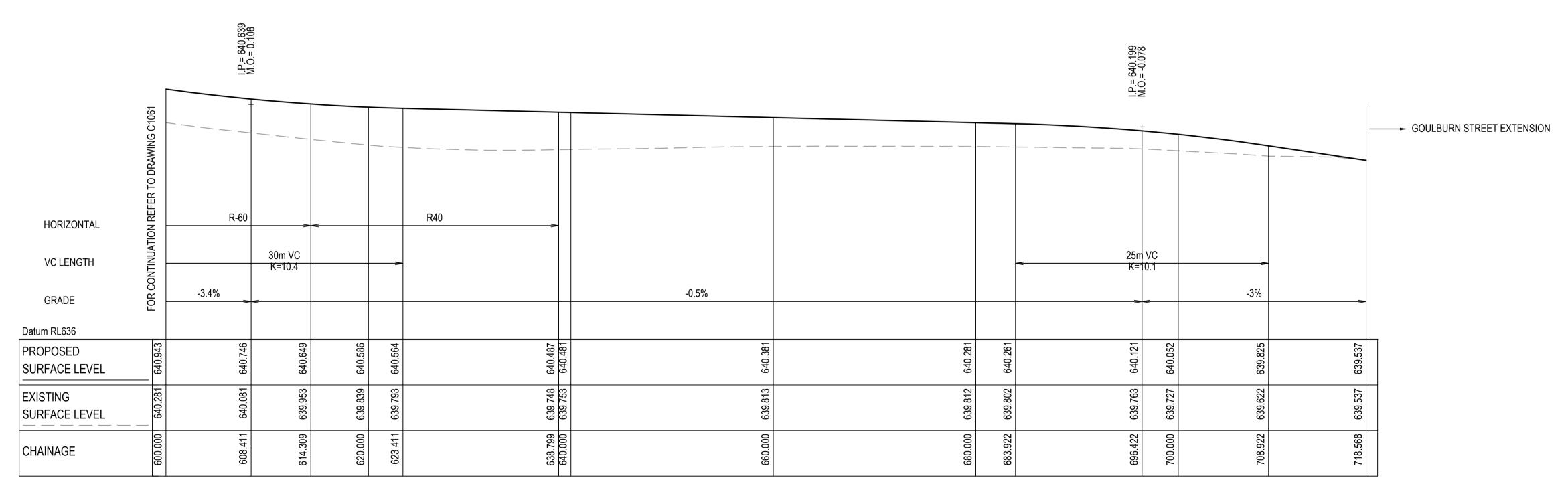




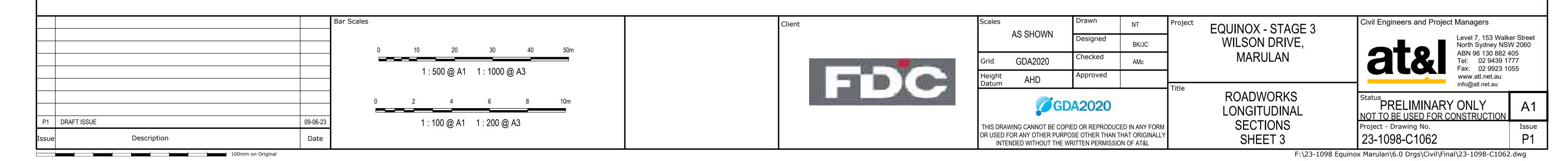


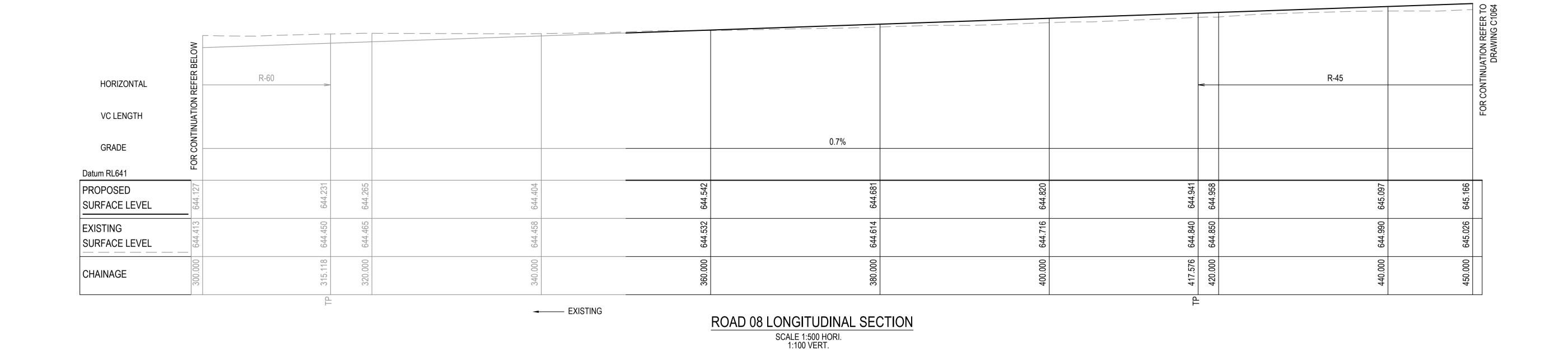
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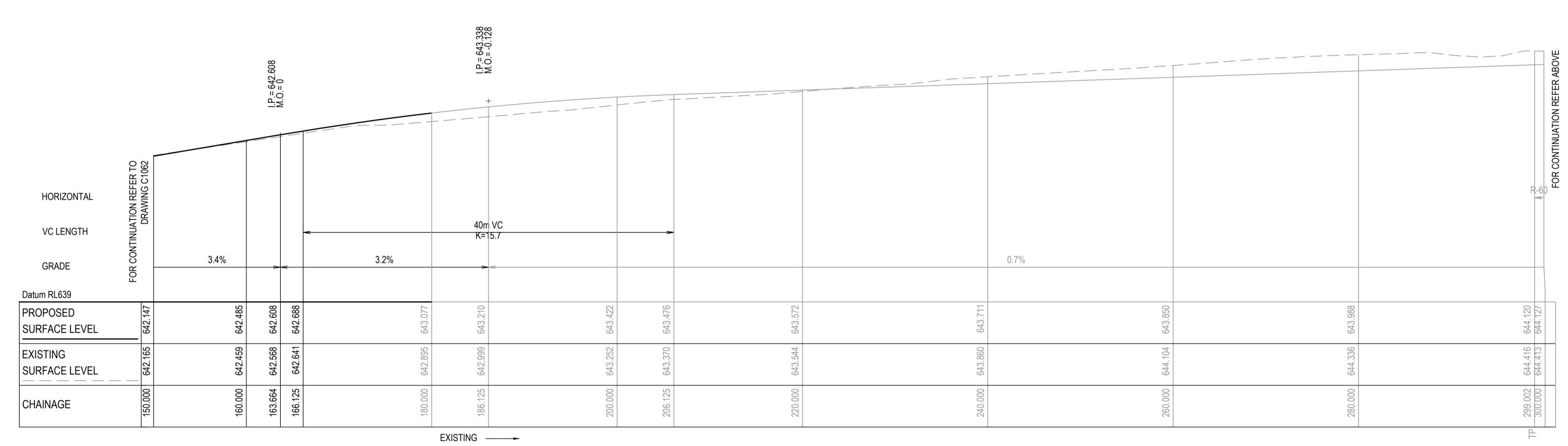
SCALE 1:500 HORI.
1:100 VERT.



## ROAD 13 LONGITUDINAL SECTION SCALE 1:500 HORI. 1:100 VERT.



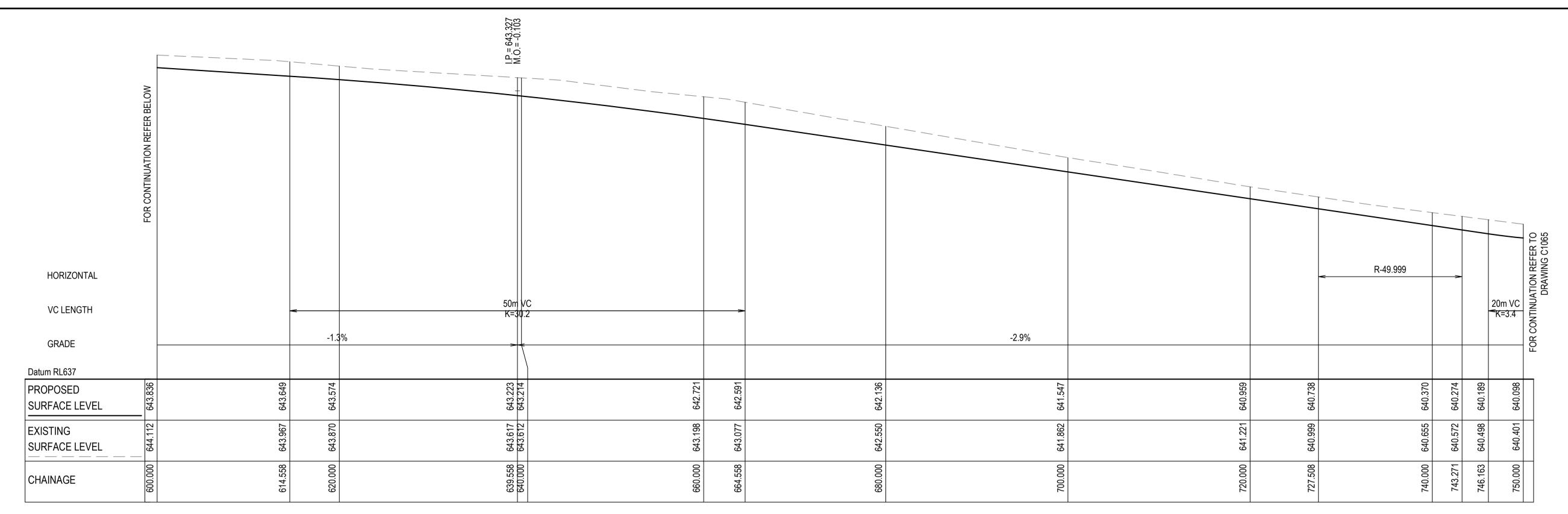




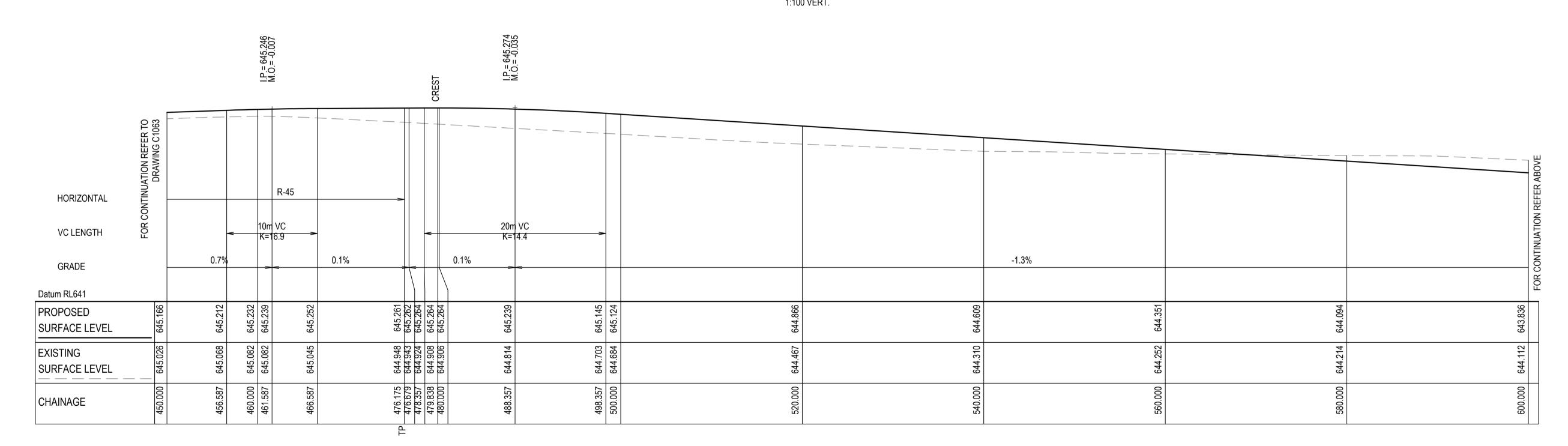
ROAD 08 LONGITUDINAL SECTION

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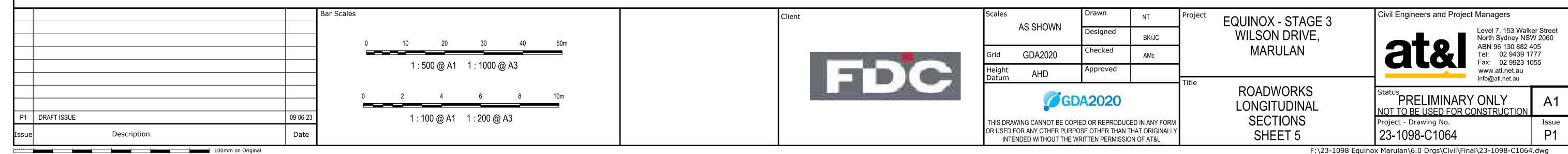
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	0 40 00 40 50		AS SHOWN	Designed BK/J0		Level 7, 153 Walker Street North Sydney NSW 2060
	0 10 20 30 40 50m		Grid GDA2020	Checked AMc	MARULAN	ABN 96 130 882 405 Tel: 02 9439 1777
	1 : 500 @ A1 1 : 1000 @ A3		Height AHD	Approved		Fax: 02 9923 1055 www.atl.net.au info@atl.net.au
	0 2 4 6 8 10m			A2020	ROADWORKS LONGITUDINAL	Status PRELIMINARY ONLY NOT TO BE USED FOR CONSTRUCTION
P1 DRAFT ISSUE 09-06-23  Issue Description Date	1 : 100 @ A1 1 : 200 @ A3		THIS DRAWING CANNOT BE COPIED OR USED FOR ANY OTHER PURPOSI INTENDED WITHOUT THE WRI	SE OTHER THAN THAT ORIG	SECTIONS SINALLY	Project - Drawing No. Issue P1
100mm on Original			INTENDED WITHOUT THE WRI	ITTEN PERIVIDATON OF ATO		uinox Marulan\6.0 Drgs\Civil\Final\23-1098-C1063.dwg

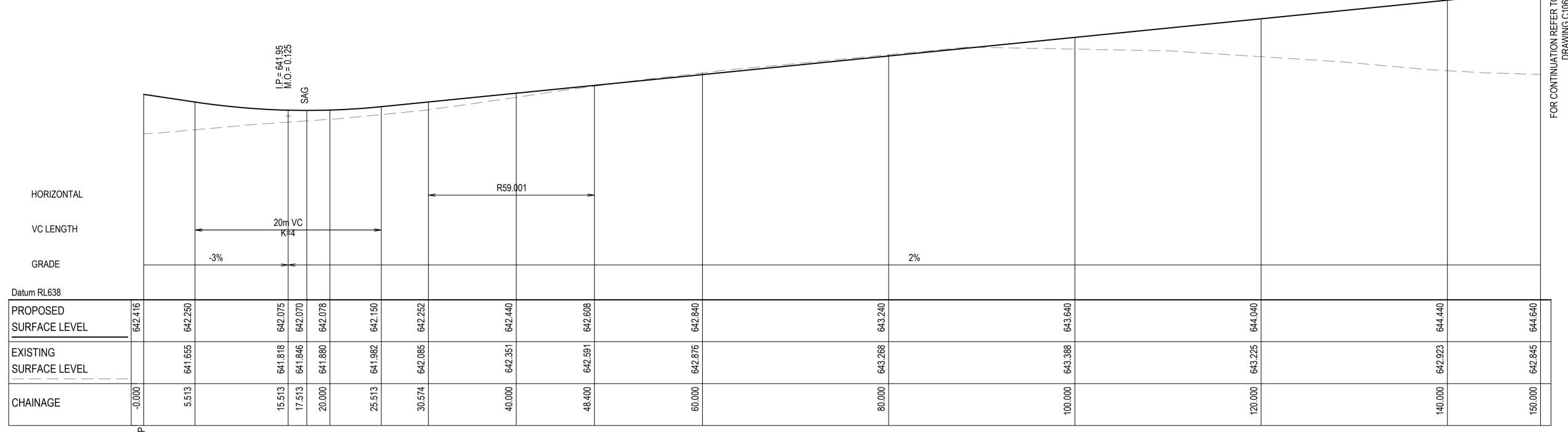


#### **ROAD 08 LONGITUDINAL SECTION** SCALE 1:500 HORI. 1:100 VERT.

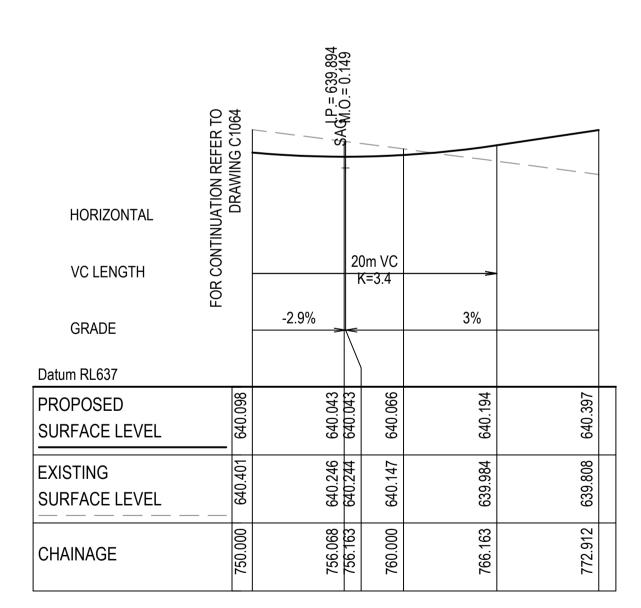


#### ROAD 08 LONGITUDINAL SECTION SCALE 1:500 HORI. 1:100 VERT.



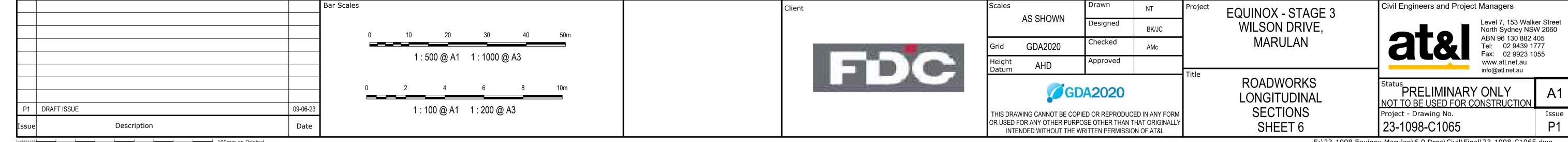


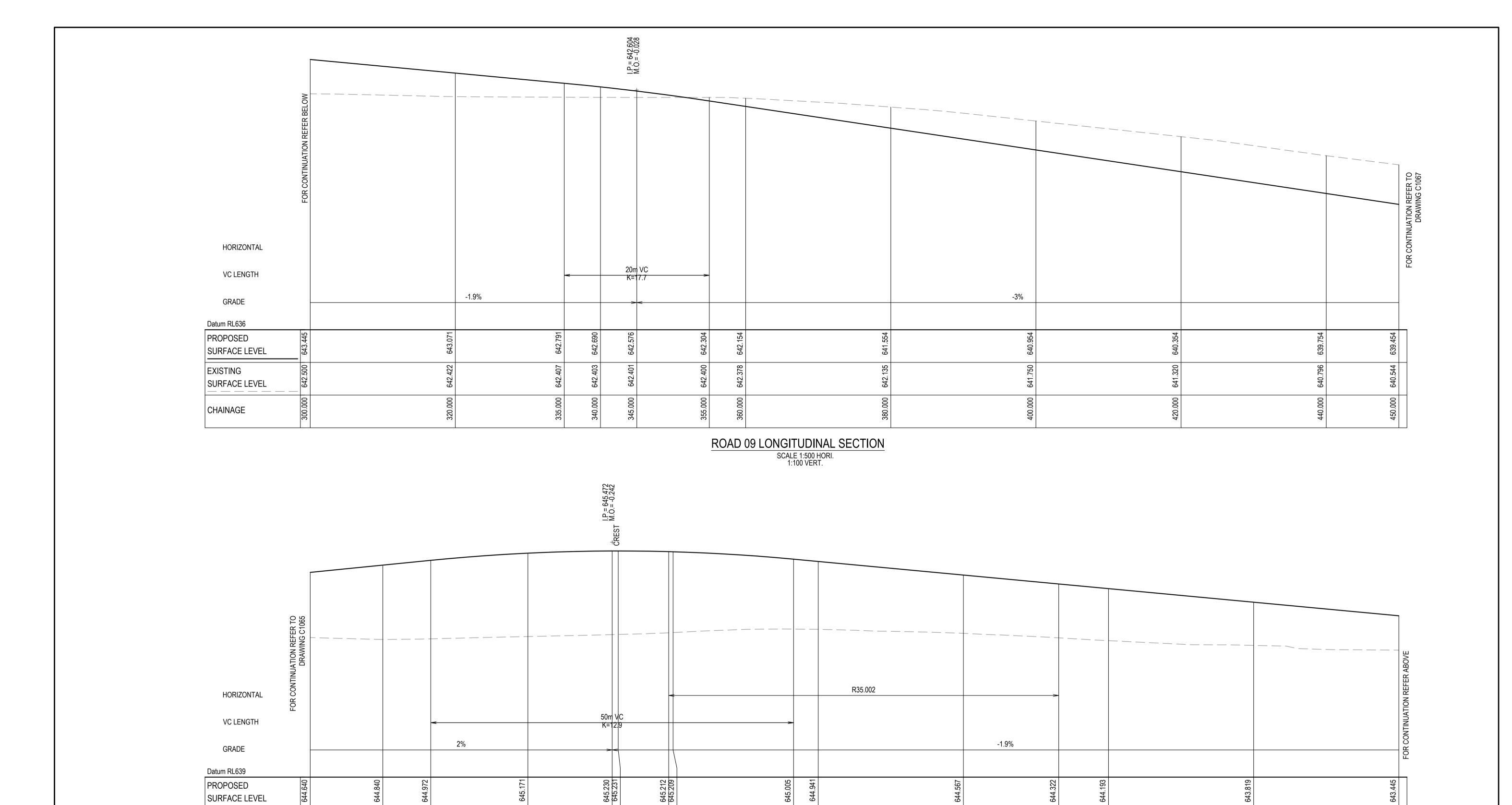
ROAD 09 LONGITUDINAL SECTION
SCALE 1:500 HORI.



ROAD 08 LONGITUDINAL SECTION

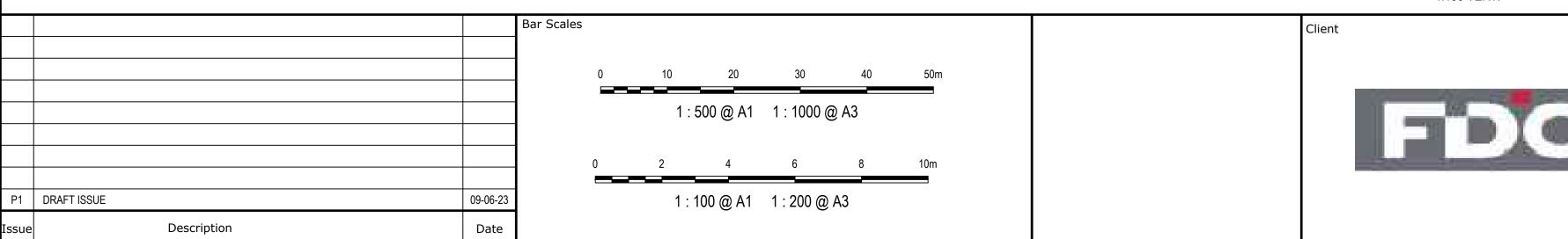
SCALE 1:500 HORI.
1:100 VERT.





199.400

# ROAD 09 LONGITUDINAL SECTION SCALE 1:500 HORI. 1:100 VERT.



EXISTING

CHAINAGE

SURFACE LEVEL



Scales	A C CLIOWN	Drawn	NT	Pro
	AS SHOWN	Designed	BK/JC	
Grid	GDA2020	Checked	AMc	
Height	AHD	Approved		

**GDA2020** 

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ROADWORKS LONGITUDINAL SECTIONS

SHEET 7

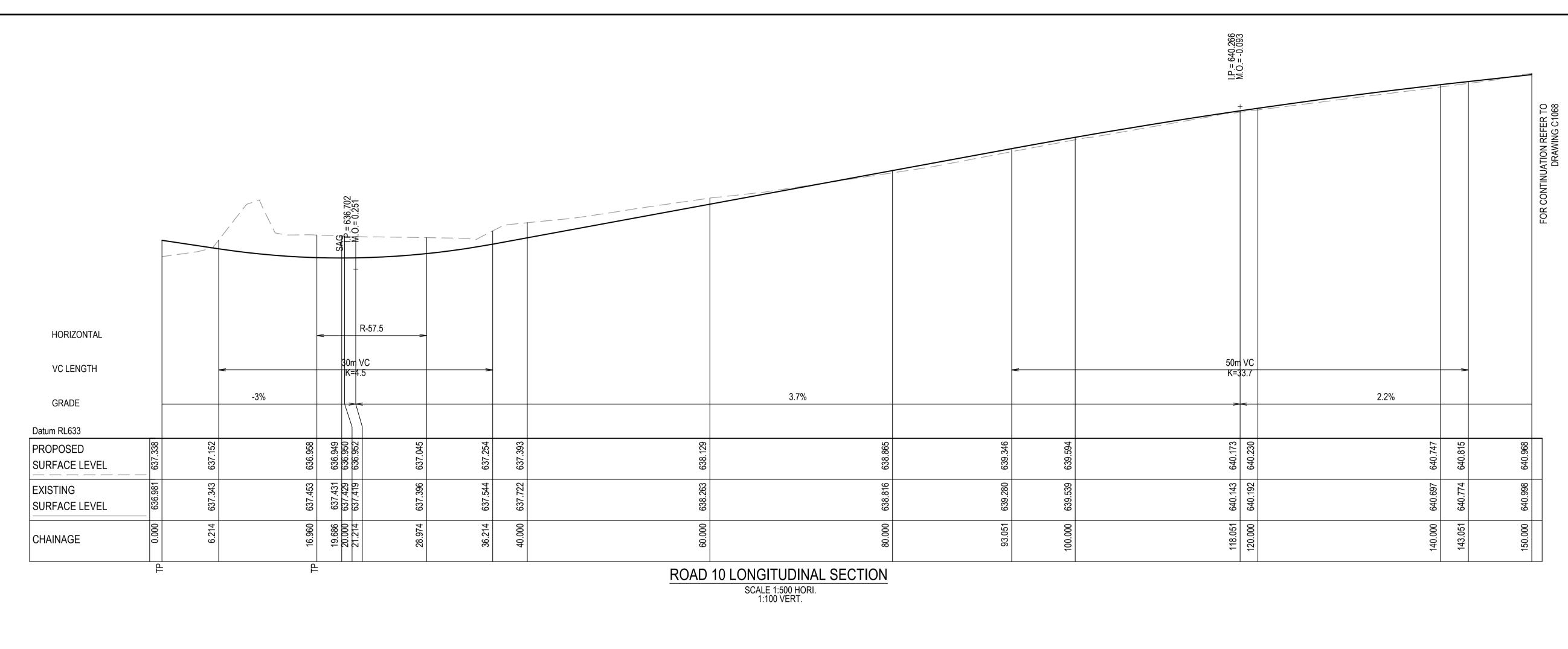
EQUINOX - STAGE 3 WILSON DRIVE,

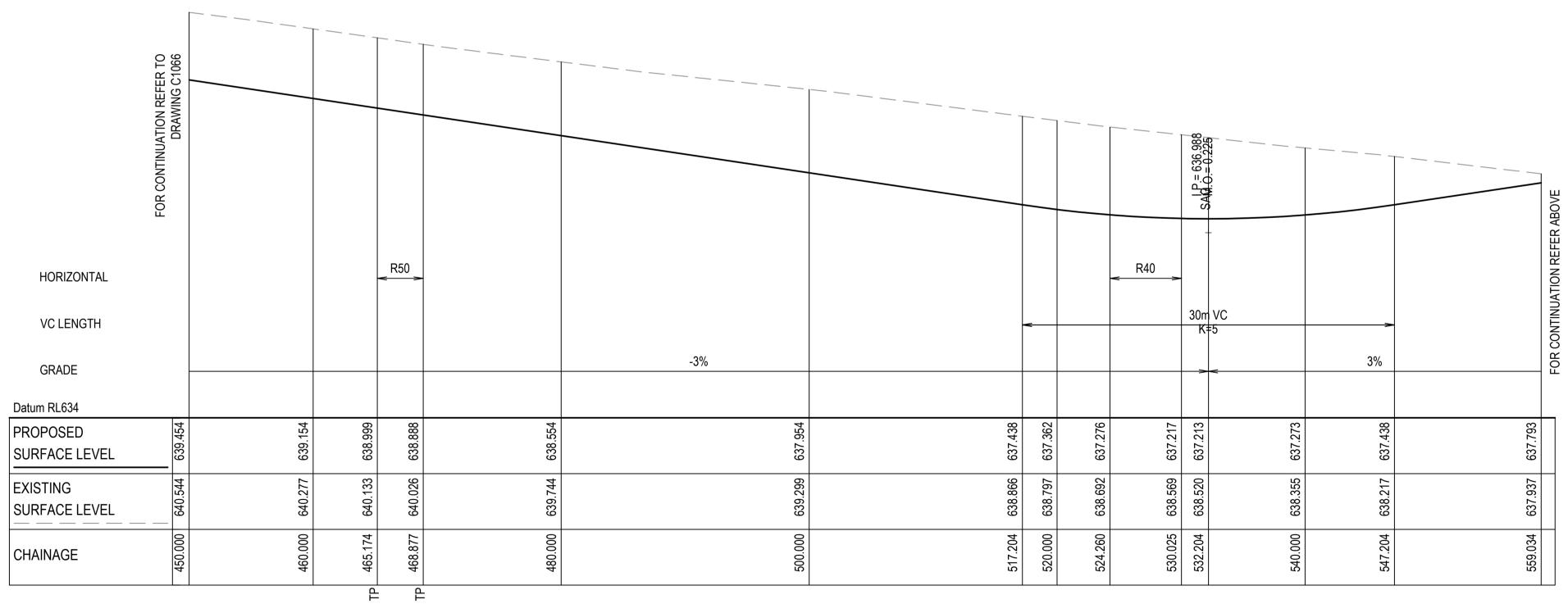
MARULAN

Civil Engineers and Project Managers Level 7, 153 Walker Street North Sydney NSW 2060 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9923 1055 www.atl.net.au info@atl.net.au

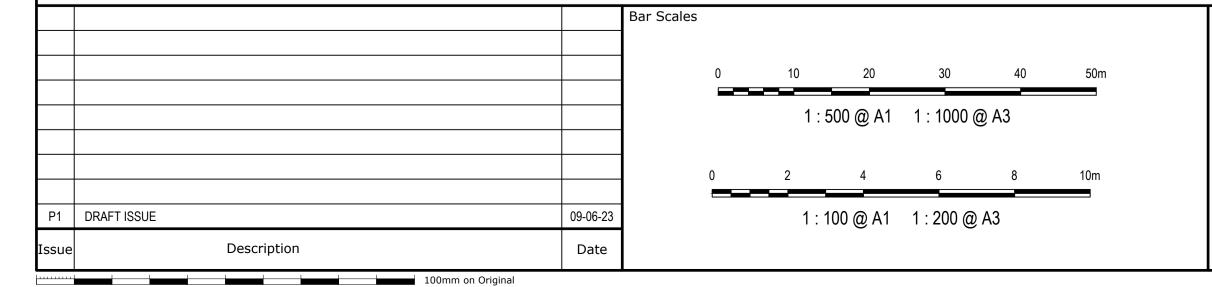
Project - Drawing No. 23-1098-C1066

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ROAD 09 LONGITUDINAL SECTION SCALE 1:500 HORI. 1:100 VERT.





Client

Scales	4 C CLIO/4/N	Drawn	NT	Project
	AS SHOWN	Designed	BK/JC	
Grid	GDA2020	Checked	AMc	
Height	AHD	Approved		
Datum				Titlo

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<b>GDA2020</b>	RO LON
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OR USED FOR ANY OTHER PURPOSE OTHER THAN THAT ORIGINALLY	

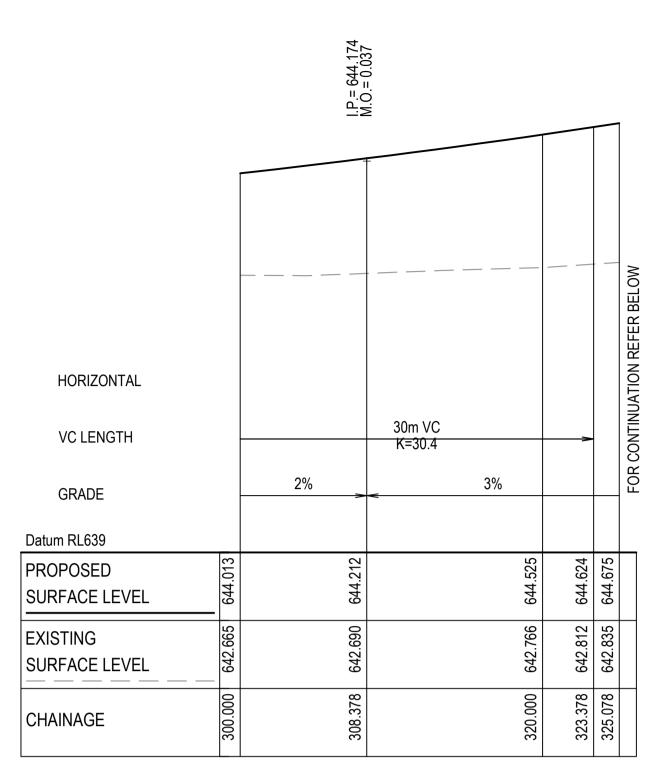
<b>EQUINOX - STAGE 3</b>
WILSON DRIVE,
MARULAN

OADWORKS NGITUDINAL SECTIONS SHEET 8

Civil Engineers and Proje	ect Manage
atel	Level 7, North Sy ABN 96

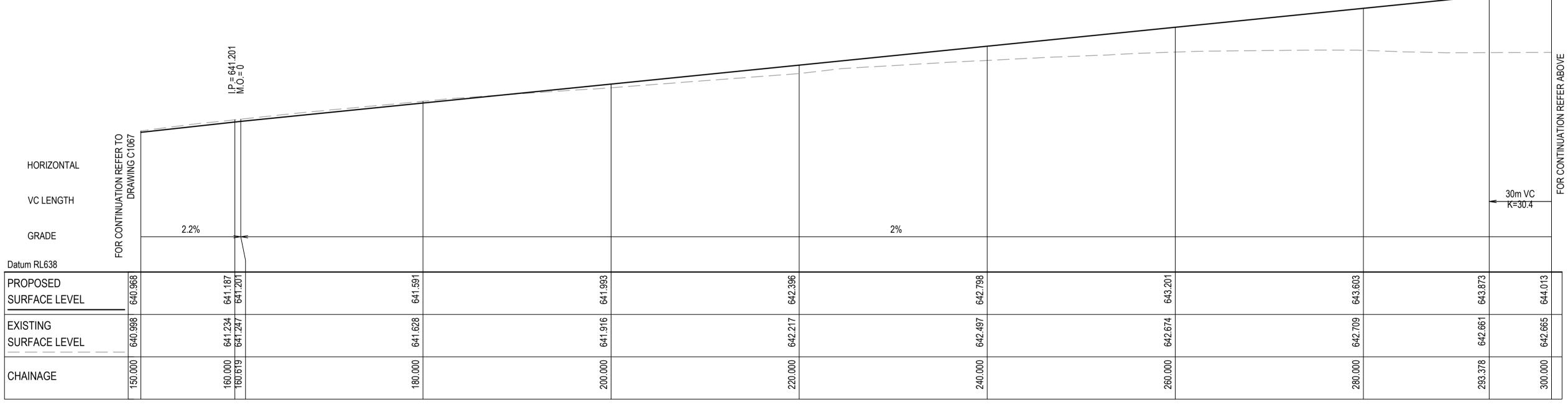
Level 7, 153 Walker Street North Sydney NSW 2060 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9923 1055 www.atl.net.au www.atl.net.au info@atl.net.au

PRELIMIN
NOT TO BE USED F
NOT TO BE USED F Project - Drawing No.
23-1098-C106



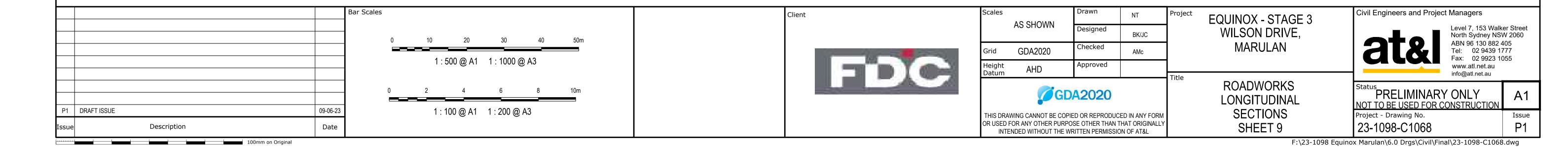
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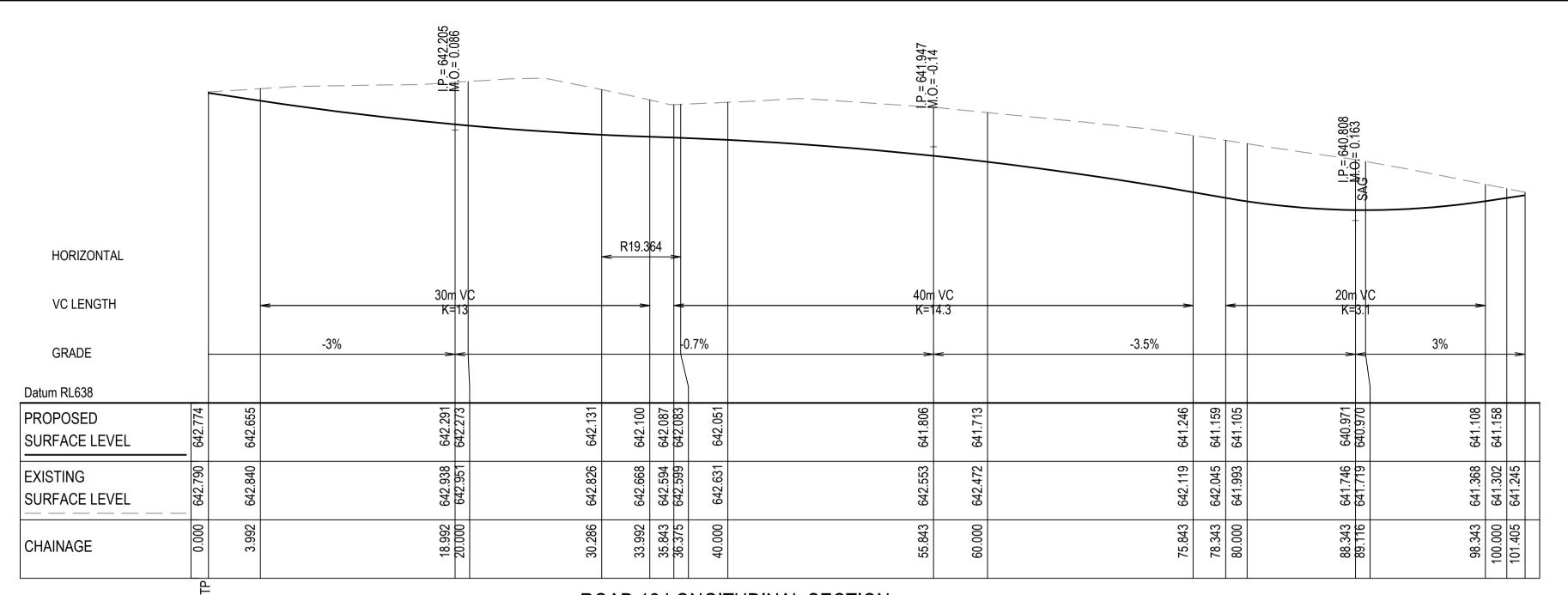
SCALE 1:500 HORI.
1:100 VERT.



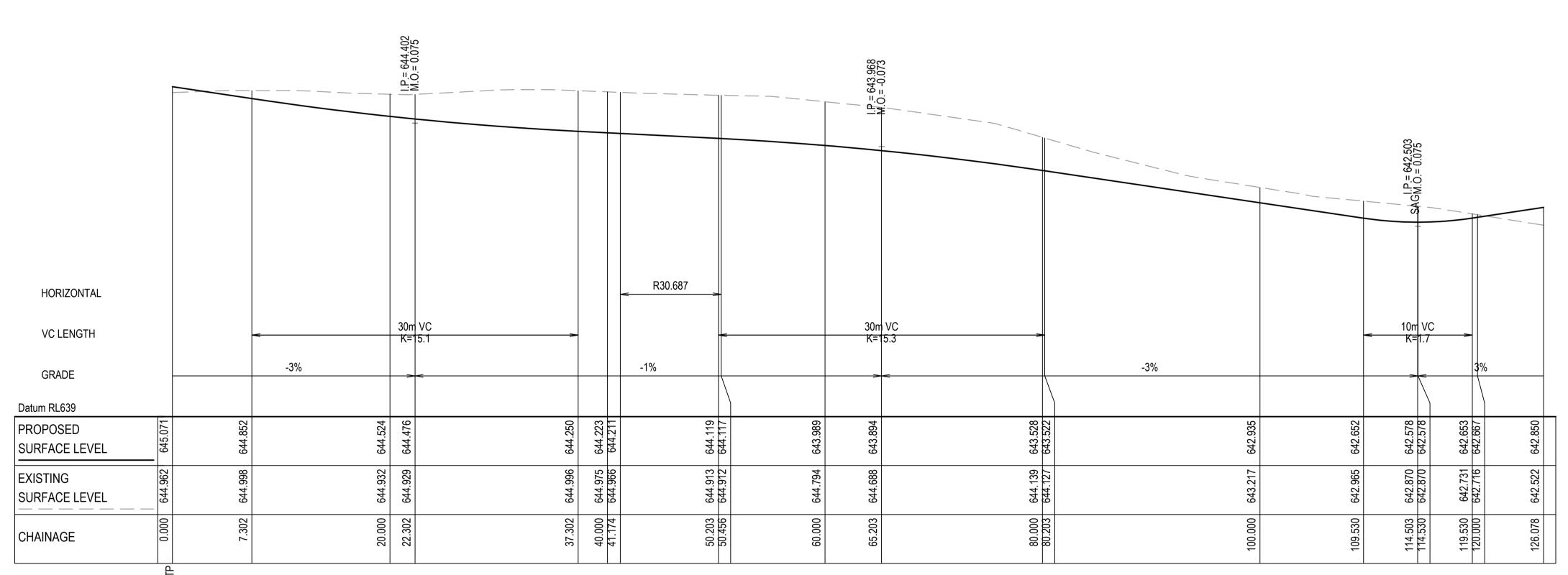
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SCALE 1:500 HORI.
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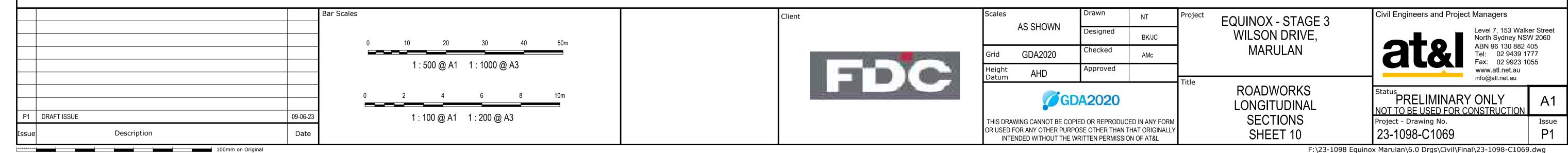


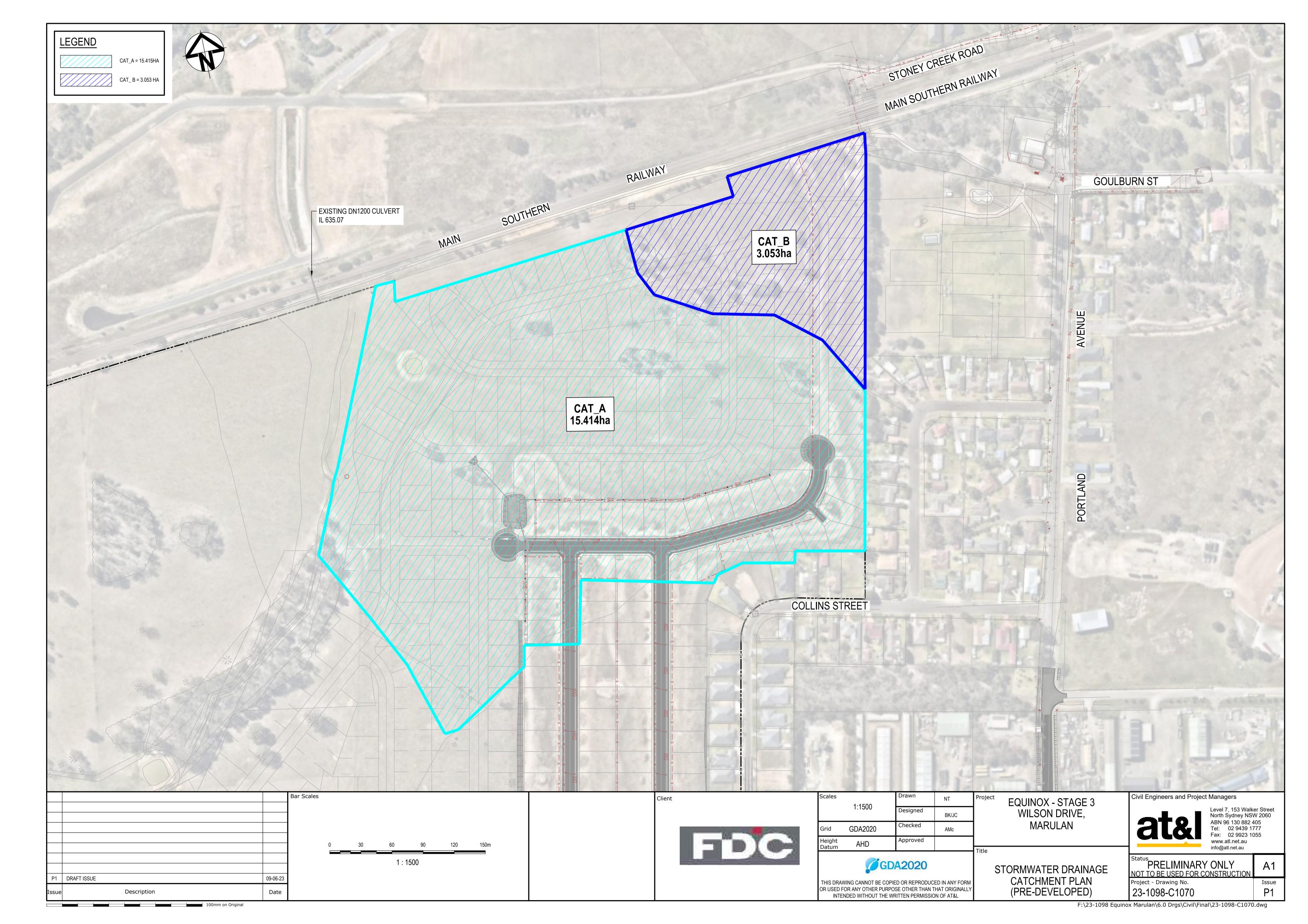


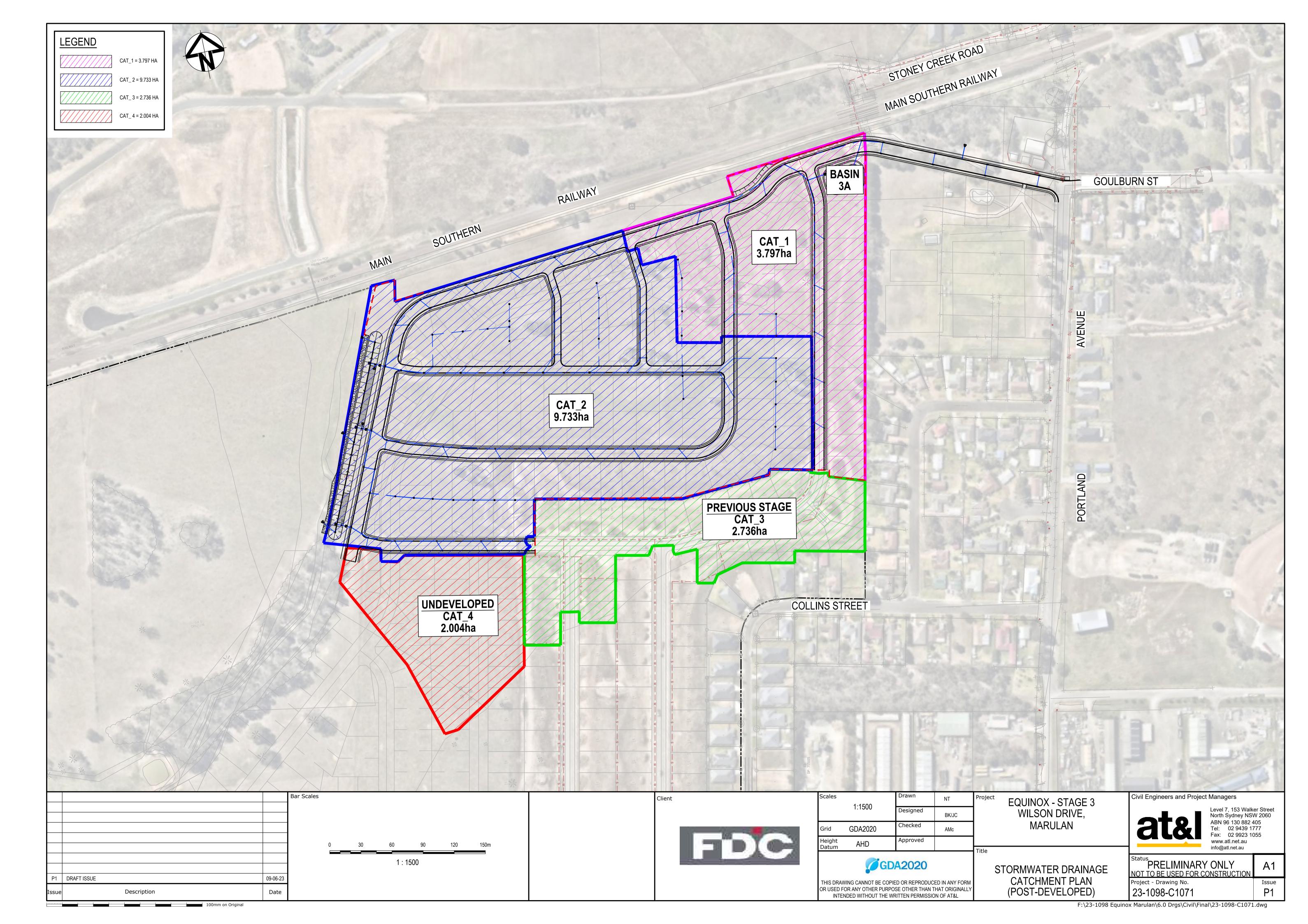
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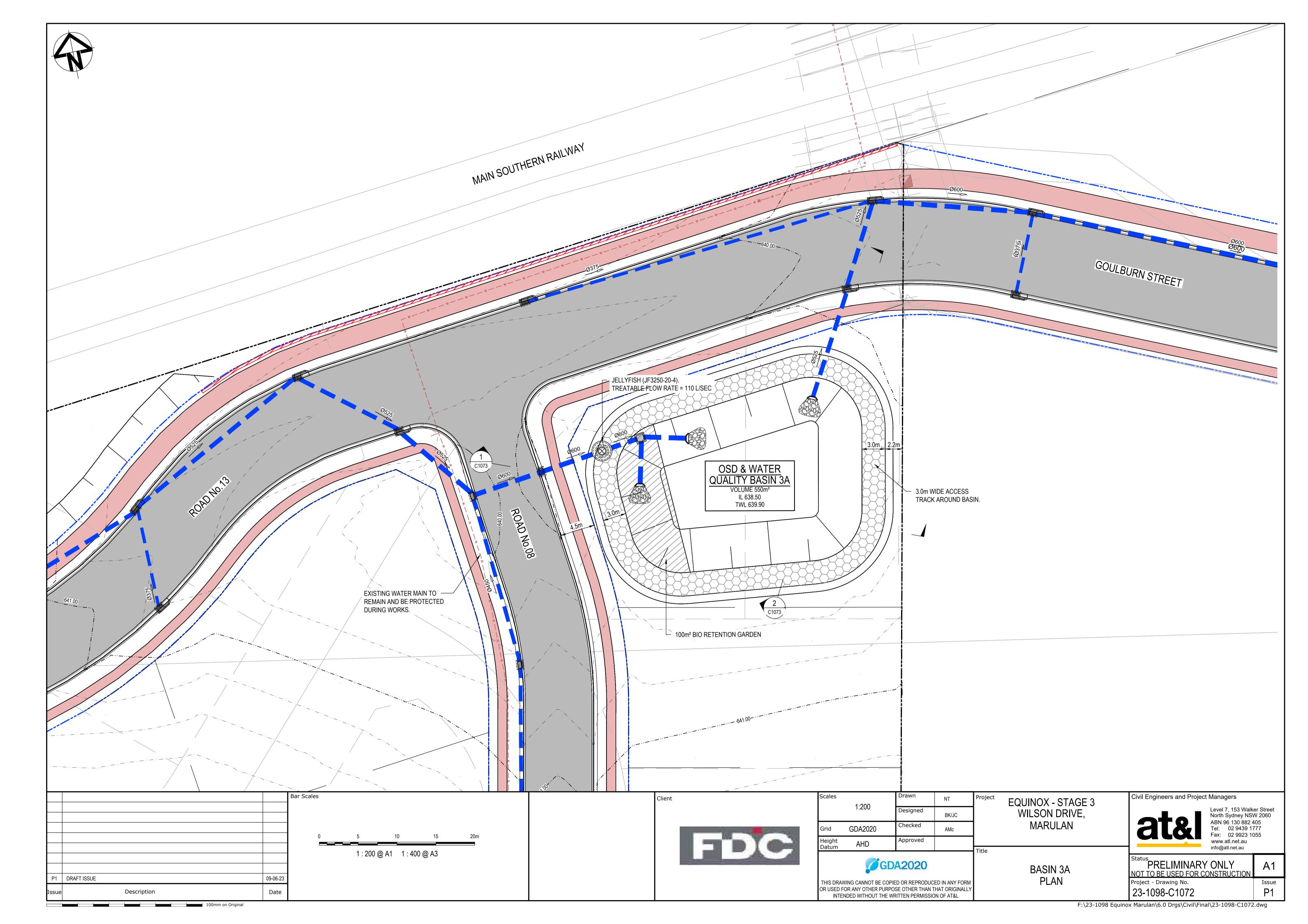


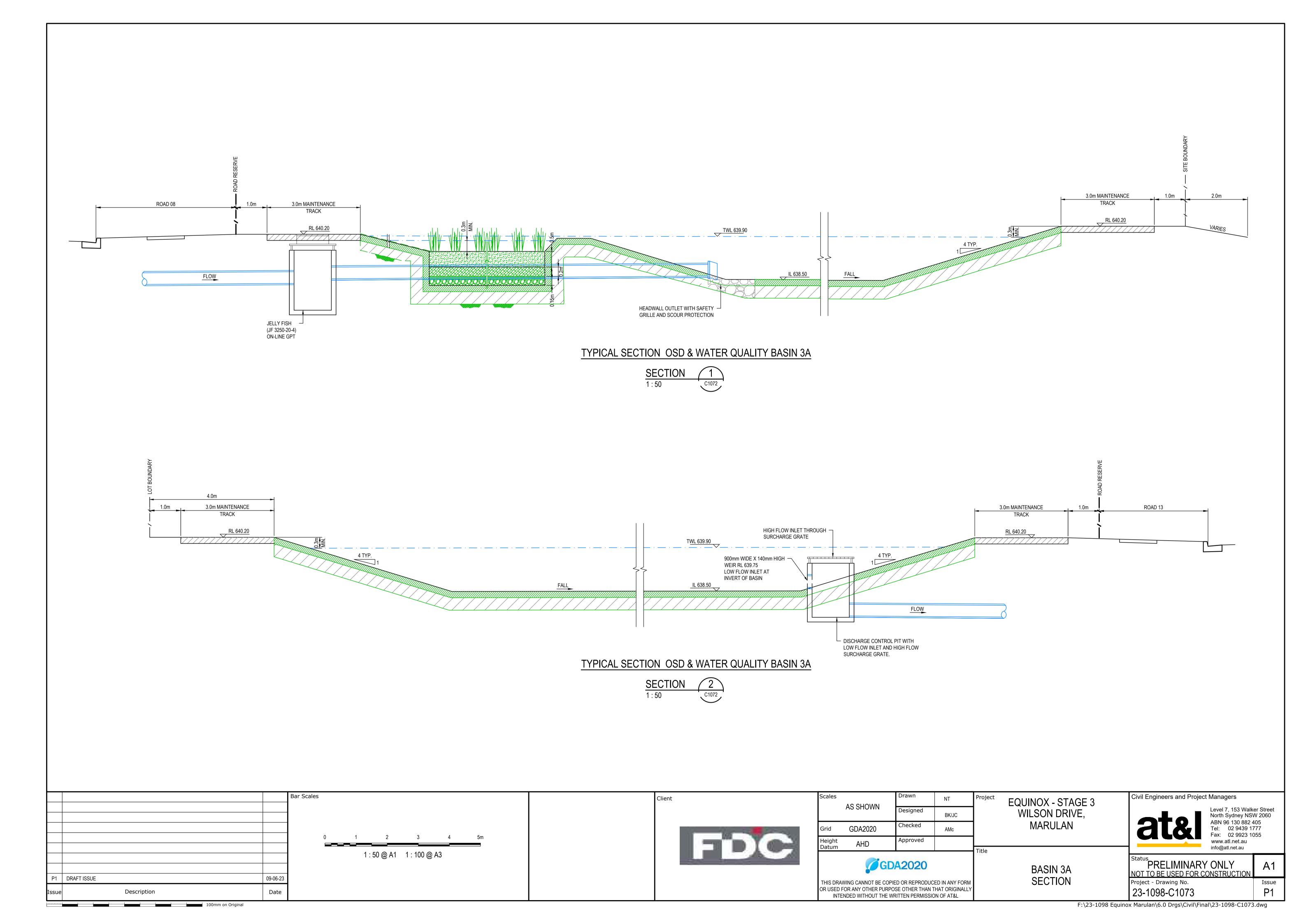
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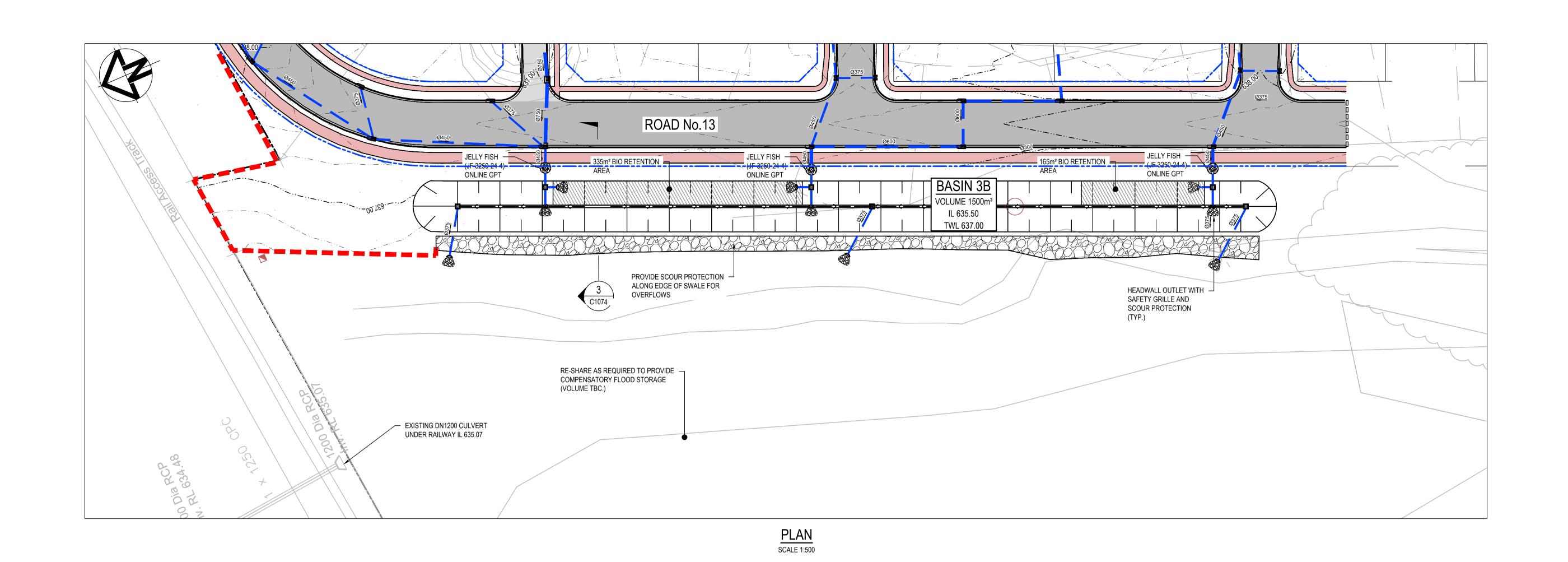


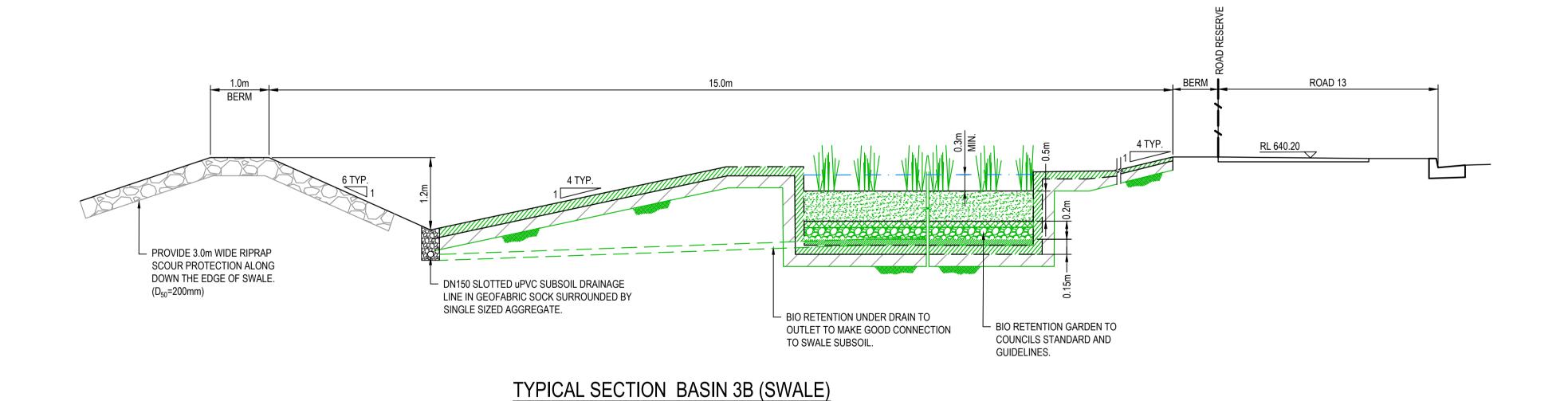


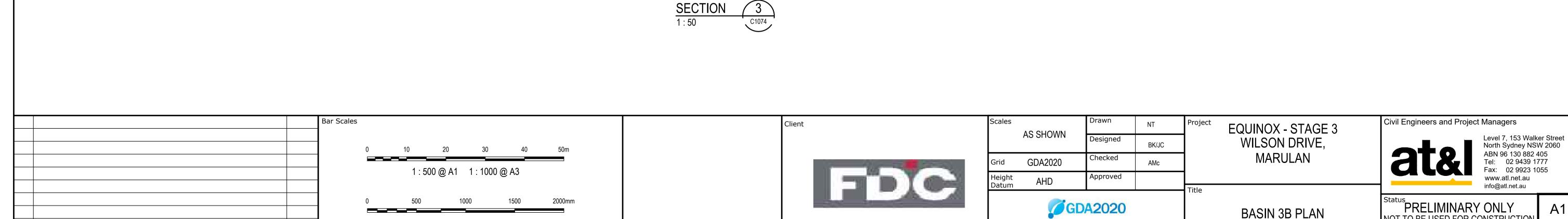












1:20 @ A1 1:40 @ A3

09-06-23

l Date ▶

P1 DRAFT ISSUE

Description

23-1098-C1074 F:\23-1098 Equinox Marulan\6.0 Drgs\Civil\Final\23-1098-C1074.dwg

Project - Drawing No.

NOT TO BE USED FOR CONSTRUCTION

BASIN 3B PLAN

AND SECTION

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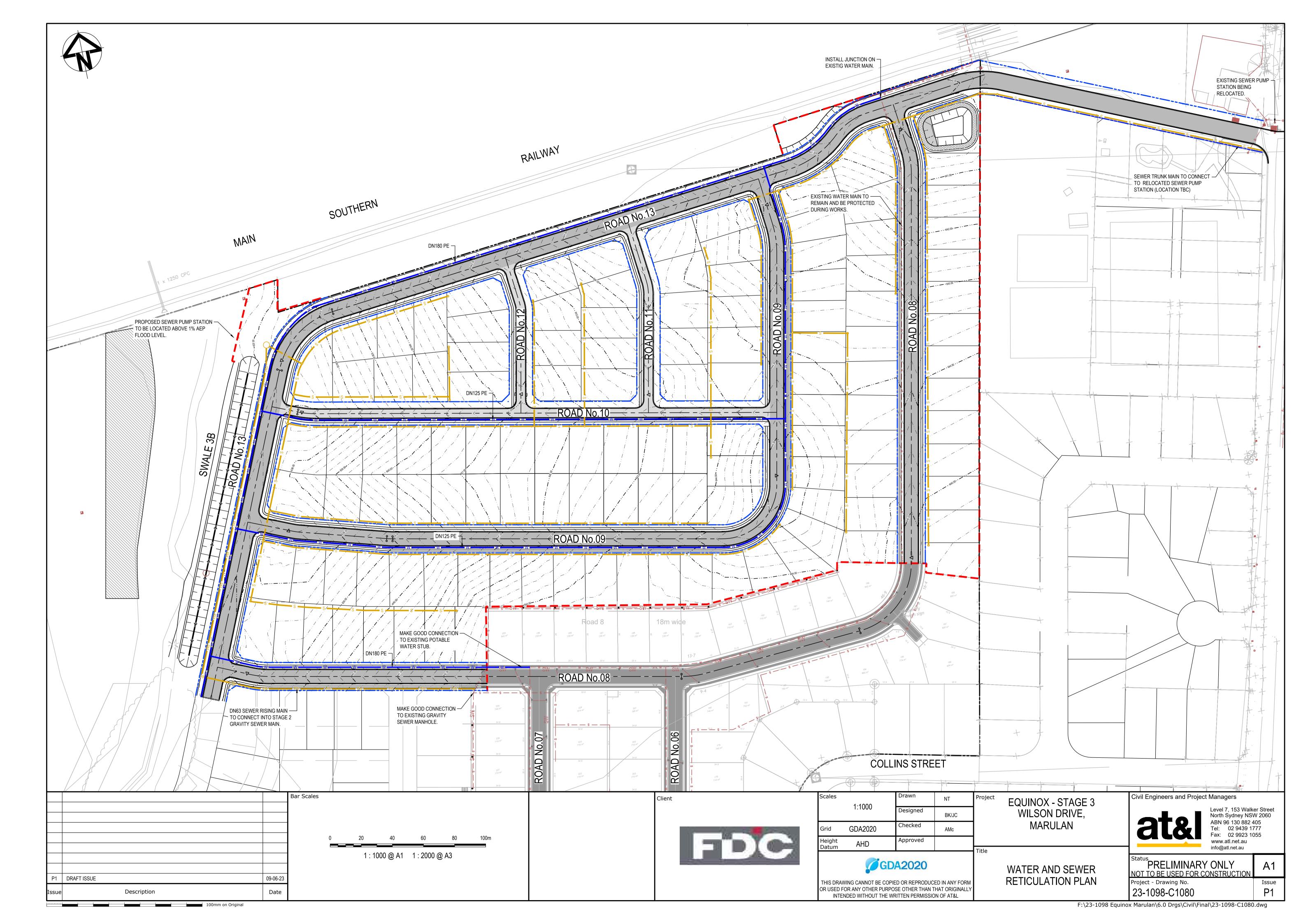
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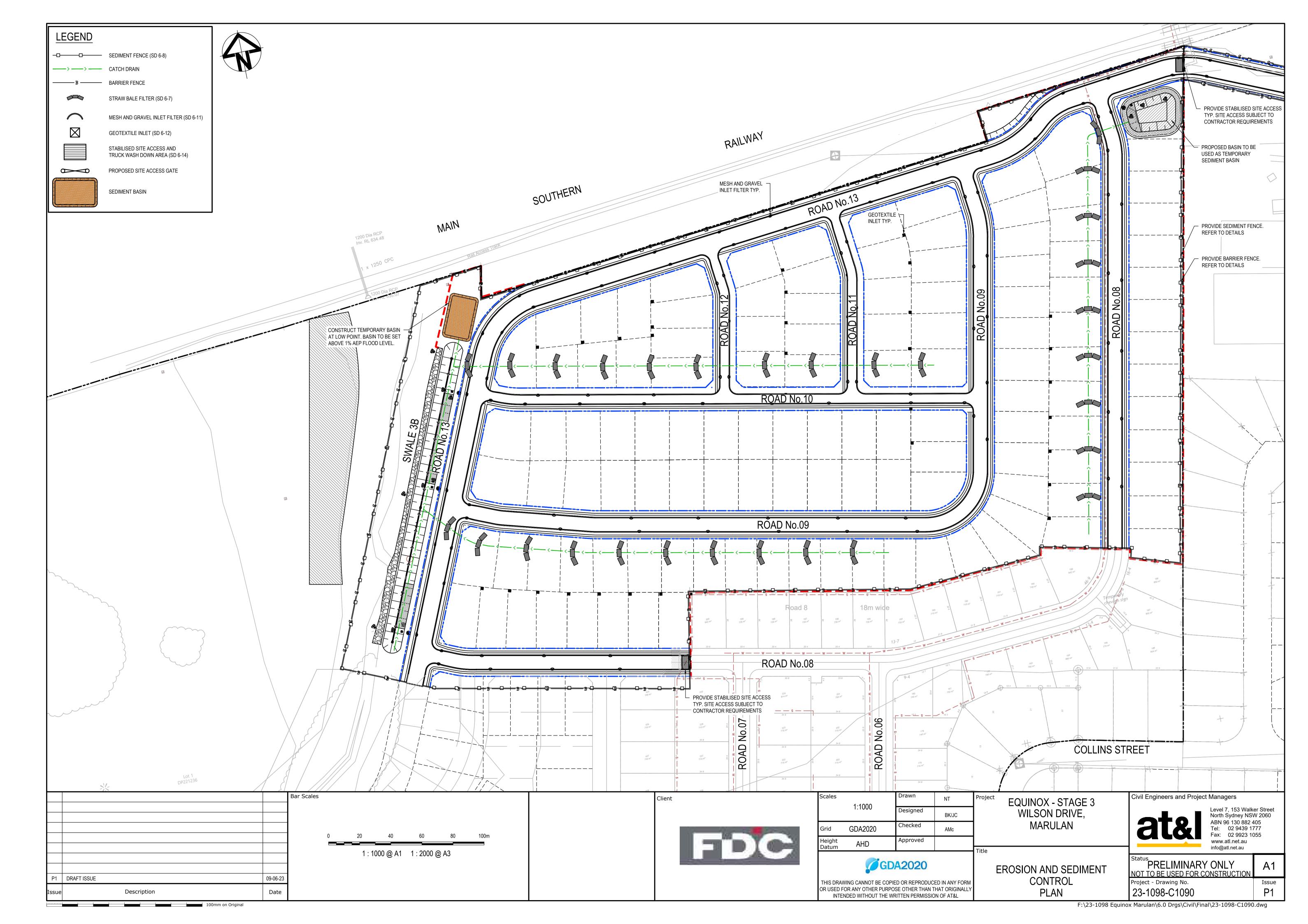
ABN 96 130 882 405 Tel: 02 9439 1777

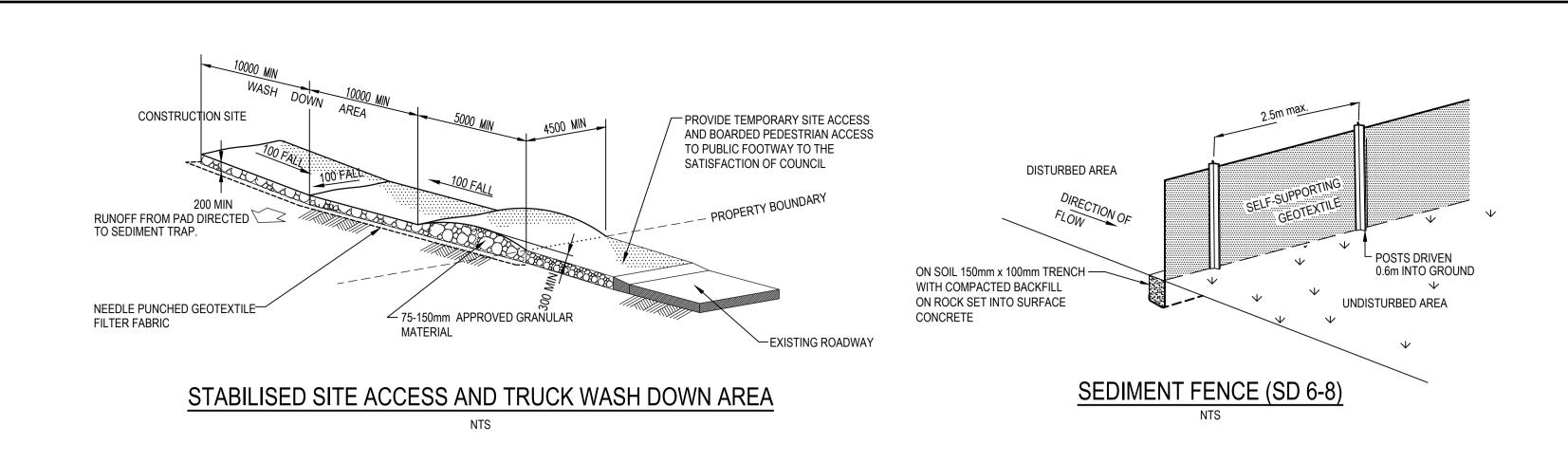
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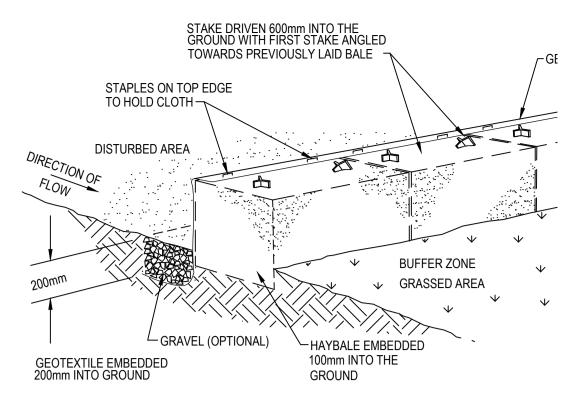
Issue

www.atl.net.au info@atl.net.au

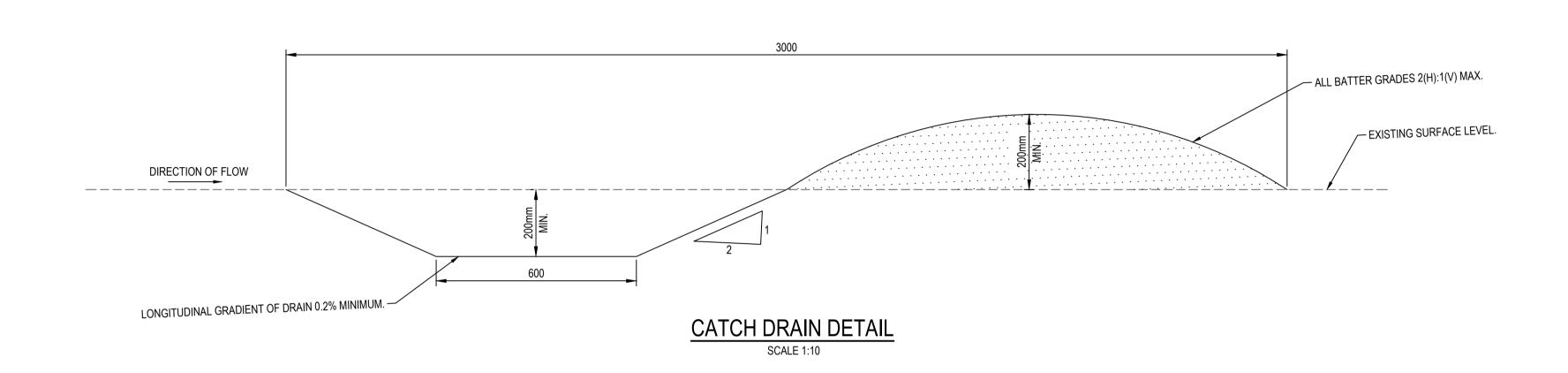


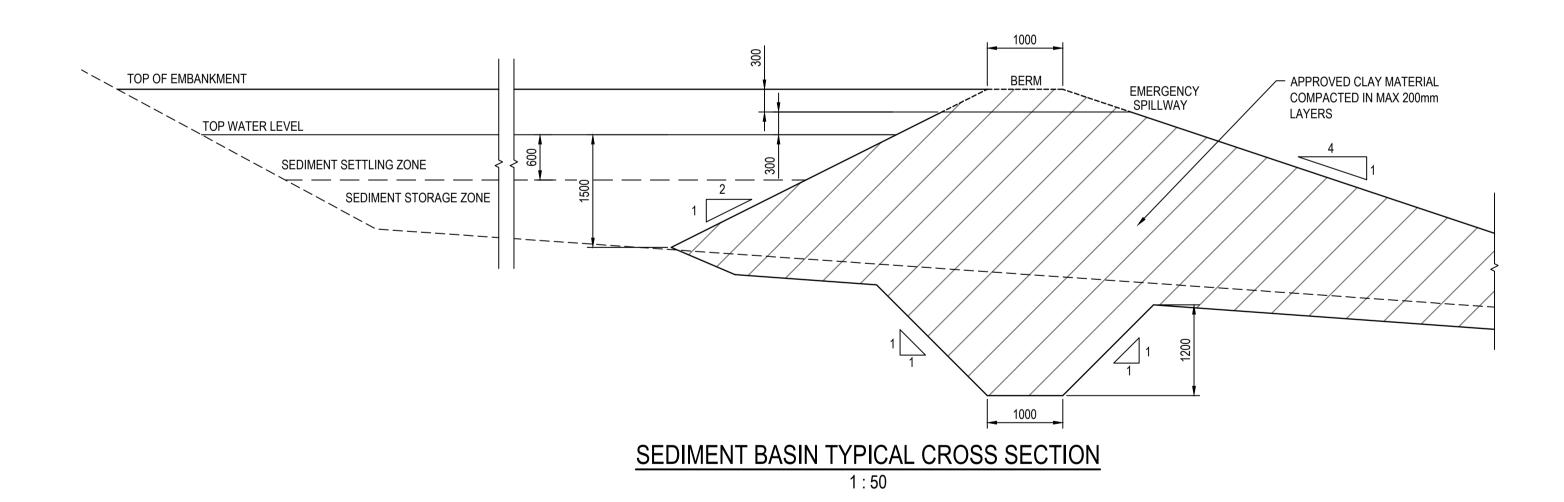


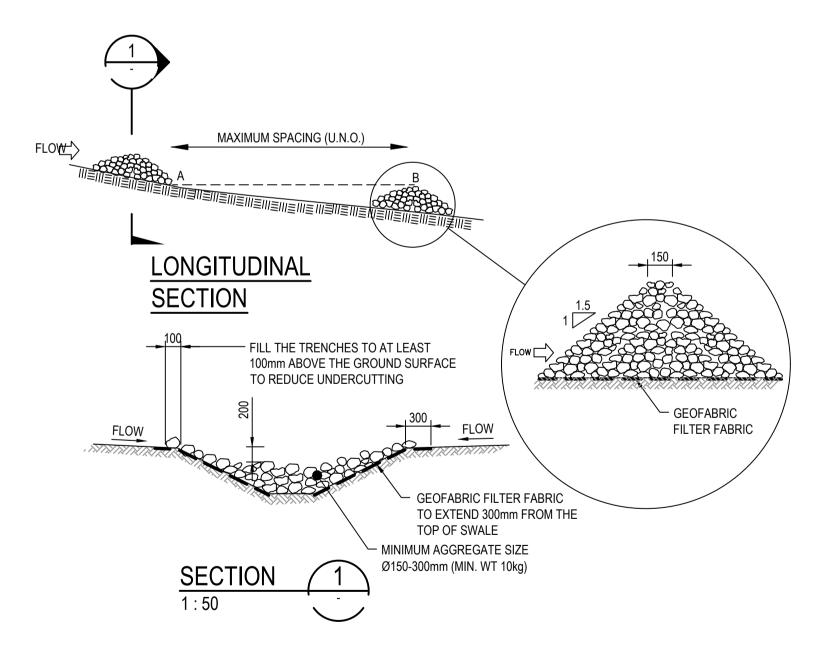




HAYBALE AND GEOTEXTILE SEDIMENT FILTE







ROCK RIFFLE CHECK DAM (SD 5-4) 1 : 50

			Bar Scales							
				0		200	400	600	800	1000mm
							: 10 @ A1	1:20@	A3	
			]				O	J		
				0		1	2	3	4	5m
P1	DRAFT ISSUE	09-06-23	]	1 : 50 @ A1						
Issue	Description	Date				'	. 50 W A1	1.100@	ΛU	

100mm on Original



Client

	Scales N.T.S.		Drawn	NT	Proje
			Designed	BK/JC	
	Grid	GDA2020	Checked	AMc	
	Height	AHD	Approved		

**GDA2020** 

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EQUINOX - STAGE 3 WILSON DRIVE, MARULAN

**DETAILS** 

PRELIMINARY ONLY

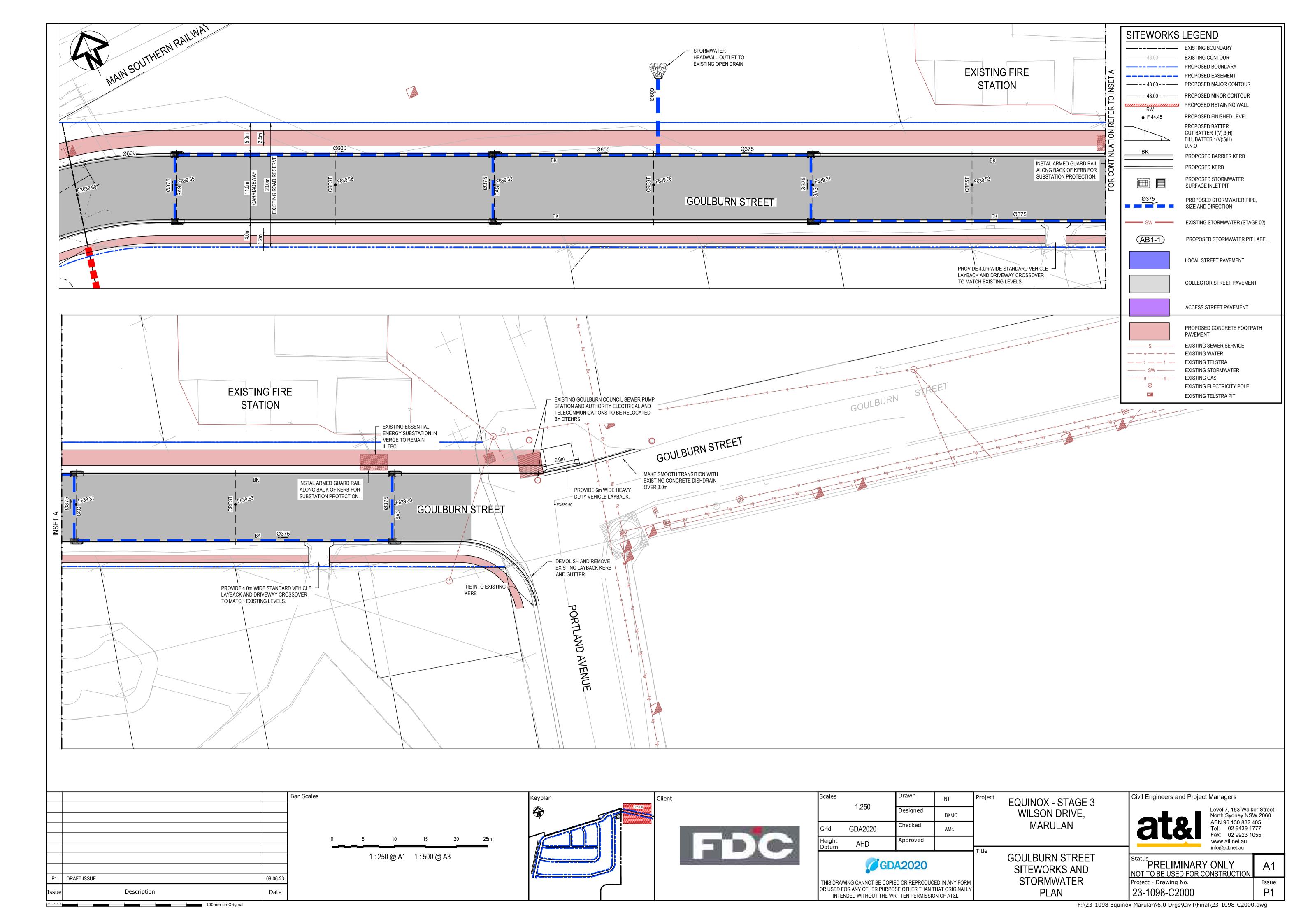
Level 7, 153 Walker Street North Sydney NSW 2060 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9923 1055 www.atl.net.au info@atl.net.au

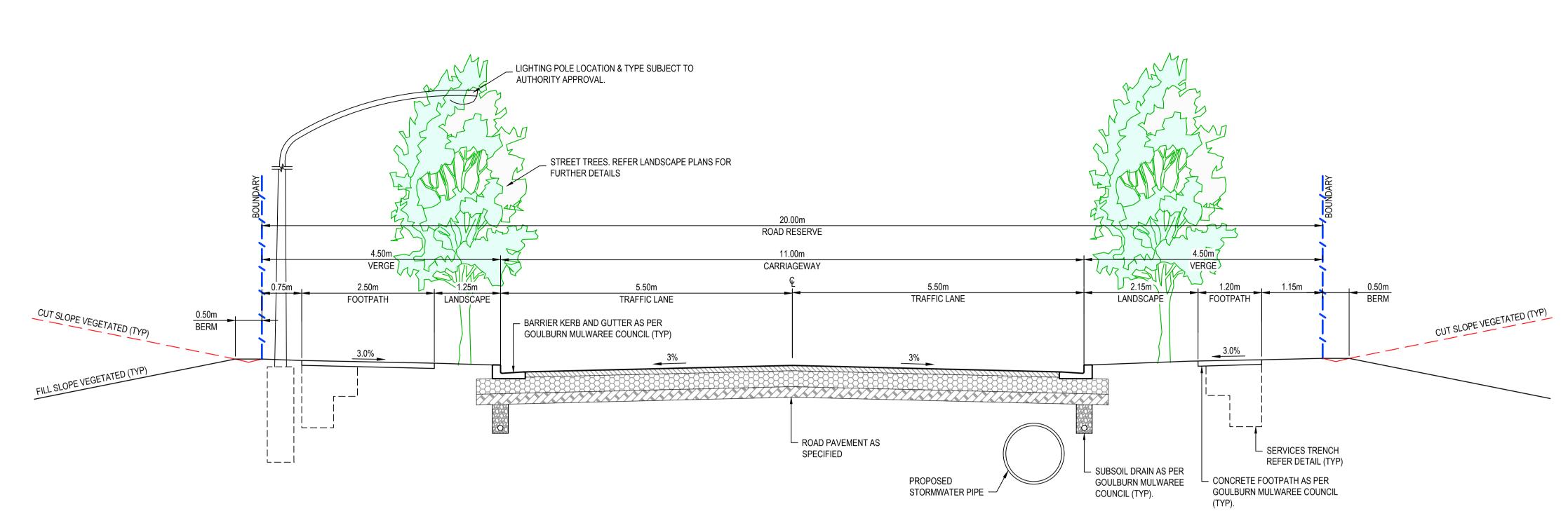
Issue

**EROSION AND SEDIMENT** 

NOT TO BE USED FOR CONSTRUCTION Project - Drawing No. 23-1098-C1091

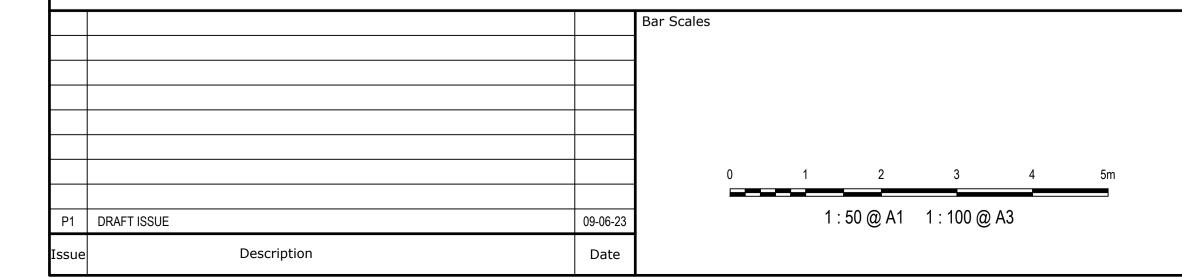
Civil Engineers and Project Managers





TYPICAL SECTION (COLLECTOR STREET)

SCALE 1:50





Client

Scales	AS SHOWN	Drawn	NT	Pr
		Designed	BK/JC	
Grid	GDA2020	Checked	AMc	
Height	AHD	Approved		

**GDA2020** 

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EQUINOX - STAGE 3 WILSON DRIVE, MARULAN

GOULBURN STREET TYPICAL SECTIONS

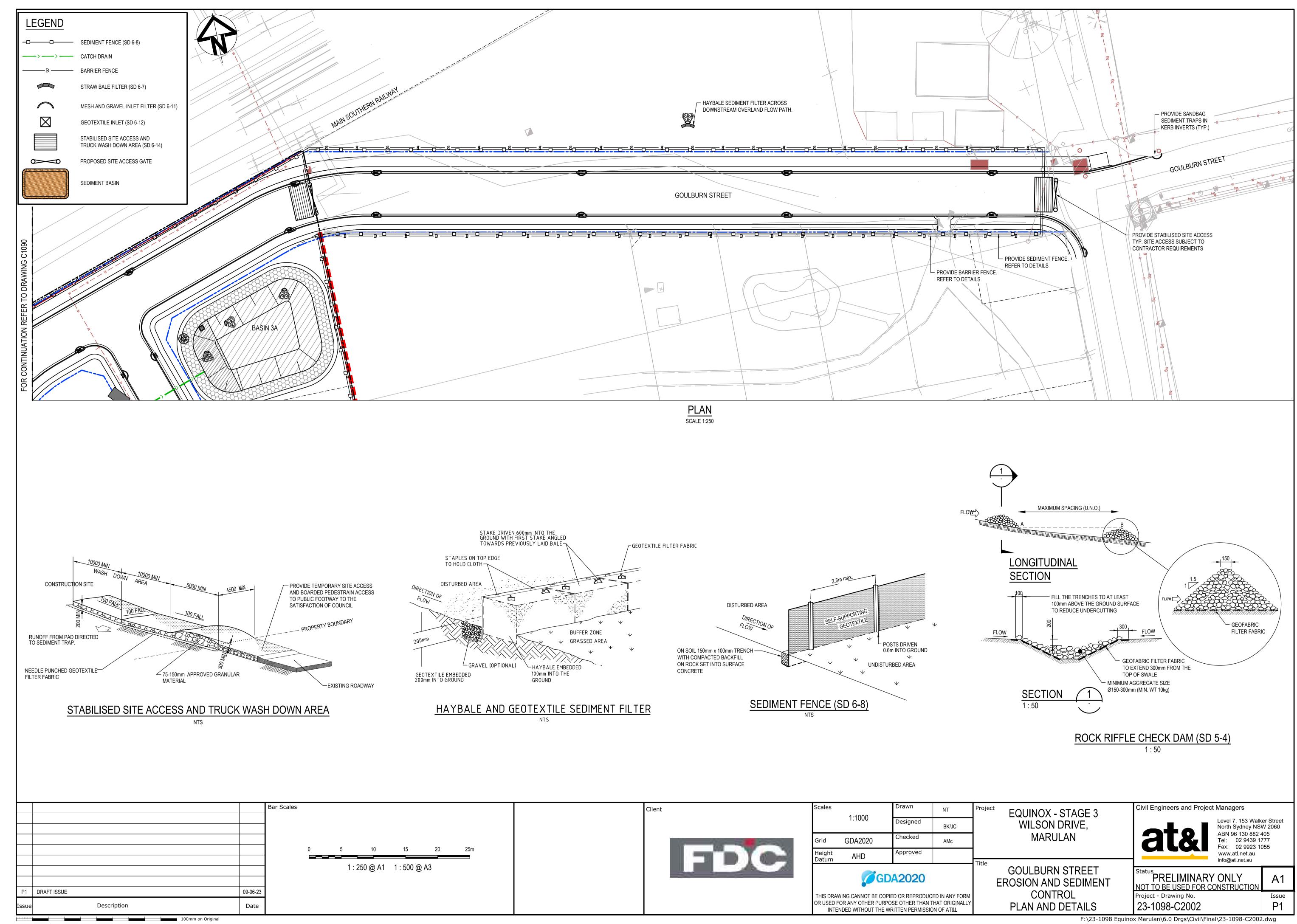
Civil Engineers and Project Managers



Level 7, 153 Walker Street North Sydney NSW 2060 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9923 1055 www.atl.net.au info@atl.net.au

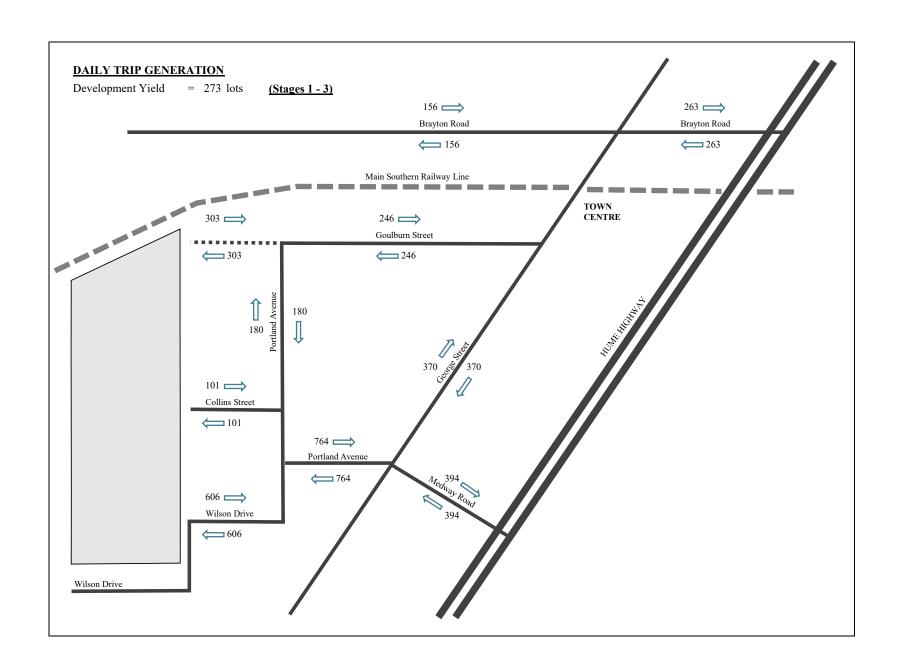
PRELIMINARY ONLY Project - Drawing No.

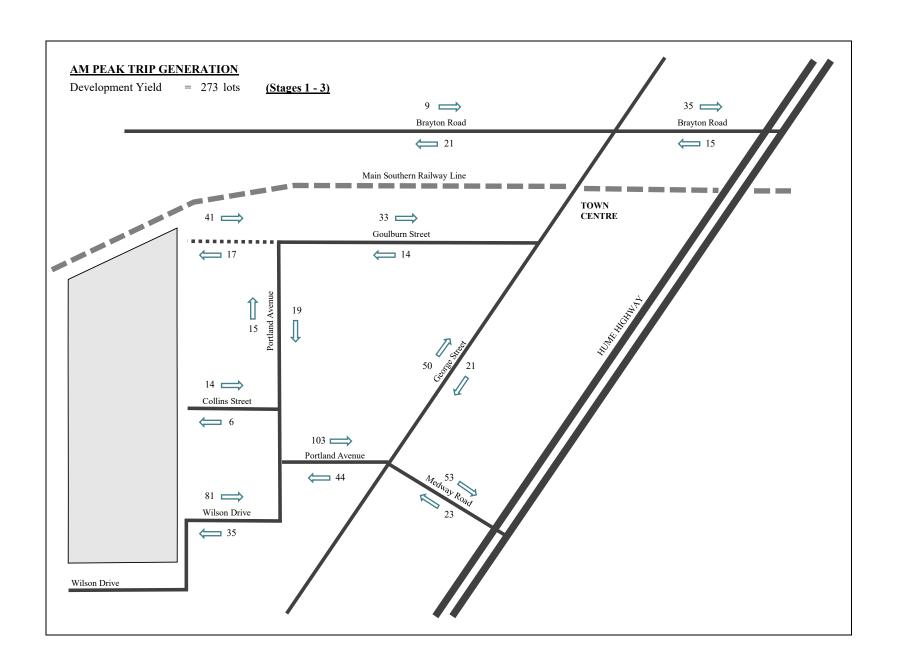
NOT TO BE USED FOR CONSTRUCTION Issue 23-1098-C2001

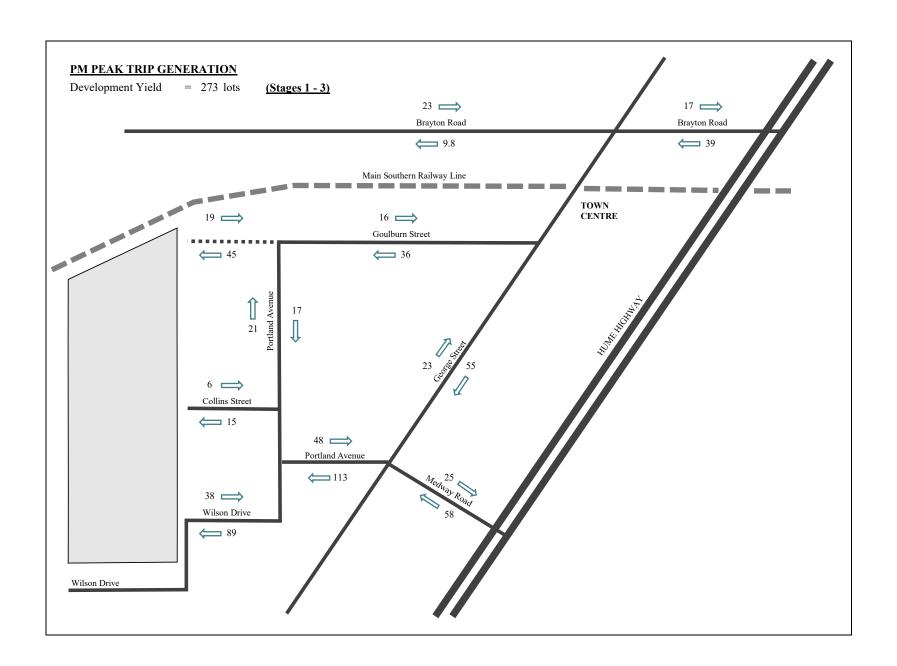


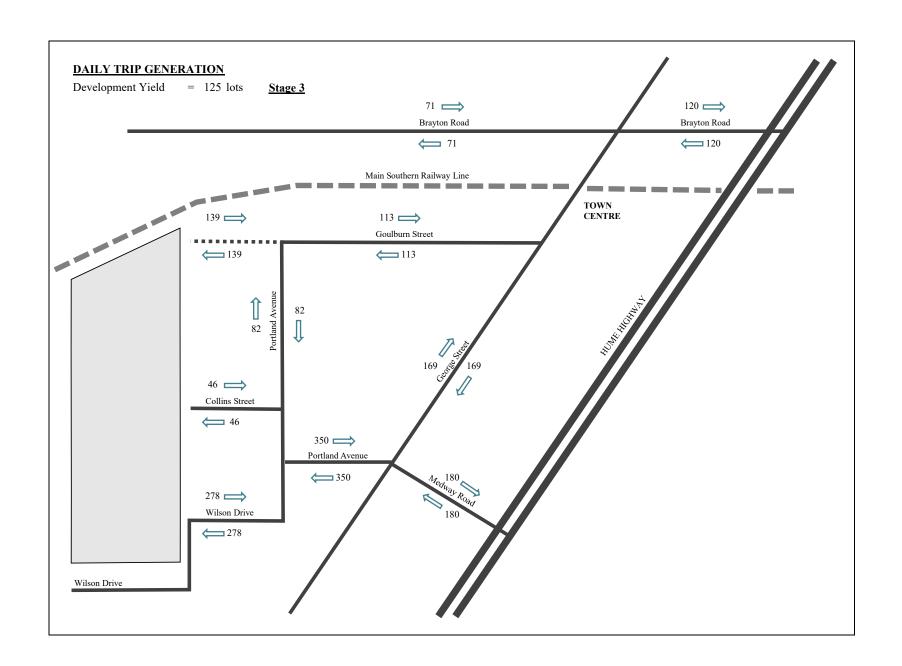
### **APPENDIX C**

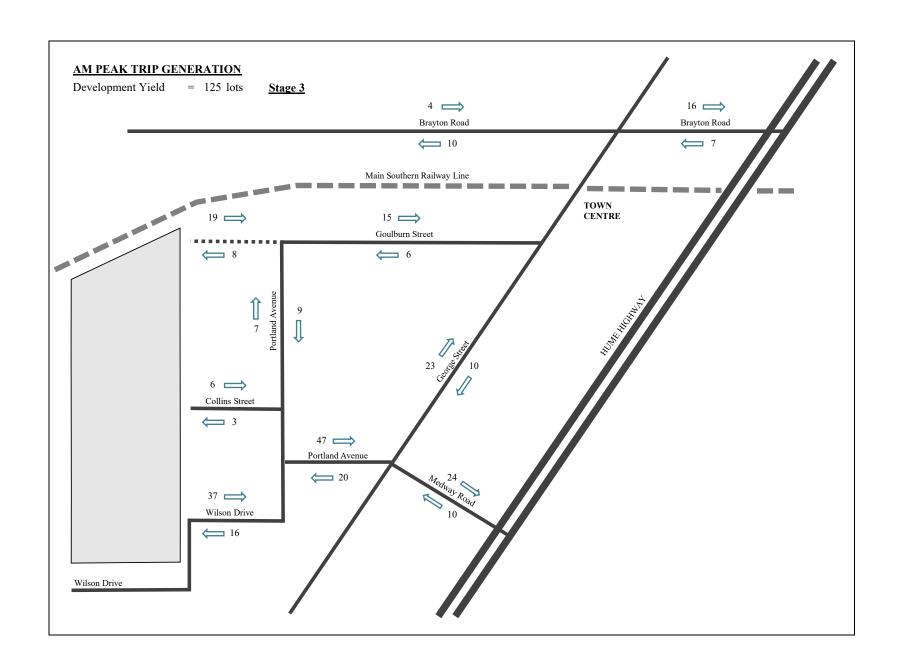
Forecast Network Volumes

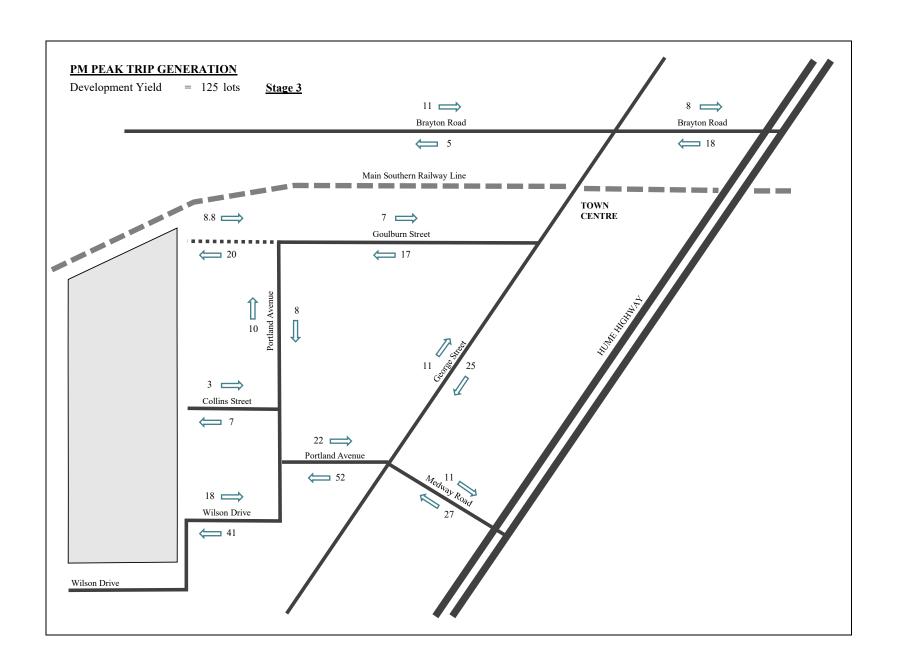












## **APPENDIX D**

Results of Traffic Counts – George Street / Goulburn Street Intersection



### **TURNING MOVEMENT SURVEY**

#### Intersection of Goulburn St and George St, Marulan

GPS	-34.712520,150.005416
Date:	Wed 25/05/22
Weather:	Fine
Suburban:	Marulan
Customer:	N/A

North: George St	
East:	N/A
South:	George St
West:	Goulburn St

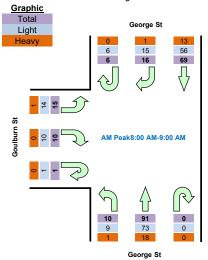
Survey	AM:	7:00 AM-9:00 AM
Period	PM:	2:30 PM-5:30 PM
Traffic	AM:	8:00 AM-9:00 AM
Peak	PM:	2:30 PM-3:30 PM

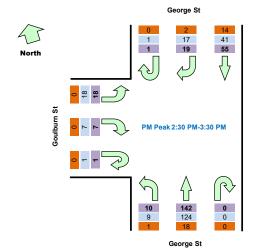
#### All Vehicles

Tir	ne	North Approach George St   South Approach George St   West Approach Goulburn St		Hourl	Hourly Total							
Period Start	Period End	U	R	SB	U	NB	L	U	R	L	Hour	Peak
7:00	7:15	0	3	2	0	16	1	0	0	5	118	
7:15	7:30	1	2	8	0	19	0	0	0	5	138	
7:30	7:45	0	4	5	0	12	2	0	0	4	142	
7:45	8:00	1	3	6	0	15	1	0	1	2	170	
8:00	8:15	0	2	14	0	20	1	0	4	6	218	Peak
8:15	8:30	0	0	15	0	18	3	0	0	3		
8:30	8:45	4	4	17	0	23	3	0	3	1		
8:45	9:00	2	10	23	0	30	3	1	3	5		
14:30	14:45	1	2	12	0	26	2	0	1	8	228	
14:45	15:00	0	5	13	0	21	6	0	0	4	245	
15:00	15:15	0	10	14	0	38	2	1	2	6	253	Peak
15:15	15:30	0	1	9	0	38	1	0	1	4	234	
15:30	15:45	1	6	22	0	33	5	0	0	2	225	
15:45	16:00	0	2	10	0	33	2	0	4	6	201	
16:00	16:15	0	5	10	0	29	2	0	2	6	215	
16:15	16:30	0	4	9	0	24	3	0	1	4	219	
16:30	16:45	0	8	3	0	23	2	0	0	9	221	
16:45	17:00	0	5	19	0	32	3	0	0	12	214	
17:00	17:15	0	12	7	0	28	1	0	2	8	186	
17:15	17:30	0	4	10	0	26	2	0	0	5		
17:30	17:45	1	4	2	0	23	2	0	1	5		
17:45	18:00	0	4	4	0	27	4	0	0	4		

Peak Time North Approach Geo					South Ap	proach G	George St	West App	roach Go	ulburn St	Peak
<b>Period Start</b>	Period End	U	R	SB	U	NB	L	U	R	L	total
8:00	9:00	6	16	69	0	91	10	1	10	15	218
15:00	16:00	1	19	55	0	142	10	1	7	18	253

Note: Site sketch is for illustrating traffic flows. Direction is indicative only, drawing is not to scale and not an exact streets configuration.







Light Vehicle

Light Vehic	Light Vehicles												
Tir	me	North Ap	proach G	eorge St	South Ap	proach G	eorge St	West App	roach Go	ulburn St			
Period Start	Period End	U	R	SB	U	NB	L	U	R	L			
7:00	7:15	0	2	2	0	14	1	0	0	5			
7:15	7:30	1	2	5	0	14	0	0	0	5			
7:30	7:45	0	3	5	0	9	2	0	0	3			

7:45	8:00	1	2	4	0	11	1	0	1	2
8:00	8:15	0	2	11	0	15	1	0	4	6
8:15	8:30	0	0	11	0	17	2	0	0	3
8:30	8:45	4	4	14	0	18	3	0	3	1
8:45	9:00	2	9	20	0	23	3	1	3	4
14:30	14:45	1	2	9	0	23	2	0	1	8
14:45	15:00	0	5	11	0	18	6	0	0	4
15:00	15:15	0	8	8	0	29	2	1	2	6
15:15	15:30	0	1	8	0	34	1	0	1	4
15:30	15:45	1	6	16	0	30	5	0	0	2
15:45	16:00	0	2	9	0	31	1	0	4	6
16:00	16:15	0	5	9	0	25	2	0	2	5
16:15	16:30	0	4	8	0	23	3	0	1	2
16:30	16:45	0	8	2	0	20	2	0	0	8
16:45	17:00	0	5	9	0	30	3	0	0	12
17:00	17:15	0	11	6	0	26	1	0	2	7
17:15	17:30	0	3	10	0	24	2	0	0	5
17:30	17:45	1	4	1	0	19	2	0	1	5
17:45	18:00	0	4	4	0	23	4	0	0	4

Peak Time North Approach George St				South Ap	Peak						
Period Start	Period End	U	R	SB	U	NB	L	U	R	L	total
8:00	9:00	6	15	56	0	73	9	1	10	14	184
15:00	16:00	1	17	41	0	124	9	1	7	18	218

Heavy Vehicles

ricavy verne					South Approach George St West Approach Goulburn S							
	me		proach G		South A		Seorge St	West App	oroach Go	oulburn St		
Period Start	Period End	U	R	SB	U	NB	L	U	R	L		
7:00	7:15	0	1	0	0	2	0	0	0	0		
7:15	7:30	0	0	3	0	5	0	0	0	0		
7:30	7:45	0	1	0	0	3	0	0	0	1		
7:45	8:00	0	1	2	0	4	0	0	0	0		
8:00	8:15	0	0	3	0	5	0	0	0	0		
8:15	8:30	0	0	4	0	1	1	0	0	0		
8:30	8:45	0	0	3	0	5	0	0	0	0		
8:45	9:00	0	1	3	0	7	0	0	0	1		
14:30	14:45	0	0	3	0	3	0	0	0	0		
14:45	15:00	0	0	2	0	3	0	0	0	0		
15:00	15:15	0	2	6	0	9	0	0	0	0		
15:15	15:30	0	0	1	0	4	0	0	0	0		
15:30	15:45	0	0	6	0	3	0	0	0	0		
15:45	16:00	0	0	1	0	2	1	0	0	0		
16:00	16:15	0	0	1	0	4	0	0	0	1		
16:15	16:30	0	0	1	0	1	0	0	0	2		
16:30	16:45	0	0	1	0	3	0	0	0	1		
16:45	17:00	0	0	10	0	2	0	0	0	0		
17:00	17:15	0	1	1	0	2	0	0	0	1		
17:15	17:30	0	1	0	0	2	0	0	0	0		
17:30	17:45	0	0	1	0	4	0	0	0	0		
17:45	18:00	0	0	0	0	4	0	0	0	0		

Peak	Time	North Ap	proach G	eorge St	South Ap	Peak					
Period Start	Period End	U	R	SB	U	NB	L	U	R	L	total
8:00	9:00	0	1	13	0	18	1	0	0	1	34
15:00	16:00	0	2	14	0	18	1	0	0	0	35